### EVALUATION OF OPEN-GRADED FRICTION COURSE MIXTURE

### LOUISIANA TRANSPORTATION RESEARCH CENTER Technical Assistance Report Number 04-1TA

State Project No. 009-02-0018 US 71 Friction Course (CM) Route US 71 Grant Parish

By

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Table of Co
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Introduction
Project Description
Materials
Asphalt Cement
Aggregates4
Fibers
Antistrip4
Tack Coat
Open-graded Friction Course Mixture Design
Profilograph
Cost
Comments
Recommendations
References
Appendix A: Hotmix Plant and Roadway Pictures
Appendix B: Contract Specifications
Appendix C: Job Mix Formula
Appendix D: Film Thickness Calculation
Appendix E: LADOTD & Contractor's Project Personnel Comments

### List of Figures

Figure 1 - Project Location	3
Figure 2 - Mixture Design 1, Rut Measurements (mm) vs Number of Passes	9
Figure 3 - Mixture Design 2, Rut Measurements (mm) vs Number of Passes	9

### List of Tables

Table 1 - Composition of Mix Design Blends	6
Table 2 - Composite Blends and Mixture Properties	7
Table 3 - Rut Measurements (mm)	8
Table 4 - CoreLok Vs. AASHTO T166 Air Voids       1	0
Table 5 - Coefficient of Permeability (Feet/Day)       1	0

### Introduction

Open-graded friction course (OGFC) is a porous, gap-graded, predominantly single size aggregate bituminous mixture that contains a high percentage of air voids. The high air void content and the open structure of this mix promote the effective drainage of rainwater, which also minimizes hydroplaning during wet weather. This characteristic also reduces splash and spray behind vehicles and improves wet weather skid resistance. Other purported benefits of this type mix are lower pavement noise and reduced roadway glare during wet weather, which improves the night visibility of pavement markings.

OGFCs have been used throughout the United States since 1950 [1, 2]. Some state departments of transportation have reported good performance, but many others have reported poor performance [1, 2]. Louisiana first developed an OGFC in the late 1960s and early 1970s to provide a skid resistant surface [3, 4]. The open texture of the friction course reduced water spray and increased critical hydroplaning speeds. Louisiana's OGFC was developed prior to the initiation of the Federal Highway Safety Program Management Guide, Highway Safety Program 12, and Instructional Memorandum 211-3-73 of 1973 dealing with the establishment of a Skid Accident Reduction Program. Louisiana had already placed several OGFCs prior to the Instruction Memorandum. After receiving the Instructional Memorandum, the Louisiana Department of Transportation and Development (LADOTD) issued an Engineering Directive to use Plant Mix Seal (PMS) (used interchangeably with OGFC and asphaltic concrete friction course (ACFC)) on all roads with an ADT greater than 4,000. In 1980, the ADT limit was revised to require the friction course on all roads with an ADT greater than 3,000.

In late 1980, problems with the OGFCs were encountered. Many of these surfacings had reached their end-of-life, having lasted much longer than the original life expectancy of five years, typically 10 - 12 years. The end-of-life was signaled by severe raveling in the wheel paths due to oxidized asphalt binders and subsequent decrease in serviceability. This, in conjunction with numerous OGFC failures during the construction phase or shortly thereafter, led to a moratorium on its use.

Inspection of the failed construction projects and project records indicated that the problems encountered with the OGFC were related to moisture and temperature. The temperature problems were related to both mix and weather; the moisture problems were generally associated with a particular aggregate type. To address these issues, changes were made to the specifications, including a maximum moisture content for the aggregate, institution of a

construction season from May to September, and an increased minimum ambient air temperature. Based on these changes, the moratorium was lifted.

It should also be noted the design asphalt content of OGFCs was significantly decreased in 1979. This decrease in asphalt content along with the use of asphalt cements composed of base asphalt that oxidized rapidly contributed to all of these construction problems although it was not recognized at the time. Typical deterioration consisted of raveling in the wheel path.

In the next 1 ½ years, 12 OGFCs were placed without incidence. However, because the winters of 1982 and 1983 were extremely severe, the previously placed friction courses were reaching their end-of-life at approximately 8 to 11 years of age. Additionally, because of several oil boycotts and increases in the cost of crude, asphalt cement contents were reduced. In the beginning of 1984, with hundreds of miles of OGFC at end-of-life and raveling, a public and political uproar forced the imposition of a second moratorium which is still in effect today.

In 1984, a final experimental polymer modifying OGFC was placed on LA 48, Poydras-Reggio ten mile construction project. Two experimental sections (4 lane miles each) were constructed using a latex modified asphalt (similar to the current PG 70-22m) and an elastomeric polymer modified asphalt (similar to the current PG 76-22m). These sections were placed with an asphalt content 0.7 percent higher (similar to the 1960/70 binder levels) than the control sections with AC-30 which used the 1979 binder content. It was polymer modified asphalt along with fibers that permit additional asphalt creating greater film thickness and reduced draindown. Within one year the control sections raveled in the crossovers and turn lanes; within two years the control sections had raveled in the inside and outside wheel paths. The polymer modified section was still performing without raveling when the entire project was rehabilitated circa 1999-2001. This 15-17 year lifespan is consistant with design models used today.

This paper documents Louisiana's first use of this type mix since the suspension of OGFC mixes in 1984.

