

Caminada Bridge (LA): Coastal Engineering Analysis & Numerical Modeling in Support for Design *(Hurricane Wind and Wave Action)*

Hossein Ghara, P.E., M.B.A., State of Louisiana DOT&D

Artur Wagner, P.E., State of Louisiana DOT&D

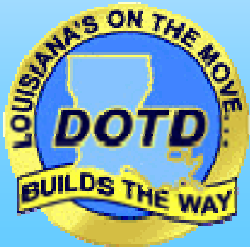
Jenny Fu, P.E., State of Louisiana DOT&D

Vladimir Shepsis, P.E., PhD Coast & Harbor Engineering, Inc.

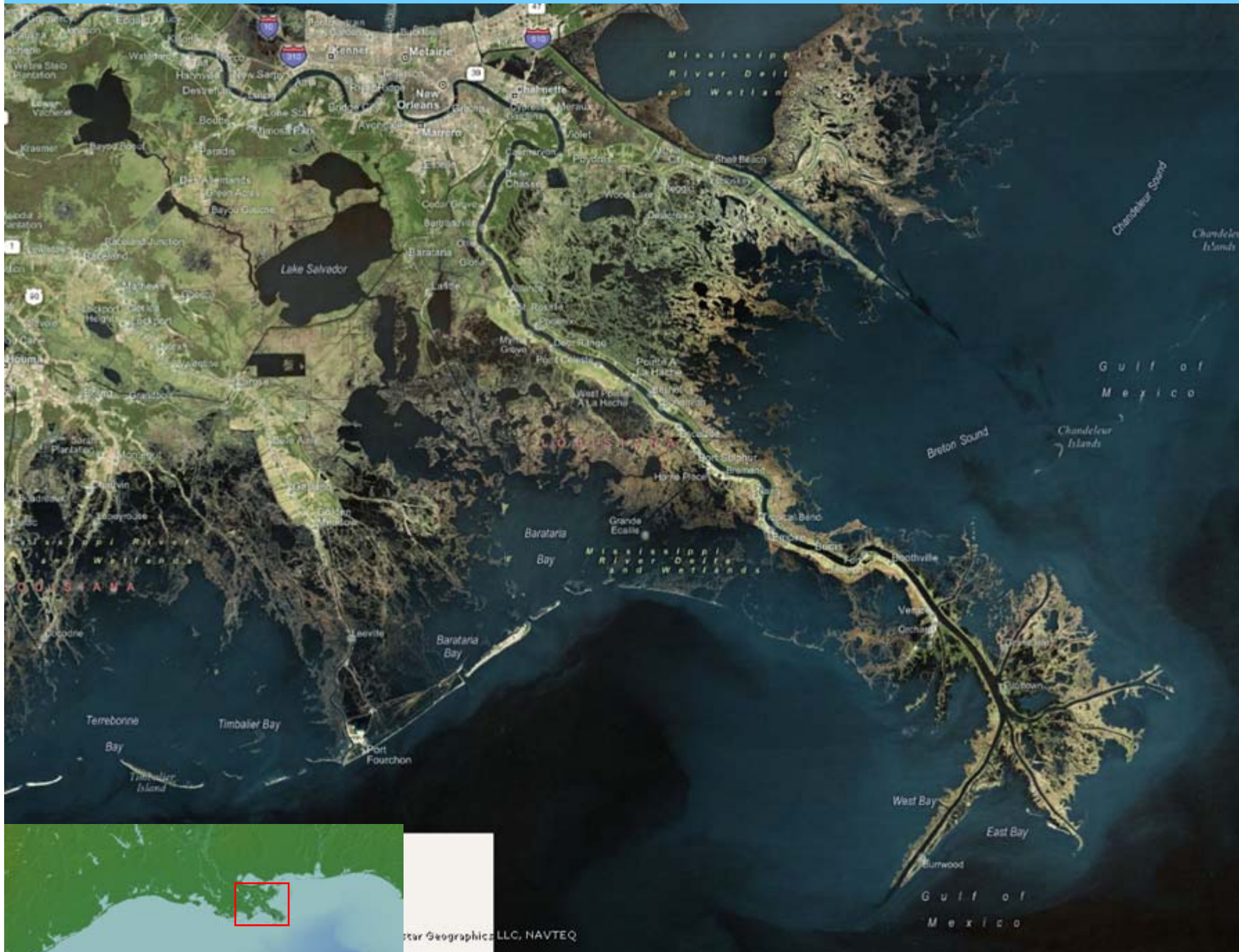
Matteo Tirindelli, PhD Coast and Harbor Engineering, Inc.