### **Project Description**

Figure 1 illustrates the project location, which is on US 71 in Grant Parish and begins 4.041 miles (Log Mile 4.041) north of the beginning of the control section (Rapides Parish line, SE of Rock Hill) and commences northward for 0.157 miles to its project ending limits (Log

Mile 4.198). This highway is 24 feet wide (2 12- foot travel lanes) with 2 10-foot improved hot mix shoulders. The contract was bid under State Project No. 009-02-0018 and was awarded to the low bidder, Diamond B. Construction Co., LLC. The work order date for this project was June 7, 2003, and the final inspection date was June 26, 2003. The OGFC was placed on June 15, 2003 and was completed in one day. The OGFC was placed at approximately three-fourths of an inch compacted thickness, and the area covered was minimal at 2181.30 square yards. Approximately 94.0 tons of OGFC was placed during construction. This project site was selected because District 08 wanted to improve the existing surface characteristics at this location, and the placement of the OGFC was applicable in this area.



Figure 1 Project Location

### Materials

#### Asphalt Cement

An elastomeric type of polymer modified asphalt cement was specified for this project, meeting the LADOTD specification for PG76-22m. The PG76-22m asphalt cement was listed on QPL #41 and was supplied by Marlin Asphalt, LTD. The polymer modified asphalt cement content was 6.6 percent as designed by the contractor, Diamond B. Construction Co., LLC.

### Aggregates

The final aggregate blend, Design 2, was composed of 67.2 percent - #78 sandstone, Friction Rating I; 7.4 percent - #11 sandstone, Friction Rating I; and 18.7 percent - #89 siliceous limestone, Friction Rating III (see tables 1 and 2). The sandstone was supplied by Pine Bluff Sand & Gravel Co. (Source Code AB13). The limestone was supplied by Vulcan Material Company (Source Code AA50). The aggregates used complied with the requirements set forth in Subsection 1003.06(b) of the Standard Specifications.

### Fibers

A mineral fiber in pellet form was added to the mix at a mix percentage of 0.1 by weight to protect against drain down. The fiber was supplied by Interfibe. The contract specifications required that drain down testing be conducted in accordance with Section 508 of the 2000 Edition of the Louisiana Standard Specifications [5] on the loose mix at a temperature 60°F (15°C) higher than normal mixing temperatures. A maximum drain down of 0.3 percent is required. The approved Job Mix Formula (JMF) indicated that the maximum drain down for the OGFC mixture tested was 0.08 percent using the minimum dosage of fiber specified.

### Antistrip

The contractor was required to perform the Boil Test and modified Lottman test to evaluate the mixture's susceptibility to moisture damage. An Ad-Here LA 2 from Arr-Maz Products, Inc. was added at mix percentage of 0.6 by weight. The Lottman test was modified to require five freeze thaw cycles.

### Tack Coat

The contractor elected to use the unmodified SS-1 emulsion for tack coat as allowed by Section 504 of the Standard Specifications [5]. The SS-1 emulsion was listed on QPL #41 and supplied by Asphalt Products Unlimited. The tack coat rate to be applied was 0.07 gallons/square yards, as required by the Special Provisions of the contract. The tack coat rate was not measured; however, the tack coat coverage was uniform and covered 100 percent of the existing dense graded asphalt surface, which was approximately 3 years old.

### **Open-graded Friction Course Mixture Design**

The mix design procedures for this project were detailed in the Special Provisions of the contract. The specified OGFC design requirements followed the recommendations as outlined in the 2000 Edition of the Journal of the Association of Asphalt Paving Technologists (AAPT) [6]. The contractor was required to use approved PG76-22m asphalt cement complying with Section 1002 in the Standard Specifications and listed in QPL #41 [5]. It was further specified that the aggregates, coarse and fine, should be 100 percent crushed stone with a Friction Rating of I, thus complying with the requirements set forth in Subsection 1003.06(b) of the Standard Specifications [5]. A Cellulose fiber or mineral filler may also be used to ensure protection against drain down. Also, an anti-strip additive was required to prevent stripping. The OGFC Special Provisions of the contract and the approved JMF are in Appendix B and Appendix C, respectively.

During the mix design process, the contractor evaluated two designs. See tables 1 and 2. The first mixture design, Design 1, incorporated a blend of two sandstone gradations that subsequently failed during the Hamburg rut testing performed by LTRC. The samples obtained from the Design 1 mix disintegrated or fell apart during testing in the Hamburg. In the second mixture design, Design 2, the contractor was allowed to blend less than 25 percent of a #89 siliceous limestone meeting a Friction Rating III. The incorporation of the #89 stone was necessary to introduce some intermediate fine material into the design blend to ensure the stability of the mix during Hamburg rut testing. This particular aggregate was allowed because its availability facilitated the timely completion of the project. It should be noted that Design 2, which Diamond B. Construction Company selected as the JMF and submitted to LADOTD, was subsequently used to construct the OGFC layer.

The Design 1 and Design 2 mixtures were tested for rutting characteristics at LTRC utilizing the Precision Machine and Welding version of the Hamburg Type Wheel Tester. The designs were also tested for drainage characteristics using the Karol-Warner falling head permeability device. The Instrotek CoreLok device was also utilized for Bulk Specific Gravity of mix,  $G_{mb}$ , measurements for the compacted specimens.

Table 1 indicates the aggregates and additives used for each trial design of the OGFC mixture. The PG76-22m, fiber, and anti-strip rate remained constant in both mix designs. The only variations between both mix designs were the incorporation of the #89 siliceous limestone and the actual percentages of aggregate blended to achieve a composite blend.

	Perce	entage	
Material	Design 1	Design 2	Source
#78			
Sandstone,			
FR I	84.0	67.2	Pine Bluff Sand & Gravel
#11			
Sandstone,			
FR I	9.3	7.4	Pine Bluff Sand & Gravel
#89			
Limestone,			
FR III		18.7	Vulcan Materials
PG76-22m	6.6	6.6	Marlin Asphalt
Fibers	0.1	0.1	Interfibe
Ad-Here LA	0.6 by Wt. of	0.6 by Wt. of	
2	AC	AC	Arr-Maz

Table 1Composition of mix design blends

Table 2 indicates the composite blend and mixture properties for the contractors' Design 1 and Design 2 composite blends. This table also presents the quality assurance (QA) data from the actual plant-produced mix during construction.