2008 LTRC Bridge Structures Conference

New Orleans
February 21, 2008



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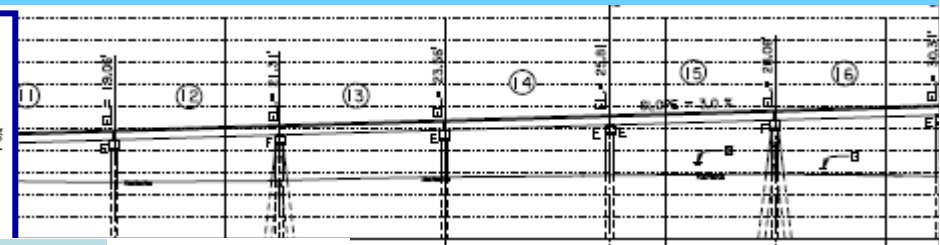
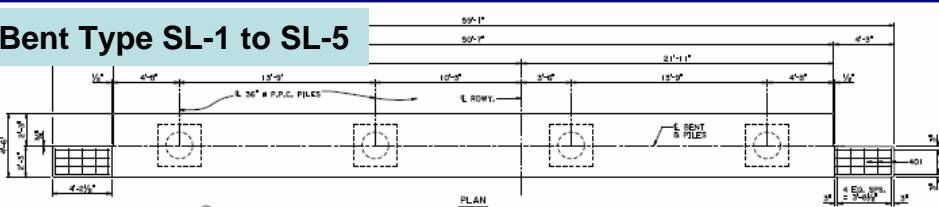
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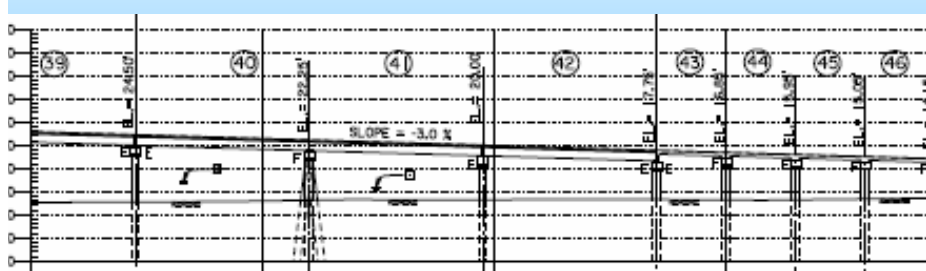
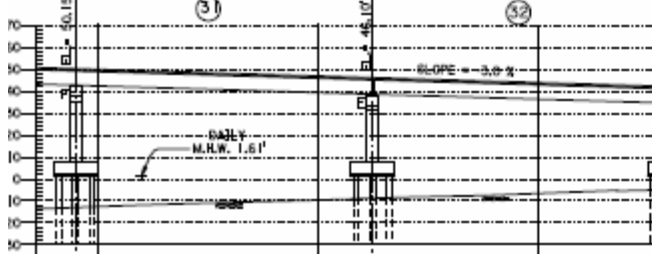
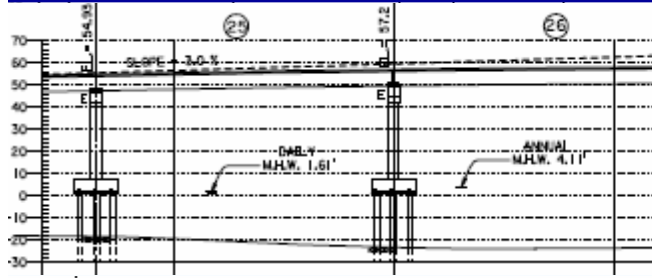
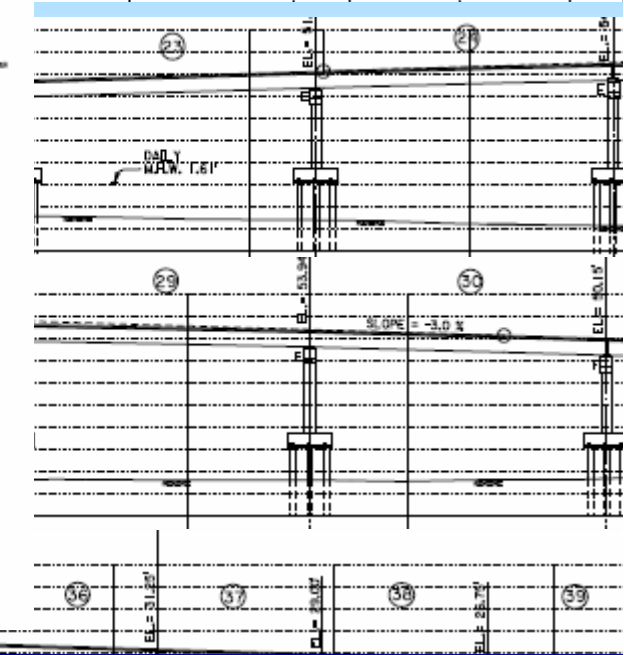
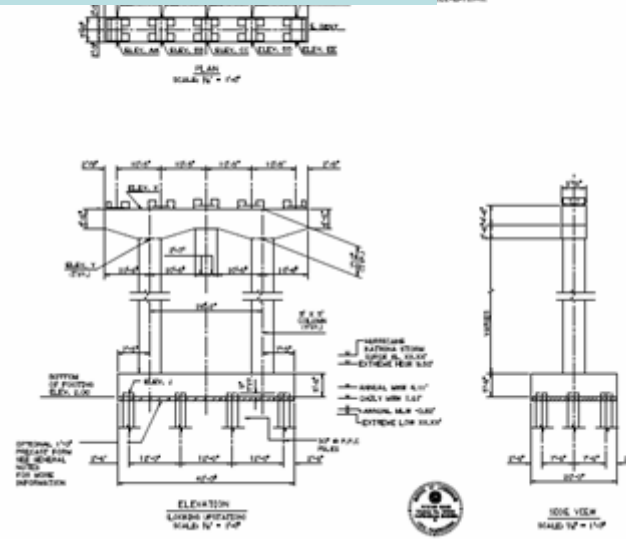
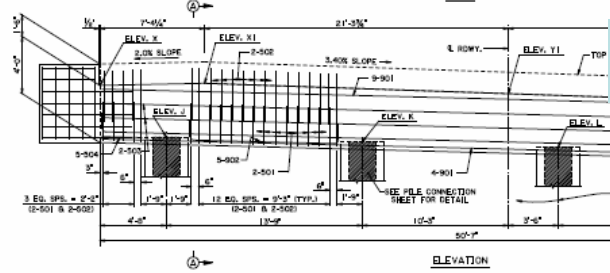
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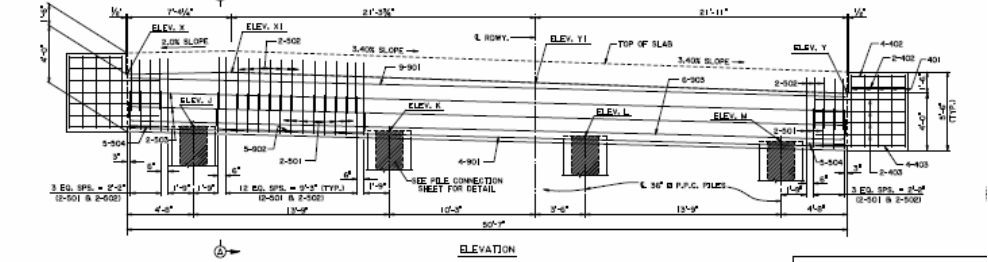
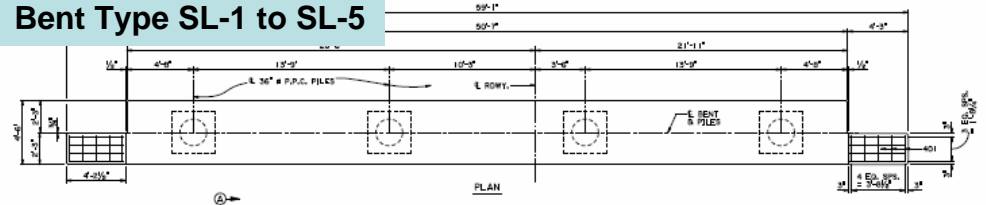
Bent Type SL-1 to SL-5



Bent Type BTC-1 to BTC-4



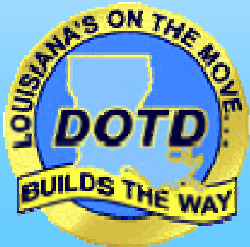
Bent Type SL-1 to SL-5



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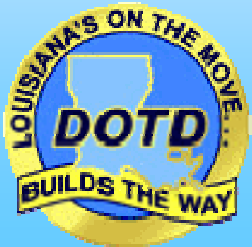
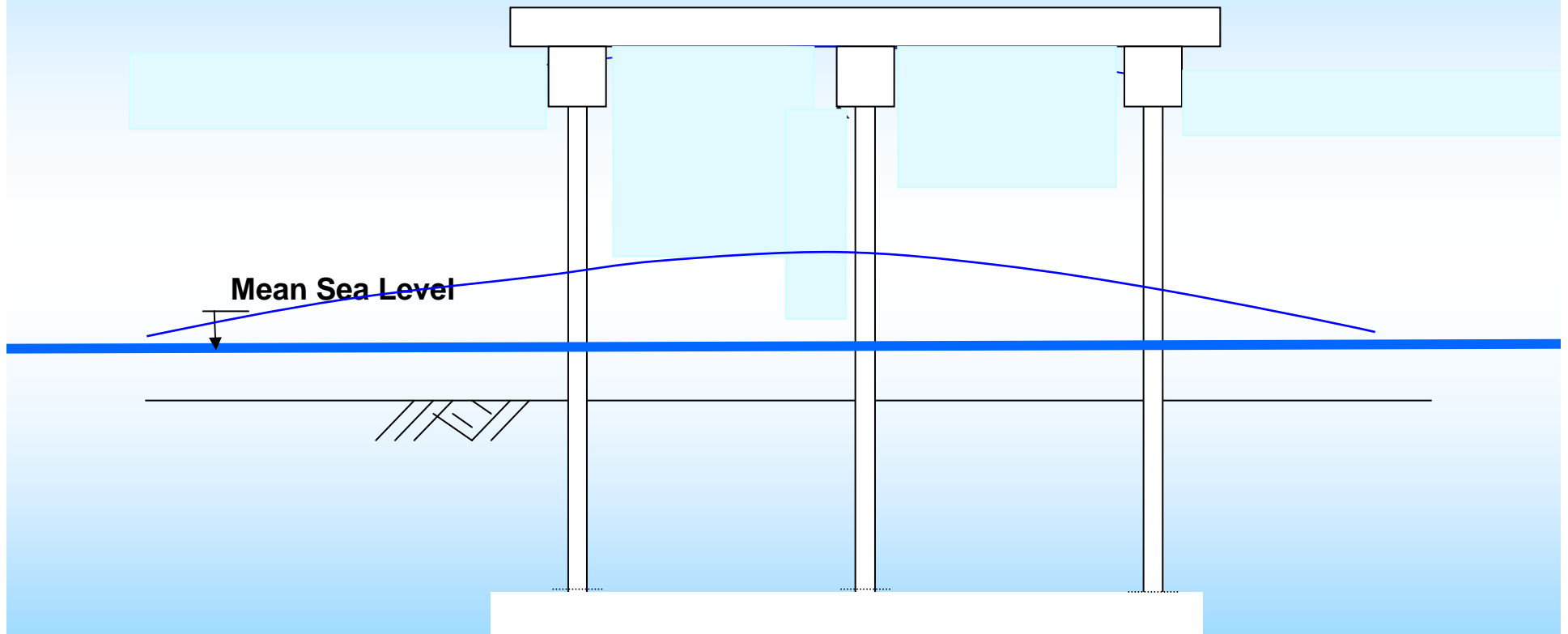
Objective

Provide reliable vertical and horizontal forces on bridge elements during design hurricane storm event. Bridge elements to compute forces include deck slabs, girders, piles, pile caps, columns, and footings