The LTRC gradation data indicated in table 2 are based on samples taken from the second truck during production. This data does not match the JMF or the District's QA data. The Design AC and Maximum Theoretical Specific Gravity,  $G_{nm}$ , however, correspond with the QA data. It is suspected that because the samples were acquired from the second truck, the hot mix plant had not had sufficient time to stabilize production. One hypothesis is that a purging of the bag house fines resulted in the finer gradation. The District's QA samples were acquired at a later time during production.

			Percent Pas	sing	
Sieve Size	Design 1	Design 2	OA Data	LTRC 2 <sup>nd</sup> Truck	Required Gradation
3/4" (19mm)	100	100	100	100	100
1/2" (12.5mm)	90	92	93	91	85 - 100
3/8" (9.5mm)	58	64	68	66	55 - 75
No. 4 (4.75mm)	14	16	21	26	10 - 25
No. 8 (2.36mm)	9	8	11	18	5 - 10
No. 16 (1.18mm)	7	6	9	16	
No. 30 (.600mm)	6	5	8	15	
No. 50 (.300mm)	5	4	7	14	
No. 100 (.150mm)	3.8	3.4	6	10	
No. 200 (.075mm)	2.8	2.3	3.9	6.1	2 - 4
Gmb	1.916	2.173			
G <sub>mm</sub>	2.374	2.368	2.381	2.389	
VCA	33.0	23.0			18
%Air Voids, AASHTO T166	19.3	8.2			
Gsh	2.558	2.604			
Gse	2.619	2.612			
Pha	0.9	0.8			
P <sub>be</sub>	5.9	6			
Permeability, ft/day	276	453			246
Permeability, ft/day LTRC					
Results	235	278			
Drain Down	0.08	0.08			0.3
Design AC	6.6	6.6	7.0	6.8	

Table 2Composite blends and mixture properties

Table 3 shows the rut measurements taken from the Precision Machine and Welding version of the Hamburg Type Wheel Tester (PMW Wheel Tracker). The PMW Wheel Tracker tests mixtures for rutting properties and moisture susceptibility. Samples pass if they attain no more than 6.0 mm of rutting after 20,000 passes of the PMW Wheel Tracker. Also, the PMW Wheel Tracker will stop the measurement process if the samples have attained more than 20.0 mm at 20,000 passes. Two samples each from mixture Design 1 and mixture Design 2 were subjected to these tests. The tests were conducted at 50 °C. Both sets of samples were tested at 56 passes per minute. Prior to testing, the samples were submerged under water for 90 minutes at the required testing temperature. The rut depths indicated in table 3 are an average of the center 5 of 11 measuring points taken from each sample. The distance between each measuring point is approximately 1.14 inches. As indicated in table 3, Design 1 did not pass the required criteria, nor did it perform as well as Design 2. Design 2 had an average rut measurement of 3.32 mm after 20,000 passes.

### Table 3

Number of	**De	sign 1	Desi	ign 2
Passes	Sample #1	Sample #2	Sample #1	Sample #2
14,981	21.98			
4591		23.38		
	**Design 1 faile	ed. Samples fell		
	apart during tes	sting or failed to		
	make the 20,00	0 pass criteria.		
20,000			3.50	3.14
			Averag	e = 3.32

#### Rut measurements (mm)

Figures 2 and 3 are the graphical illustrations of the deformation under loading vs. number of passes for mixture designs 1 and 2 as tested in the PMW Wheel Tracker.



Figure 2 Mixture Design 1, rut measurements (mm) vs. number of passes



Figure 3 Mixture Design 2, rut measurements (mm) vs. number of passes

Table 4 is a comparison of the percent air voids of the contractor's mix Design 1, mix Design 2, and roadway cores as measured by the CoreLok device versus the contractor's results that were determined by AASHTO T166. It is noted that there is a considerable variance between

results. The percent air voids measured by the CoreLok device is significantly greater than the results determined by AASHTO T166.

			Percen	t Air Void	S	
	Desi	gn 1	Desi	ign 2	Roadwa	y Cores
Contractor's Results	19	0.3	8	.2		
	Sample	Sample	Sample	Sample		
	1	2	1	2	Core #1	Core #2
LTRC (CoreLok)	19.4	27.6	14.3	13.8	16.7	17.6
Average	23	5.5	14	1.0	17	7.1
LTRC (T166)					11.2	10.6
Avenage					1(	) ()

Table 4CoreLok vs. AASHTO T166 air voids

Table 5 is a comparison of permeability results between the contractor's mix Design 1, mix Design 2, LTRC-prepared samples based on the contractor's mix design blends, and roadway cores. LTRC prepared two samples for each design and obtained the average coefficient of permeability. The falling head permeability (K-value) of the OGFC mixtures was calculated based on Darcy's Law. Each sample was tested twice and the average was reported. Design 2 resulted in a higher coefficient of permeability because of the decrease in material passing the No. 200 sieve.

## Table 5Coefficient of permeability (feet/day)

		<b>Coefficient of Permeability (feet/day)</b>					
	Design 1		Design 2		Roadway Cores		
	Sample	Sample	Sample	Sample	Sample		
	1	2	1	2	1	Sample 2	
LTRC Results	212.95	257.15	231.05	324.72	188.9	226.35	
Average	2.	35	2'	78	2	208	
<b>Contractor Results</b>	27	76	45	53			

### Profilograph

A smoothness specification was not required on this section of roadway because of the small quantity of material placed. The total length of the project paved was approximately 800 feet.

### Cost

This project was estimated at \$50,000 for the construction of the OGFC layer on the 0.157mile stretch of a 2-lane, 12-feet wide roadway, (approximately 2,210 square yards). The low bid by Diamond B. Construction Co., LLC was \$54,508.02. This bid included all items for project completion, i.e. striping, signs and barricades, mobilization, etc. The pay item for the OGFC was Item S-001, and it was paid for by the square yard at a unit price of \$19.64/sq.yd. The quantity used to date was 2,181.30 square yards, which equates to a cost of \$42,841. When this square yard cost value is converted to a price per ton of mix placed at a lift thickness of <sup>3</sup>/<sub>4</sub>" (~ 94.0 tons), it equates to approximately \$455/ton. Therefore, this project does not have a sufficient quantity to do a proper evaluation of cost comparisons between hot mix and a specialty mix such as the OGFC. Based on a material square yard cost method, we would estimate a budget value of \$3.00 to \$3.50 per square yard for 10-mile-long projects.

### COMMENTS

The high air void content and the open structure of this mix promoted the effective drainage of rainwater as intended. Since roadway drainage is enhanced, splash and spray behind vehicles should be reduced and ponding of water should be minimized, thus minimizing hydroplaning during wet weather. Future performance evaluations of the roadway will be performed.

### Recommendations

OGFC's are recommended for immediate use to further enhance safety by improving roadway surface drainage, minimizing hydroplaning, reducing splash/spray and roadway glare, improving wet weather visibility and visibility of traffic markings.

Prior to full implementation use of OGFC's, it is recommended to construct at least one OGFC project in each District to familiarize LADOTD and industry with the OGFC specifications and mix design procedure.

### References

- 1. Kandhal, P., and Mallick, R., "Open Graded Asphalt Friction Course: State of the Practice," National Center for Asphalt Technology Report No. 98-7, May 1998.
- Mallick, R., Kandal, P., Cooley, Jr., L.A, and Watson, D., "Design, Construction, and Performance of New-Generation Open-Graded Friction Courses," National Center for Asphalt Technology Report No. 2000-01, April 2000.
- 3. Paul, H. R., "Louisiana Frictional Surfaces," Presentation at the Transportation Research Board, Washington, D.C., January 1989.
- 4. "Louisiana Standard Specification for Roads and Bridges," State of Louisiana, Department of Transportation and Development, Baton Rouge, 1977 Edition.
- 5. "Louisiana Standard Specification for Roads and Bridges," State of Louisiana, Department of Transportation and Development, Baton Rouge, 2000 Edition
- Mallick, R., Kandal, P., Cooley, Jr., L.A, and Watson, D., "Design, Construction, and Performance of New-Generation Open-Graded Friction Courses," Asphalt Paving Technology 2000, Journal of the Association of Asphalt Paving Technologists, vol. 69:pp. 391 – 423.

### Appendix A

**Hotmix Plant and Roadway Pictures** 

# OGFC







# OGFC









### Appendix B

**Contract Specifications** 

BYLAD DATE 4-24-03

### STATE OF LOUISIANA DEPARTMENT OF TRANSPORTATION AND DEVELOPMENT



### CONTRACT

FOR

### STATE PROJECT NO. 009-02-0018

US 71 OPEN GRADED FRICTION COURSE (CM)

**ROUTE US 71** 

**GRANT PARISH** 

**DISTRICT 08** 

Page 1 of 9

#### LOUISIANA DEPARTMENT OF TRANSPORTATION AND DEVELOPMENT

#### CONTRACT

This agreement, is made and executed in TWO original copies, between the Louisiana Department of Transportation and Development acting through its Secretary, hereafter designated as the "Department", and Diamond B Construction Company, LLC, hereafter designated as the "Contractor."

The Department did advertise for, receive and accept a bid from the Contractor for work on a Department construction project identified as,

State Project No. 009-02-0018, US 71 OPEN GRADED FRICTION COURSE (CM), US 71, GRANT PARISH, consisting generally of OPEN GRADED FRICTION COURSE, PAINTED TRAFFIC STRIPING, REFLECTORIZED RAISED PAVEMENT MARKERS, AND RELATED WORK ON US 71 IN GRANT PARISH.

The Contractor's submission is evidenced by a copy of the "Construction Proposal Signature and Execution Form" incorporated herein as part of the Contract Documents defined hereafter.

In consideration of the agreements herein contained, to be performed by the parties hereto and of the payments hereafter agreed to be made, it is mutually agreed by both parties that:

#### CONTRACT DOCUMENTS

The contract consists of the "Contract Documents" including but not limited to the following:

- a. Agreement (This Instrument)
- b. Construction Proposal Signature and Execution Form
- c. Louisiana Standard Specifications for Road and Bridges, 2000 Edition (hereafter referred to as "2000 Standard Specifications")
- d. Project Construction Proposal (Notice to Contractors, Special Provisions, Supplemental Specifications, Schedule of Items) Plans e.;
- f. Plan revisions
- ZERO (0) Addenda made or issued prior to receipt of bids g.
- h. Payment, Performance and Retainage Bonds or Retainage Agreement

For these purposes, all of the provisions contained in the listed Contract Documents are incorporated herein by reference with the same force and effect as though said Contract Documents were herein set out in full. The Contract Documents are kept in the official file at the Department together with the acknowledgment of receipt correspondence signed by the Contractor.

Page 2 of 9

#### LOUISIANA DEPARTMENT OF TRANSPORTATION AND DEVELOPMENT

#### CONTRACT

#### INTENT OF CONTRACT

In accordance with the 2000 Standard Specifications and the Contract Documents, the Contractor agrees to the terms and requirements for the intent of the contract to provide all materials, equipment and labor and perform the work required to complete the project in a thorough and workmanlike manner, to the satisfaction of the appropriate official of the Department.