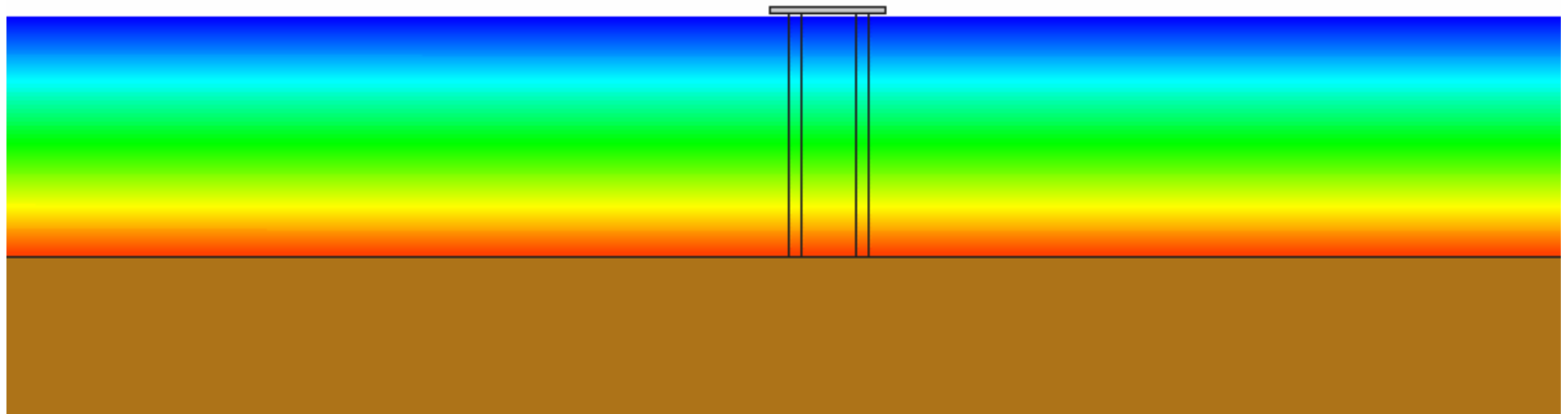
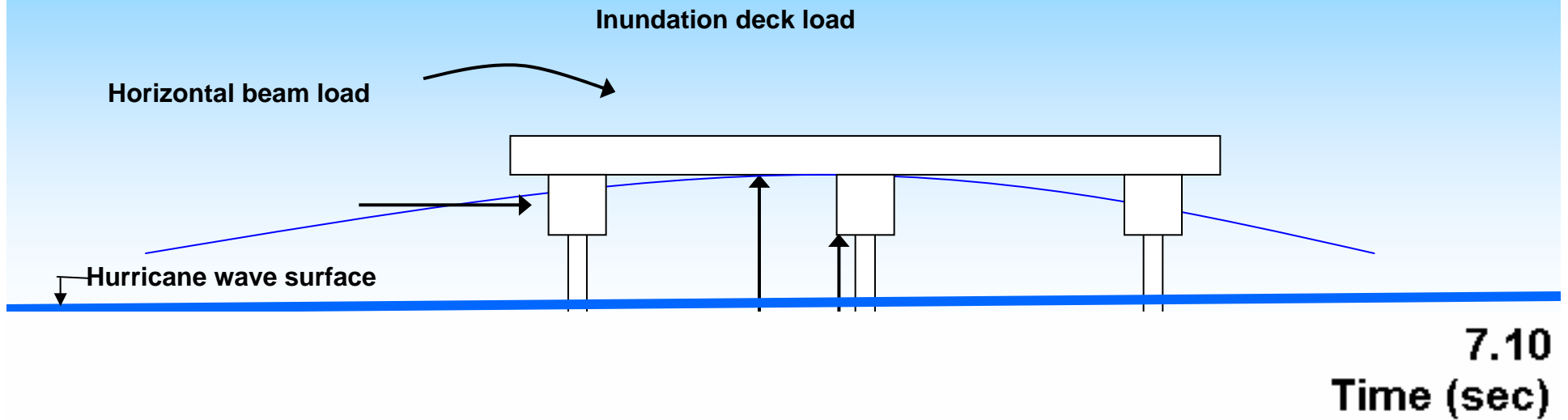
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Wave Loads Diagram



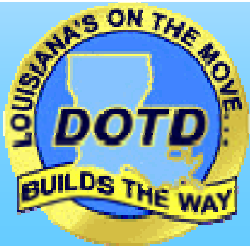
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Wave Loads Diagram



Scope

- **Developing the design hurricane event parameters (water surface elevation and waves);**
- **Computing vertical and horizontal forces using different methodologies, including 90% AASHTO Guide; and**
- **Comparing the results of computations and selecting the recommended design forces.**

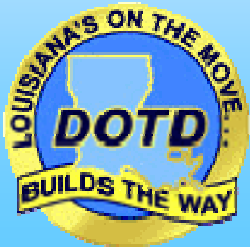


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Hurricane Event Return Periods

Major Factors

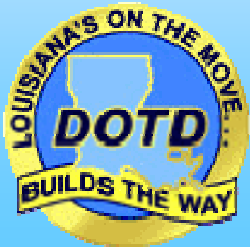
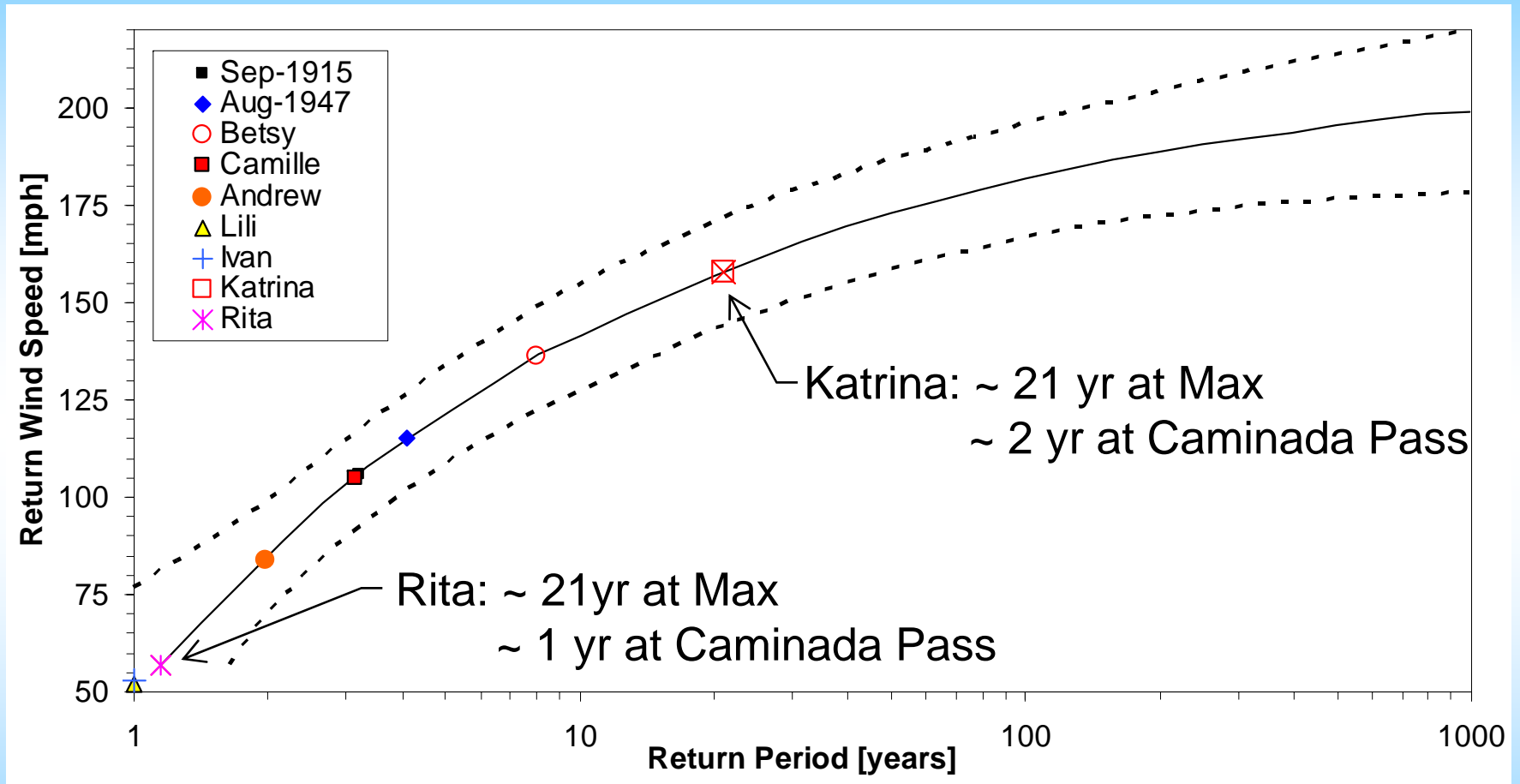
- Wind Speed
- Surge Elevation
- Minimum Pressure
- Size of Hurricane
- Storm Track
- Storm Speed