#### CONTRACT AMOUNT

The Contractor did submit as advertised with his (her) bid, a dollar value amount for each of the items designated in the construction proposal on the "Schedule of Items" and that the "Schedule of Items," attached hereto and incorporated herein as part of the Contract Documents, submitted by the contractor, establish that the total contract amount for this project is FIFTY-FOUR THOUSAND, FIVE HUNDRED FIGHT AND 02/100 dollars (\$54,508.02), as obtained by a summation of the product of the unit bid price submitted by the contractor for each item multiplied by the item quantity as estimated by the Department. The Contractor agrees to accept and the Department agrees to pay for the work at the prices stipulated in this contract in lawful money of the United States in a timely manner as set forth in the 2000 Standard Specifications.

#### CONTRACT TIME

The entire contract shall be completed in all details and ready for final acceptance within FIFTEEN (15) WORKING days. Performance of work on this contract shall begin on the date stipulated in the "Notice to Proceed" and shall be completed within the time specified in the Contract Documents, subject to such extensions as may be authorized.

#### ALTERATION OF CONTRACT

In accordance with the 2000 Standard Specifications and the Contract Documents, the Contractor agrees to the terms and requirements for alteration of the contract.

#### STIPULATED DAMAGES

Contractor agrees to the assessment of Stipulated Damages as provided in the Subsection 108.08 of the 2000 File Standard Specifications as amended by the Contract Documents.

#### DAMAGE CLAIMS

Contractor acknowledges that he/she has reviewed and understands Subsection 107.17 of the 2000 Standard Specifications and specifically agrees to be bound by the terms and conditions thereof.

#### JOINT EFFORT

This Agreement shall be deemed for all purposes prepared by the joint efforts of the parties hereto and shall not be construed against one party or the other as a result of the preparation, drafting, submittal or other event of negotiation, drafting or execution of the Agreement.

Page 3 of 9

۰. LOUISIANA DEPARTMENT OF TRANSPORTATION AND DEVELOPMENT CONTRACT This contract shall become effective on the date all parties hereto have signed the same. Diamond B Construction Company, LLC CONTRACTOR 72-0847970 (Federal Identification Number) au J. Su Sr. B Witness (Signature of Authorized Agent) 04-29-03 Witness (Date) BRYAN L BOSSIER SR., PRES11 (Typed or Printed Name and Title) LOUISIANA DEPARTMENT OF TRANSPORTATION AND DEVELOPMENT Sura Sauthan Witness By:// M Wayne Wm. Wyne Marchand, P.E. District Administrator, District 08 5-8-03 (Date) Page 4 of 9

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STATE PROJECT NO(S).	009-02-0018		
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THIS BID FOR THE CAPTIONED PROD	BCT IS SUBMITTED BY:		
Name of Principal (Individual, Firm, Corp.	ration Comptany, LLC		
If Joint Venture, Name of First Farmer		March 1	
12487		if Joint Venture, Name of Second Partner	
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#### LOUISIANA DEPARTMENT OF TRANSPORTATION AND DEVELOPMENT PAYMENT, PERFORMANCE, AND RETAINAGE BONDS Be it known that Diamond B Construction Commons LLC

t known that \_\_\_\_\_\_ Diamond B Construction Company, LLC\_\_\_\_\_, as Principal, Hartford Accident and Indemnity Company

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as Surety(ies), authorized to do business in other potential claimants, for all obligations incurred by the Principal under its contract for the construction of STATE PROJECT NO. 009-02-0018, US 71 OPEN GRADED FRICTION COURSE (CM), US 71, GRANT Parish, in the full contract amount of FIFTY-FOUR THOUSAND, FIVE HUNDRED EIGHT AND 02/100 DOLLARS (\$ 54,508.02). The obligations of the Principal and Surety under these payment, performance, and retainage bonds shall continue in full force and effect until all materials, equipment, and labor have been provided, AND all requirements contained in the contract, plans, and specifications have been completed in a timely, thorough, and workmanlike manner. The parties acknowledge that these bonds are given under the provisions and limitations contained in La. R.S. 48:250 et seq.

By this instrument(s), the Principal and Surety(ies) specifically bind themselves, their heirs, successors, and assigns, in solido, under the following bonds:

**PAYMENT BOND.** To the Louisiana Department of Transportation and Development and all "Claimants," as defined in La. R.S. 48:256.5 in the full contract amount of **FIFTY-FOUR THOUSAND**, **FIVE HUNDRED EIGHT AND 02/100 DOLLARS (\$54,508.02)**, in order to secure the full and timely claims under the project. The parties agree this bond is statutory in nature and governed by La. R.S. 48:256.3.

Claims pursuant to La. R.S. 48:256.5 shall be made to the Undersecretary, DOTD, Headquarters Administration Building, Rm 226, 1201 Capitol Access Road, Baton Rouge, LA 70802.

**PERFORMANCE BOND.** To the Louisiana Department of Transportation and Development in the full contract amount of **FIFTY-FOUR THOUSAND**, **FIVE HUNDRED EIGHT AND 02/100 DOLLARS** (\$54,508.02), in order to secure the full and faithful performance and timely completion of the project according to its plans and specifications, inclusive of overpayments to the contractor and stipulated damages as assessed.

**RETAINAGE BOND.** To the Louisiana Department of Transportation and Development in the full sum of **Ten percent (10%)** of the contract amount, in lieu of the sums required to be withheld from progress payments under the provisions of La. R.S. 48:256.1, inclusive of overpayments to the contractor and stipulated damages as assessed.

I, Principal, elect to	o exercise my option to have ten percent retainage withheld from all payments in I
of the above retains	age bond.
	p
	Principal
1	Principal
	8/01
	Form CC 164

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Lely	Witness BRYDAU Witness Hartford AC	ped or Printed Name and Ta cident and Indemnity Cor	PEESIDENT /CE Mpany	0
fut	Vitness By J. DAL Witness J. DAL HIBER P. O. F BATON	Attorney-in-Fact E GAULT, LIC. #96317 NULINSURANCE: GONST SOX 66068 I ROUGE, LA 70896-606	(Seal) STRUCTION	101 Jate
		Surety		
	Vitness By	Attorney-in-Fact	(Seal)	Date
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A copy of the co agency with resp Hartford Ac	intract and subsequent correspondence/ bect to the contract bonds should be dire (FOR SURETY 1) cident and Indemnity Company	communication from LA DO seted to: (FOR	TD or the contracting	8
100	le Kuitt	Bonding Age	ncy or Company Name	
/ Loc	Local Agent or Representative		Local Agent or Representative	
J. DALE G HIBERNIA P. O. BOX BATON R	AULTALIC #96317 INSURANCE CONSTRUCTION 66068 DUGE, LA 70896-6068		Address	
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P.O. Box 7618 Alexandria, LA 71306	21 E
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pay 90% of covered terrorism losse sureties/insurers until such time as ins occurs, Congress will determine the pro excess of \$100 billion.	s exceeding a statutorily-established deductible paid sured losses under the program reach \$100 billion. If acedures for, and the source of, any payments for losse
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ITEM S-001, OPEN GRADED FRICTION COURSE (03/03): This item consists of furnishing and constructing an Open Graded Friction Course in accordance with plan details, and these specifications.

General: This thin surface asphaltic concrete overlay material is intended to resist permanent deformation, reduce overspray by moving water freely through the layer and improve friction while providing a 10 year maintenance surface. This material shall be placed in thicknesses shown on the plans, otherwise the thickness shall be 3/4 inch (19 mm) minimum and 2.0 inch (50 mm) maximum.

Materials: Materials shall comply with the following:

(a) Asphalt Binder: The asphalt binder shall be PG 76-22m asphalt cement complying with Section 1002 and listed on QPL 41.

(b) Aggregate: Aggregates shall be 100% crushed stone, with a Friction Rating of I complying with the requirements of Subsection 1003.06(b), except that aggregate gradation shall be as follows:

Aggregate Gradation				
US Sieve (Metric)	Percent Passing			
3/4 inch (19 mm)	100			
1/2 inch (12.5 mm)	85-100			
3/8 inch (9.5 mm)	55-75			
No. 4 (4.75 mm)	10-25			
No. 8 (2.36 mm)	5-10			
No. 200 (75 um)	2.4			

(c) Fiber: Cellulose fiber or mineral fiber when required shall comply with Section 508 and will be required to assure protection against draindown.

(d) Anti-strip: Anti-strip shall comply with Subsection 1002.02 and be applied according to Section 502.

(e) Tack Coat: The tack coat shall conform to Section 504 and shall be applied at a minimum rate of 0.07 gallons per square yard (0.32 L/sq m).

Equipment: Equipment shall conform to Section 503.

Design Procedure: The contractor shall provide the required mixture using the following design procedures:

1. Select three trial blends of aggregate within the specification bands above.

 Determine dry-rodded voids in coarse aggregate, plus No. 4 (4.75 mm) sieve, of the coarse aggregate fraction (VCA<sub>DR</sub>)

 Add between 6 percent to 6.5 percent asphalt to each trial blend and compact to 50 gyrations of a Superpave gyratory compactor.

(Note: At this stage of design, fiber should be added at manufacturer's recommended rate. Fibers are required when draindown is observed, typical rates are 0.2 percent to 0.5 percent.)

4. Determine the voids in the coarse aggregate, VCA, for each compacted mix.

5. VCA must be equal to or less than the  $VCA_{DR}$  (this indicates stone on stone contact).

Select the trial gradation that produces stone on stone contact and a minimum 22 percent VCA.  Using selected design from Step 6, prepare two additional mixtures using 0.5 percent and 1.0 percent additional asphalt content and compact using 50 gyrations of the Superpave gyratory compactor.

 Conduct draindown test in accordance with Section 508 on the loose mix at a temperature 60°F (15°C) higher than normal mixing temperatures. (A maximum draindown of 0.3 percent is required.)

 Conduct laboratory permeability test described in ASTM PS 129, 246 feet/day (75 m/day) minimum is desired.

 Report each step of the procedure. The report must show that the selected design meets draindown, VCA of 18 percent minimum and a minimum of 246 feet/day (75 m/day) permeability.

11. Perform boil test and Lottman test. The Lottman moisture sensitivity test, in accordance with AASHTO T 283, is modified to require 5 freeze/thaw cycles and the retained tensile strength, TSR, shall be 80 percent. A minimum of 90 percent coating is required for the boil test.

12. District Lab Engineer shall review and approve the design, verifying the aggregate gravities.

<u>Plant Validation and Quality Assurance Test:</u> The validation lot is defined as the first four hours of production. Validation requires that the mixtures meet the minimum design criteria excluding Lottman and shall be based on the average of a minimum of two samples. Subsequent validation trials shall be limited to 500 tons (500 Mg) per day.

One set of plant Lottman tests shall be made during validation and reported within one week of production for verification.

A production lot is defined as the material produced in one day.

Two random acceptance samples shall be taken each day and the average shall be reported and shall meet the following:

Gmm, maximum specific gravity ±0.020 from validation target;

VCA, 18 percent minimum; after 50 gyrations of a Superpave gyratory compactor.