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Winds Return Periods (Recommended)

Caminada Pass (*based on Jagger and Elsner, 2006*)



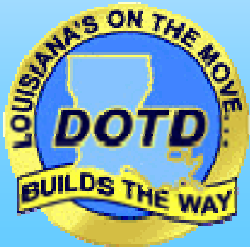
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CHE Approach

Hurricane Event Return Periods

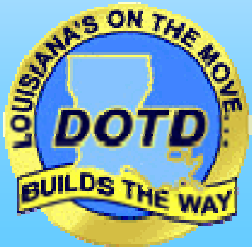
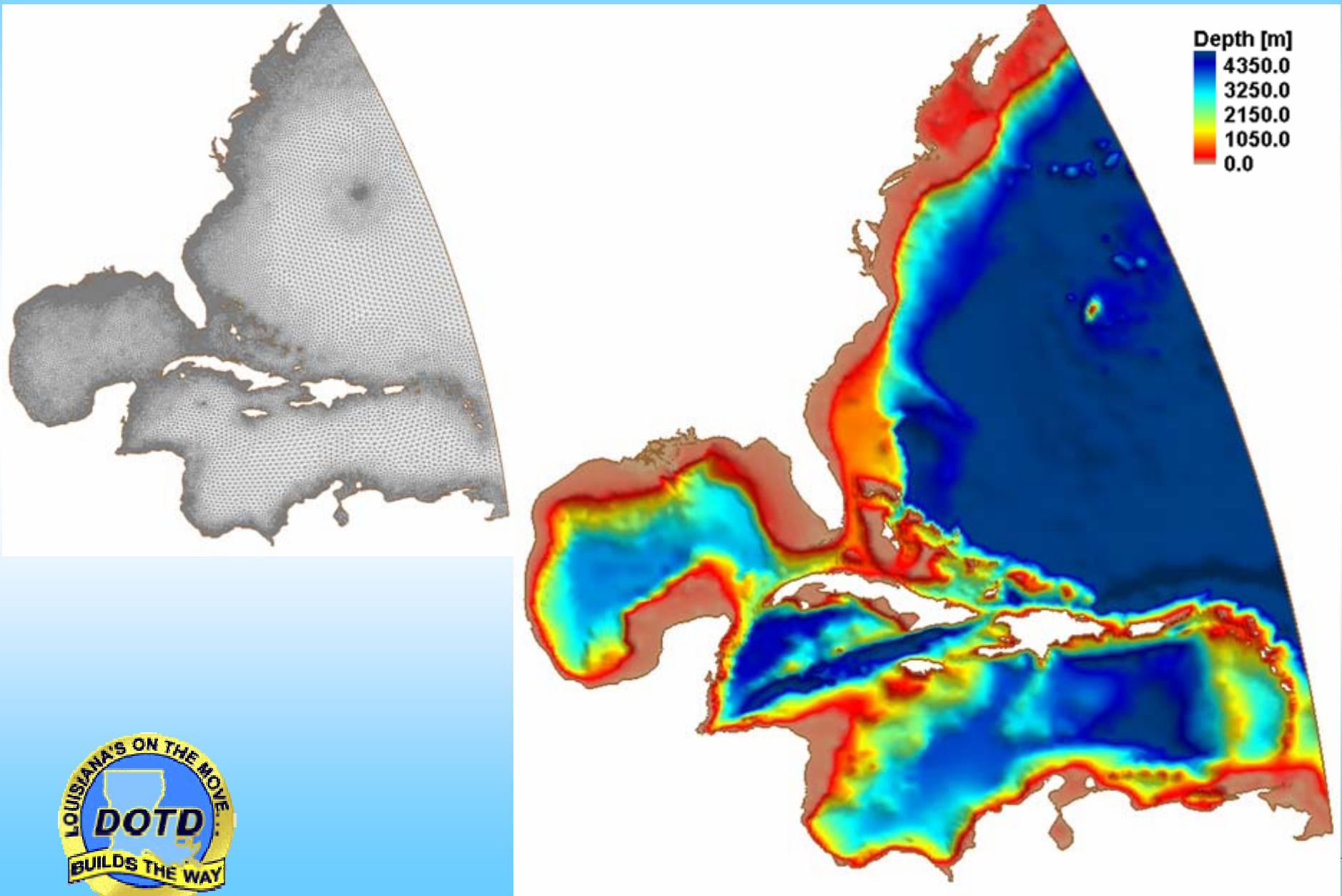
Major Factors