Draindown; 0.3 percent;

Boil Test; 90 percent coated

Percent AC, meter; (±0.2 percent from design target)

Permeability; Validation only or when requested by the engineer.

Acceptance pay will be based on the percent deviations from the job mix formula tolerances for the lowest of the pay sieves listed below.

US Sieve (Metric)	100% pay	95% pay	90% pay	or remove
No. 4 (4.75 mm)	±4%;	±4.1 to 6.0%	±6.1 to 8.0%	>8.0%
No. 8 (2.36 mm)	±3%	±3.1 to 5.0%	±5.1 to 7.0%	>7.0%

Weather Limitations: Weather limits shall comply with Section 502 except that the surface temperature shall be a minimum of 60°F (15°C) and air temperatures must be 60°F (15°C) and rising.  Using selected design from Step 6, prepare two additional mixtures using 0.5 percent and 1.0 percent additional asphalt content and compact using 50 gyrations of the Superpave gyratory compactor.

 Conduct draindown test in accordance with Section 508 on the loose mix at a temperature 60°F (15°C) higher than normal mixing temperatures. (A maximum draindown of 0.3 percent is required.)

 Conduct laboratory permeability test described in ASTM PS 129, 246 feet/day (75 m/day) minimum is desired.

 Report each step of the procedure. The report must show that the selected design meets draindown, VCA of 18 percent minimum and a minimum of 246 feet/day (75 m/day) permeability.

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Draindown; 0.3 percent;

Boil Test; 90 percent coated

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No. 8 (2.36 mm)	±3%	±3.1 to 5.0%	±5.1 to 7.0%	>7.0%

Weather Limitations: Weather limits shall comply with Section 502 except that the surface temperature shall be a minimum of 60°F (15°C) and air temperatures must be 60°F (15°C) and rising. <u>Placement and Compaction</u>: Mixture shall be placed to plan thicknesses and compacted immediately after placement without excessive breakage of aggregate. Two or three passes of a vibratory roller is typical. Newly constructed sections shall be protected until it has cooled enough to develop sufficient strength to hold traffic.

Roadway Inspection and Smoothness Requirements: The inspector will record the average paver screed height settings every hour and will also report the yield hourly by adding the tons reported on the weigh tickets and dividing by the area placed. Smoothness will be measured before construction starts and again after each day of construction. The smoothness measurement after construction shall not exceed the measured smoothness before construction.

<u>Measurement:</u> Open Graded Friction Course will be measured per square yard (sq m). The quantities for payment will be the design quantities specified in the plans, based on horizontal dimensions, and adjustments thereto.

<u>Payment:</u> Payment for Open Graded Friction Course will be made on the accepted quantity at the contract unit price per square yard (sq m) subject to the acceptance payment adjustments contained herein.

Payment will be made under:

Item S-001, Open Graded Friction Course, per square yard (sq m).



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	State Froject No.	Parish	Sheet N
	009-02-0018	Grant	2
DESIGN CONCEPT			
PROJECT LOCATION AND LIMITS This project is located on US 71 just south of Rock Hill. It begins at station 18+45, which is local along US 71 for 0.157 mile. The project ends at station 26+75.	ted 615' north of mile marker 8	5. It proceeds north	hward
EXISTING ROADWAY The existing roadway is a 24 ft, wide travelway with paved shoulders. The existing surfacin Shoulders are 10° width asphaltic concrete over a soil cement base.	ng on the travelway is asphaltic	concrete over stab	ilized base.
PROPOSED ROADWAY Plan intent is to place an open graded friction course on the existing travelway.			
Additional work will consist of replacing the existing roadway markings (painted) and refle	ctorized markers.		
			1
			1150

		State Project No.	Parish	Sheet N
	ODD IN IS IS IS	009-02-0018	Grant	3
0	GENERAL NOTES			Sheet 1 of
21	The total funding for this contract is fixed, and cost cannot overrun. The Engine	er may adjust quantities of such back		
	The country of the co	or may adjust quantities of work herein	to remain on budg	et.
4)	THEM 713-01, TEMPORARY SIGNS AND BARRICADES:			
- 53	the Do Not Pass" and "Pass With Care" signs will be erected at the same time a	as the advance warning signs for the pro-	viect.	
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Stat STA DESCRIPTION Length Width (ft.) (Squar 28+45 26+75 Travelway 830.0 24.0 2 88-40 2				009-02-0018	Grant	4
STA       DESCRIPTION       Length       Width (ft.)       Yards         18+45       26+75       Travelway       830.0       24.0       2         18+45       26+75       Travelway       18       26       2         18+45       26+75       Travelway       18       26       18         18+45       26+75       18       22       18       18         18+45       26+75       27       22       22       22         18+45       26+75       28       28       28       28       28         18+45       26+75 <td< th=""><th>uria</th><th>acin</th><th>B</th><th></th><th></th><th>Sheet 1 of</th></td<>	uria	acin	B			Sheet 1 of
STA         DESCRIPTION         Length (ft)         Width (ft.)         yards           18+45         26+75         Travelway         830.0         24.0         2           -         -         -         -         -         -         -           -	2015				ITEM S-001 Open Graded Course	Friction
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	Nº O	State Project No.	Parish	Sheet No.
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	Summary Of Estimated Quanti	ties	2	Sheet Lof
ITEM NO.	ITEM NAME			The second second
713-01	Temporary Signs and Barricades	UNIT	QUANTITY	
	and the second	Lump	Lump	
727-01	Mobilization	1		
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/31-02	Reflectorized Raised Pavement Markers	Fa	24-	11.12
737-01-A	Painted Teaffic Section (P. 1991)		-1-1-	+ +7
737-02-A	Painted Traffic Striping (Solid Line)(4" Width)	Mi.	0.519	2.432
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S-001	Open Graded Friction Course			
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### Appendix C