- Wind Speed
- Surge Elevation

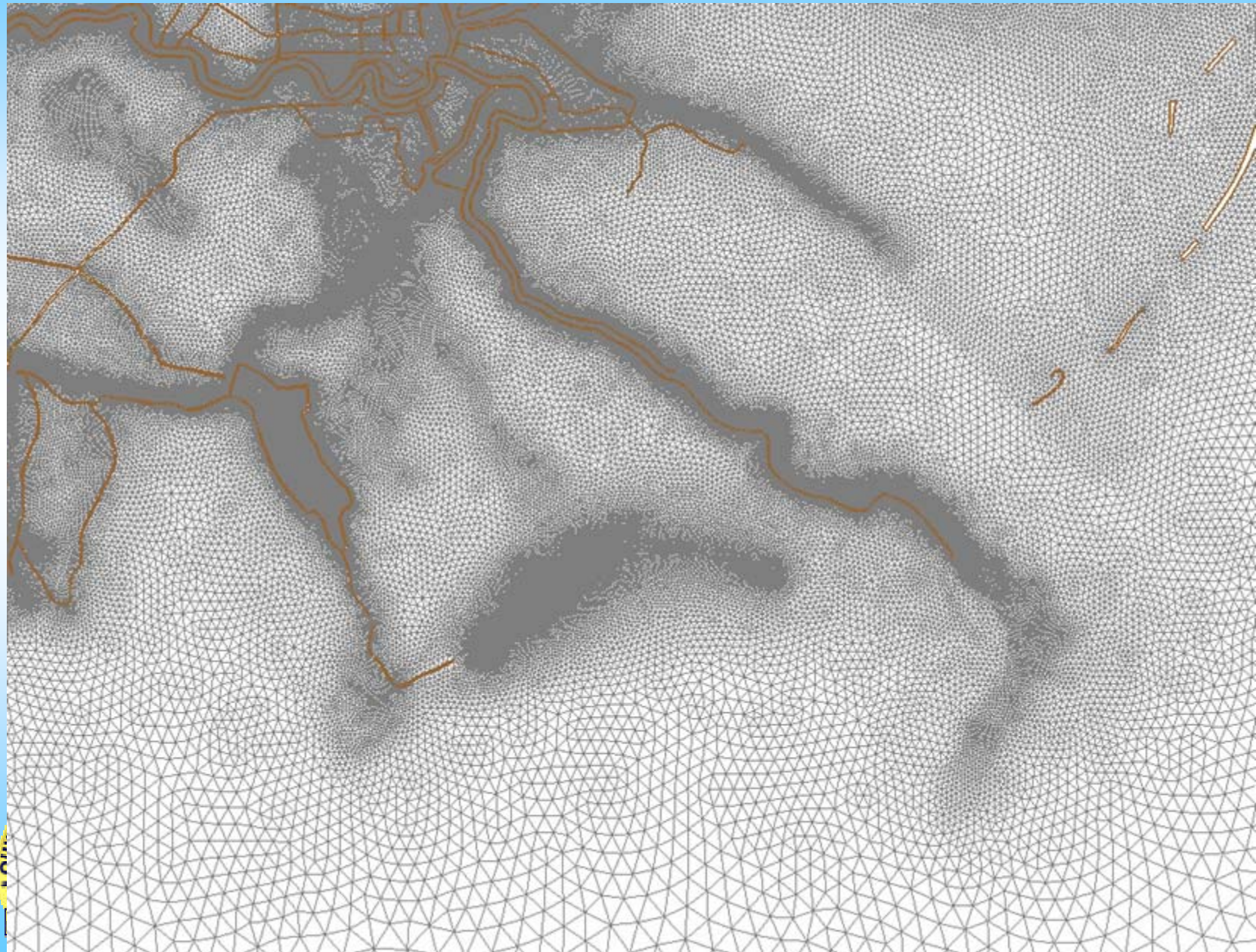


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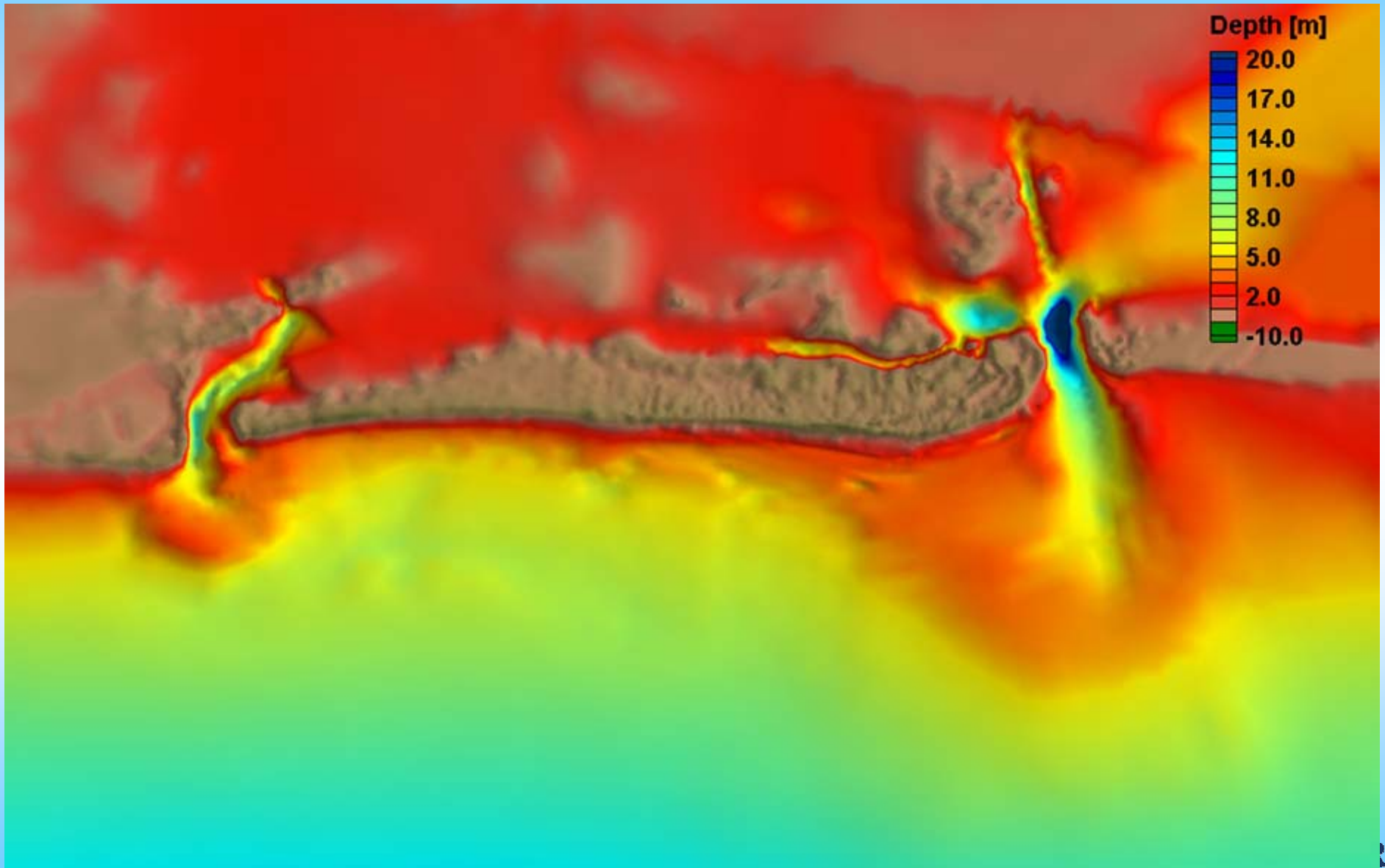
Surge Modeling



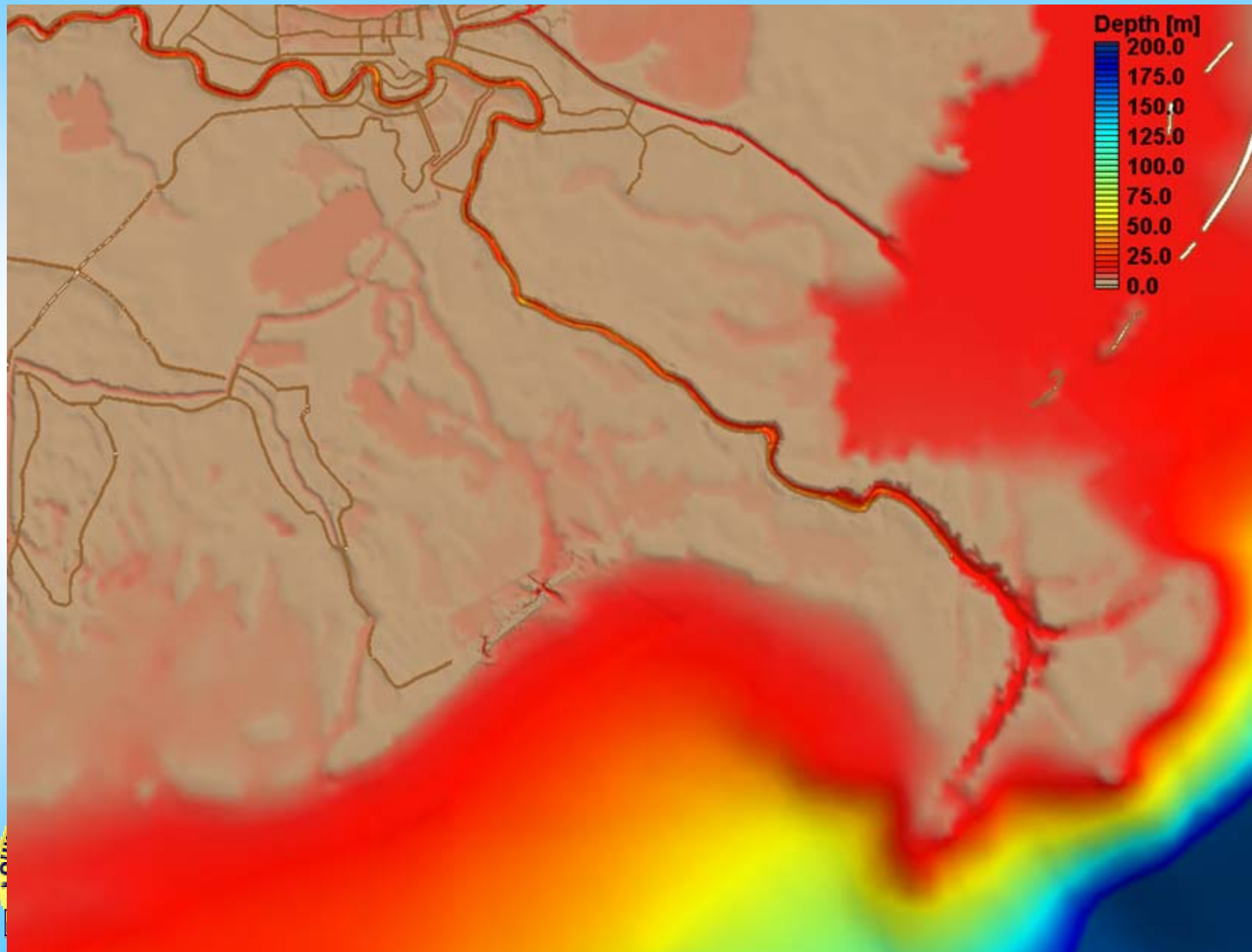
Surge Modeling (Modeling Grid)



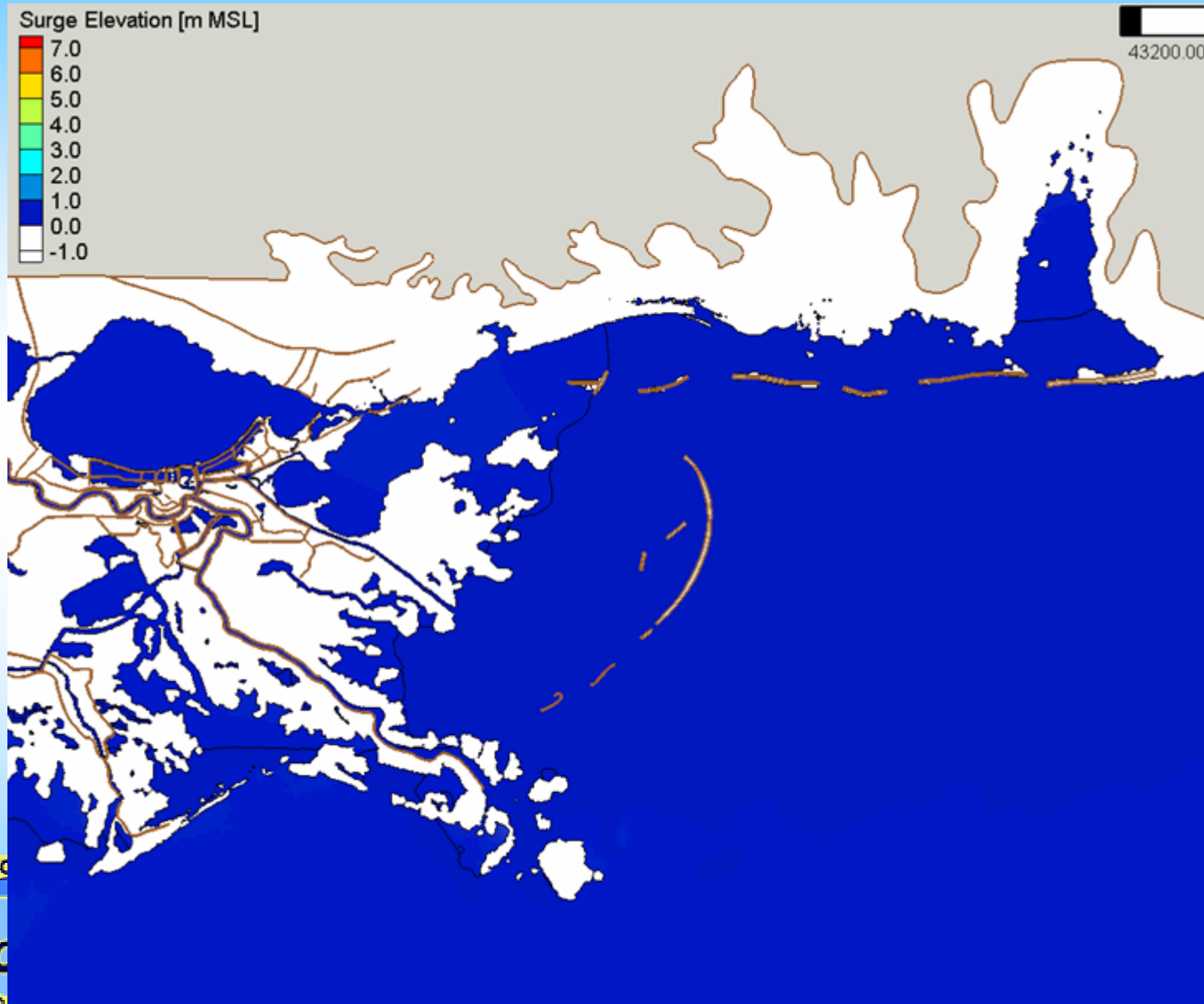
Surge Modeling



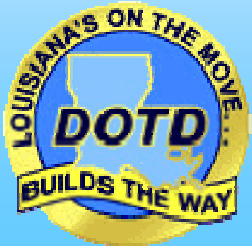
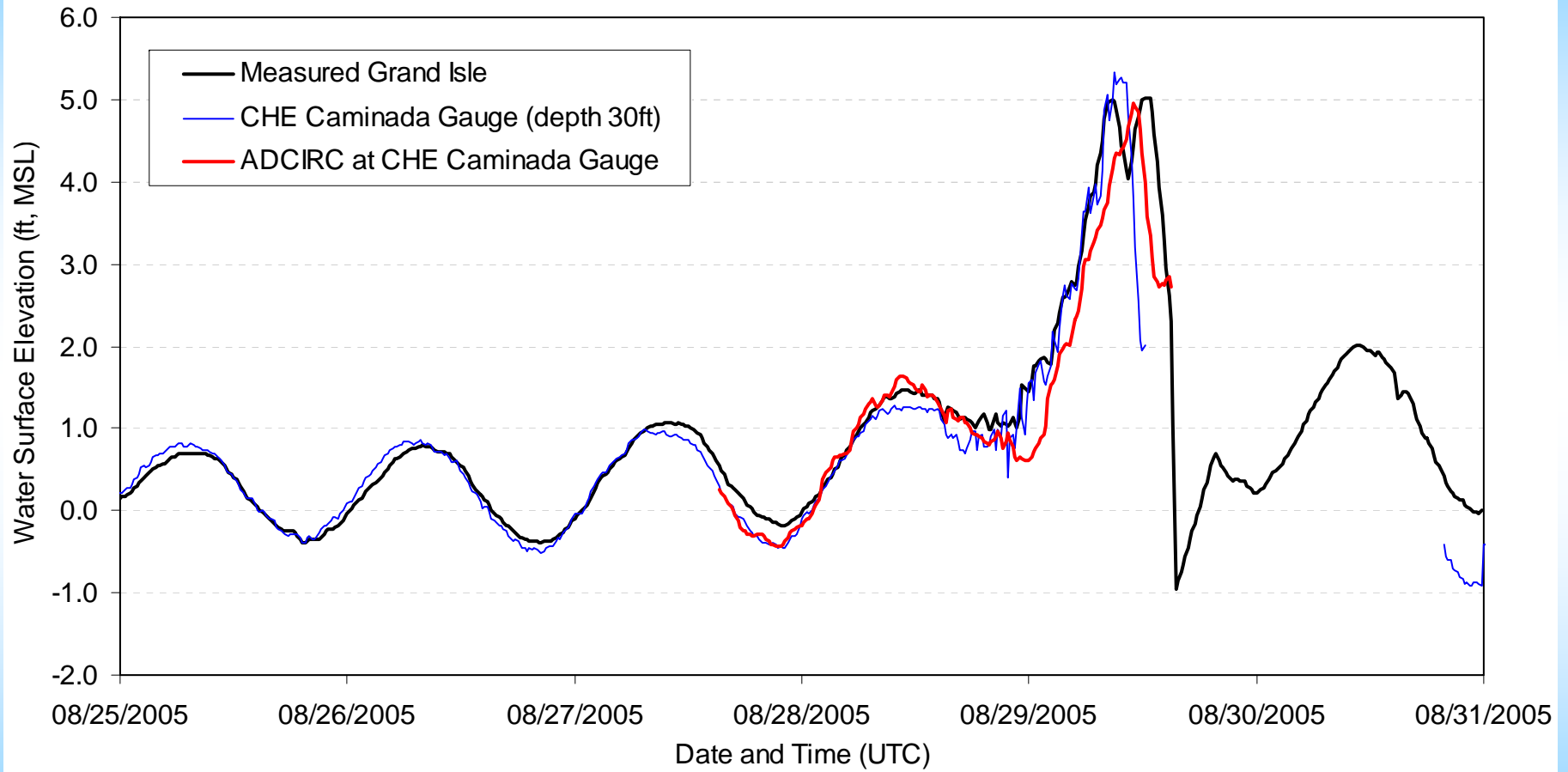
Surge Modeling