### Job Mix Formula

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rushed Aggregate	634	#89 LS	AA50	Vulcan	20	20 18.7	2.671	3
Screenings	643	#11 55	AB13	Pinebluff	8	8 7.4	2.555	
Coarse Sand					+ +			
Fine Sand					+ +			
Other					+ +			
Lime					1-1			
Ant-Strip(AS)		Adhere La2	5730	ARRMAZ		0.6	JMF LIMAS	0.5 - 0.7
ecialmed Materials							Same	
Recisimed AC						0.1	Fiber AC	
Other	Fiber		-	Interfibe				
Ce	intractor's Result	MARS	MALL TEST PRO	OPERTIES		Departmen	nt Validation	JMF
	Average	1 2	3	4		Ave	arage	Limits
GMB	2.173				-			_
GMM	2.368		-10 M		-	10 10		OFF IF
VCA	23.0				-	17		18 % Min
% Voids	8,2				-	-		
TO VMA					-			
Drain down	0.05				•			3 may
Dow 0.1mm/1/100	0.08				-	100		5 max
				and the second se	-			The second second
RECOMMENDED	FORMULA	LOOSE MIX RESULT	TS ALCO	JMF	1.1	DEPAR	TMENT RESUL	.15
in num			AVG.	Limis		Adjustment East	stor	
50 2	100			100	1	Tensie Str Cont	mikPa/PSII	
7 5 1 1/2	100			100		Tensile Str Ret	in (TSR) %	
25 1	100			100				-
19 3/4	100			100		Ross Count, %/	T 195)	100
2.5 1/2	92			88-96	- 1	%Ret. Asph. Co.	uting (TR 317	100
.5 3/8	64			60-68	1			Super-
75 No.4	16			12-20		Effective AC,%		6.0
36 No.8	8			5-10	1	Absorbed AC,%		0.8
18 No.16	6			-	- b			
00 No.30	5				1	Natural Sanda		
NO.50			( <del>)</del>		1	Sand Equivalent		
5 No 200	23			2.4				
C Extract	6.7			6.3 . 71		Moisture Conter	t of Mix %	Selling of
Crushed	100			100		Opt. Maine Tem	0.C (F)	300
* Temp C (F)	300			275-325		Opt. Compaction	Temp.C (F)	275
AC (Met/Sc)	6.6			6.5 . 6.7	- x P	5 3 CA 192 - 5 CO - 18	100000000	
S (Meter)	0.6		10	0.5 - 0.7	1	Abson Recov.,Pa	I.S (Polses)	
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### Appendix D

Film Thickness Calculation

#### OGFC Film Thickness Calculations Based on Surface Area

Sieve Size, mm (in)	Percent Passing	**Surface Area Factor (m <sup>2</sup> /kg)	Surface Area (m <sup>2</sup> /kg)
37.5 (1.5)	100	0.41	0.41000
25 (1)	100		
19 (3/4)	100		
12.5 (1/2)	92		
9.5 (3/8)	64		
4.75 (No. 4)	16	0.41	0.06560
2.36 (No. 8)	8	0.82	0.06560
1.18 (No. 16)	6	1.64	0.09840
.0.6 (No. 30)	5	2.87	0.14350
0.3 (No. 50)	4	6.14	0.24560
0.15 (No. 100)	3.4	12.29	0.41786
0.075 (No. 200)	2.3	32.77	0.75371
Total S	Surface Ar	ea	2.20027

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% VMA = 22.1	G <sub>se</sub> = 2.612	G <sub>mb</sub> = 2.173
% VTM = 8.2	P <sub>b</sub> = 6.7%	G <sub>mm</sub> = 2.368
P., = 6.6%	Ph. = 0.1%	G., = 2.604

Volume of Asphalt Binder	13.9	%
Weight of Asphalt Binder	143.17	kg
Weight of Aggregate	2026.072	kg
Weight of Asphalt per kilogram of aggregate	0.070664	

Actual Asphalt Film Thickness

31.2 microns

### Appendix E

LADOTD & Contractor's Project Personnel Comments

### LADOTD & Contractor's Project Personnel Comments

#### Nicholas F. Verret, Jr., LADOTD District 08 Design, Water Res. & Dev. Engineer

"It is obvious that this material is functioning as intended, since you can see water bleeding through it onto the shoulder after a rain......"

### Cephas Bowie, Jr., LADOTD District 08 Laboratory Engineer

"The mix design and application at this particular site on US 71 has eliminated the potential for hydroplaning. The mix provided drainage from the travel lanes and has performed well under traffic. This site is in a curve on a hill which allows the water from the travel lanes to either flow to the shoulders or down the travel lane edges during a heavy rain, however, it would be better to lay the OGFC on the travel lanes and shoulders or provide alternate drainage through the shoulders (I realize that our project was a CM job and the monies were limited). This project is performing very well."

### Mark Lacroix, Quality Control Manager, Diamond B. Construction Co., LLC

"1) Allow skid 2 and 3 aggregate in the mix. Follow current HM specs which allow 30% skid 1 and 50% skid 2 by weight of total mix. This would allow contractors to utilize commonly inventoried materials while not impacting safety.

2) Eliminate Lottman sensitivity test and evaluate agg/AC compatibility by performing a boil test. The coarse aggregate structure of the mix makes it difficult for the sample to hold together during the thaw cycle. The current spec called for 5 cycles. On the project we did, this was waived.

3) Run plant production at least 150 tons before sampling to allow the plant bag house to purge. The small project that we did showed a finer gradation than was designed as a result of this. Even so the material is functioning as intended."