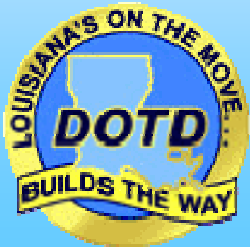
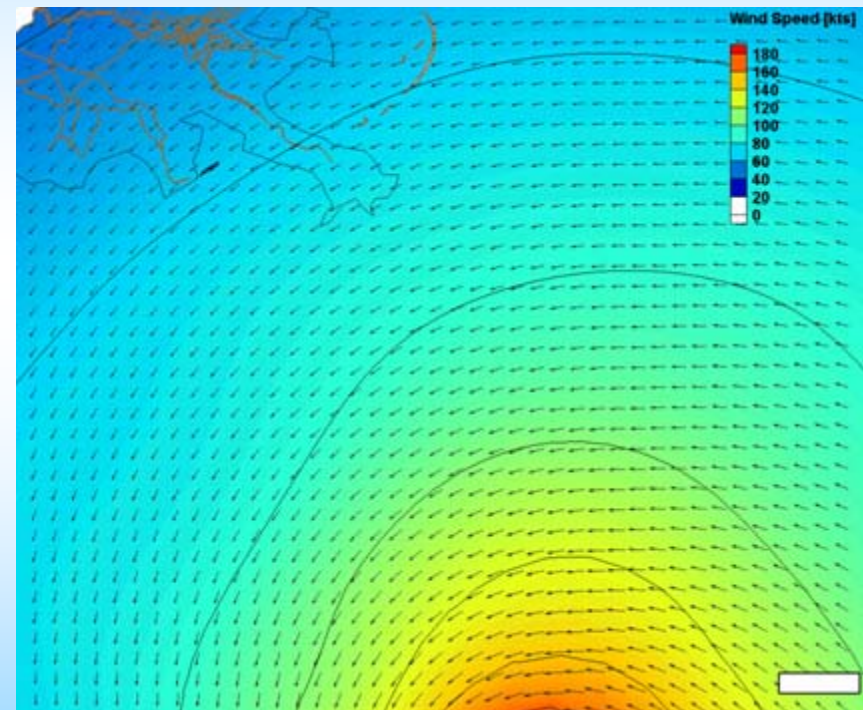
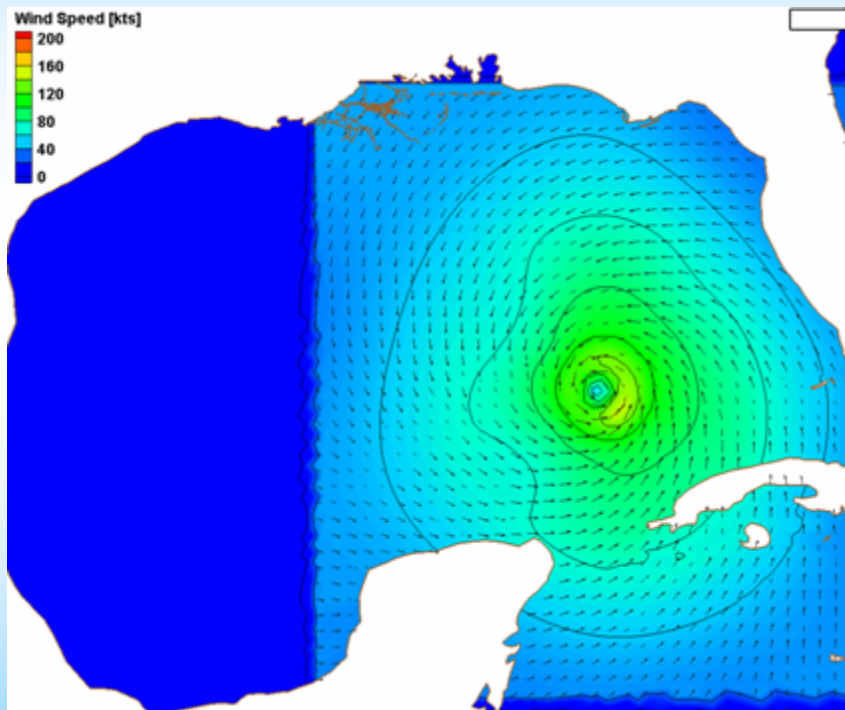
Modeling Real Katrina



Katrina Surge Model Validation

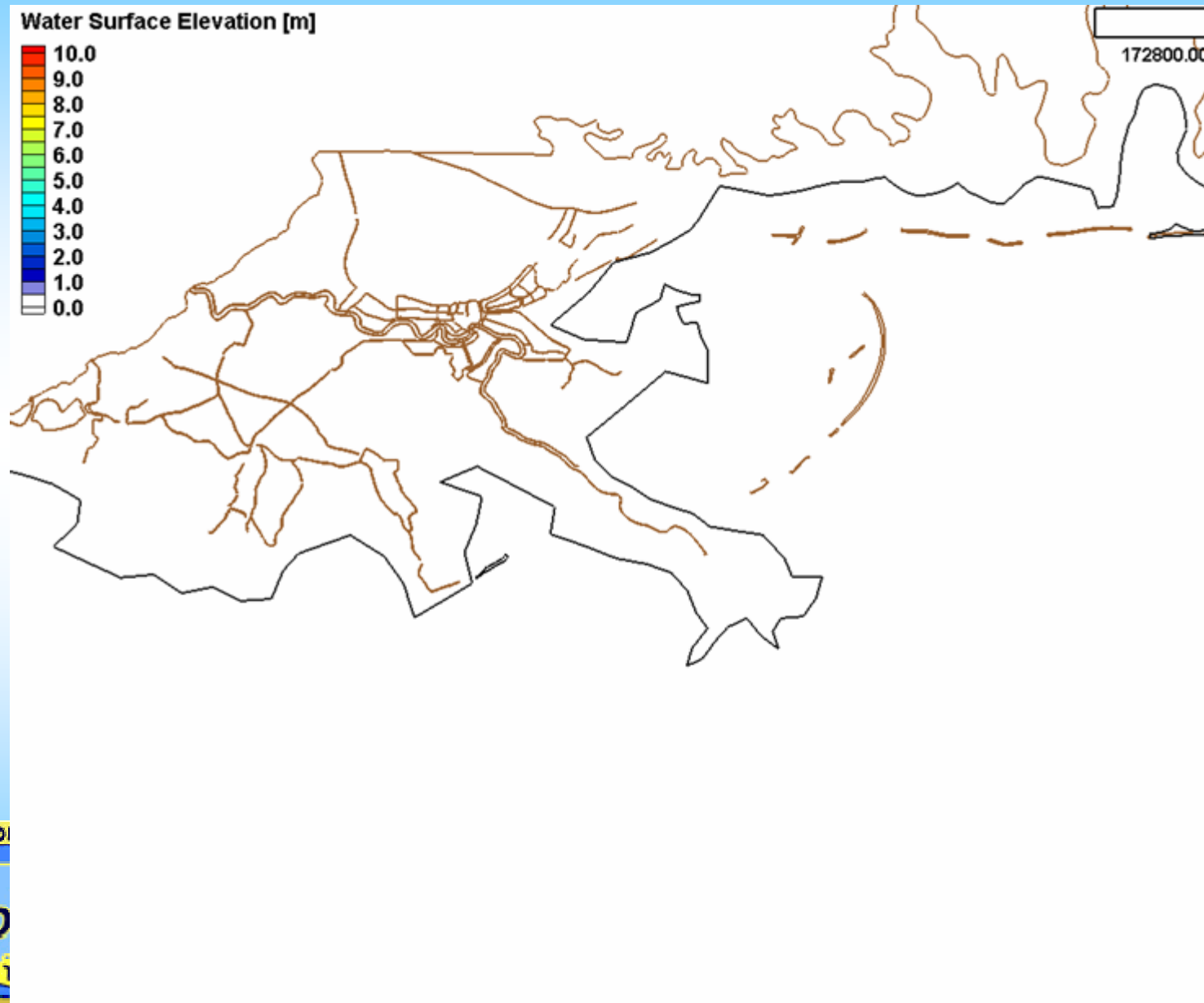


100 Years Wind Event Modeling

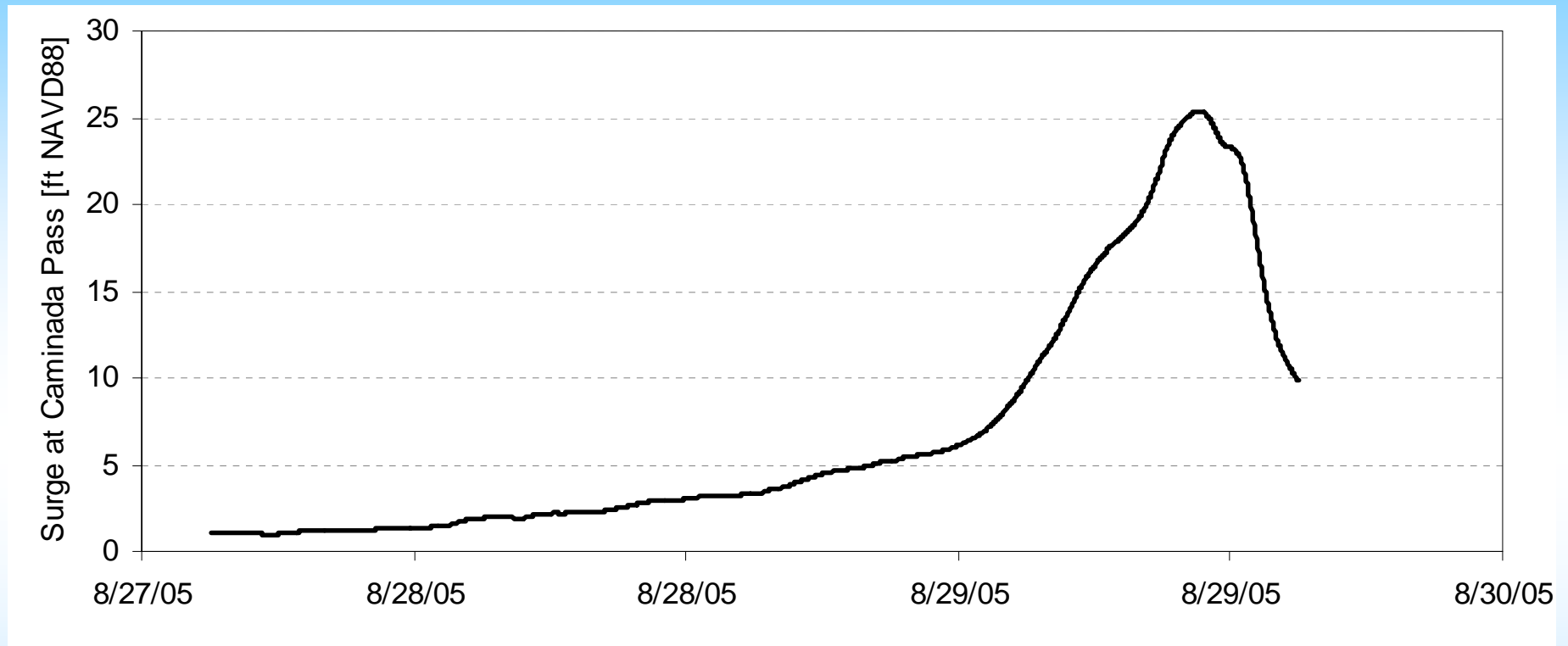


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100 Year Wind Surge Modeling

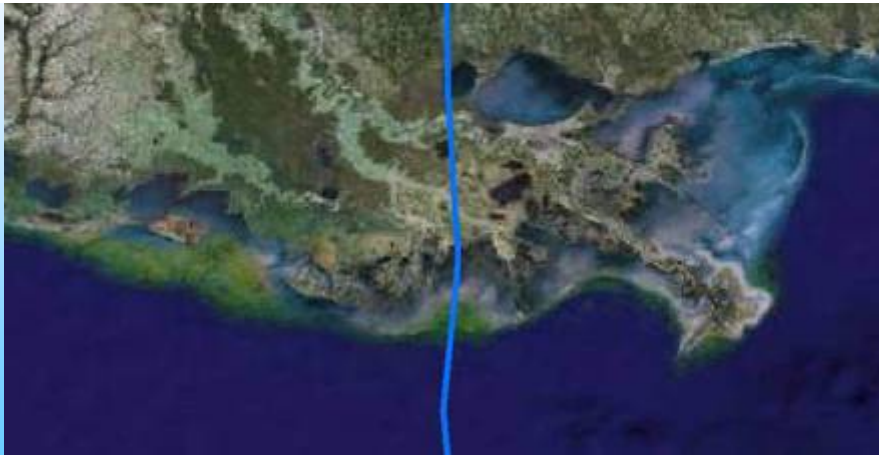
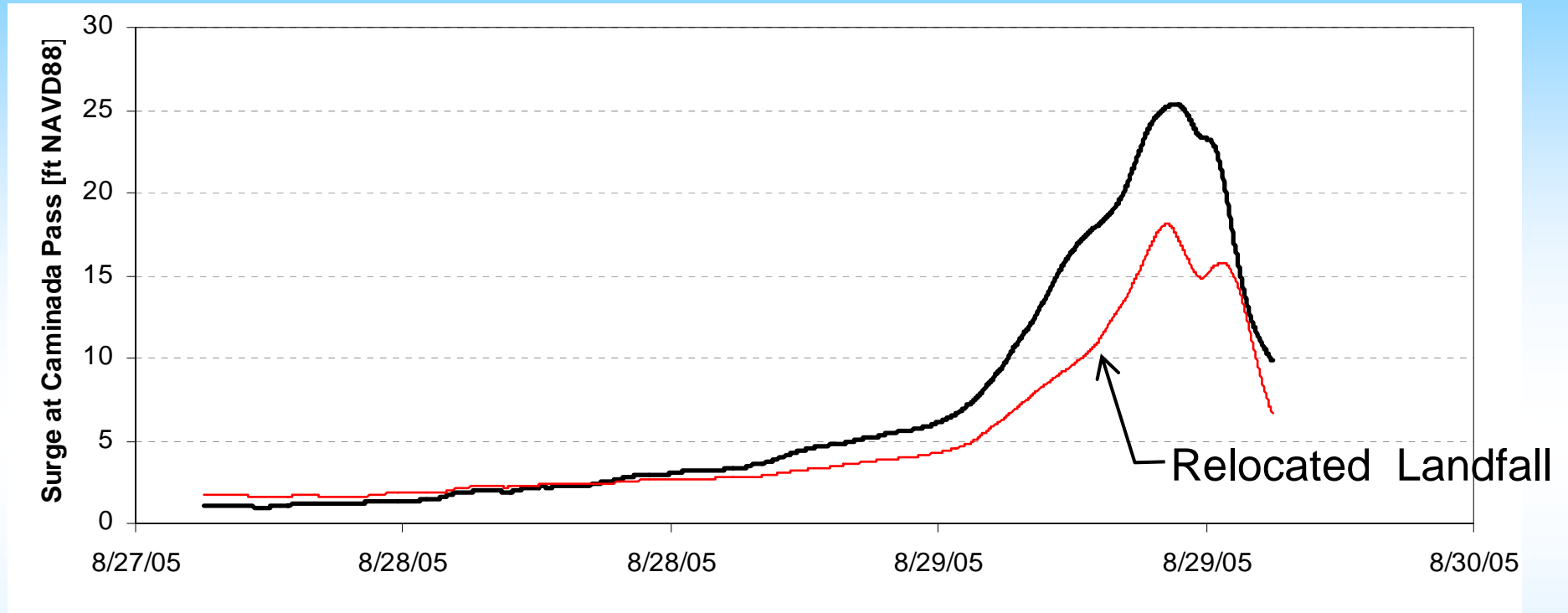


100-year Storm Surge Elevations Caminada Pass Landfall



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100-year Storm Surge Elevations Caminada Pass Landfall

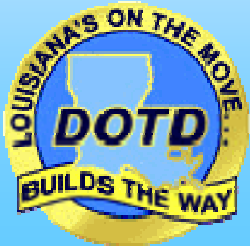


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Storm Surge Elevations Return Periods

Caminada Pass (*based on US Army COE*)

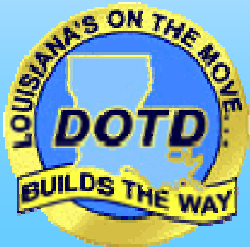
| Return Period [yr] | Storm Surge [ft NAVD88] |
|-----------------------|----------------------------|
| 50 | 8.8 |
| 75 | 9.8 |
| 100 | 10.7 |
| 150 | 11.6 |
| 200 | 12.2 |
| 250 | 12.6 |
| 300 | 12.9 |
| 350 | 13.2 |
| 400 | 13.4 |
| 450 | 13.5 |
| 500 | 13.7 |



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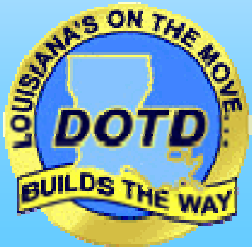
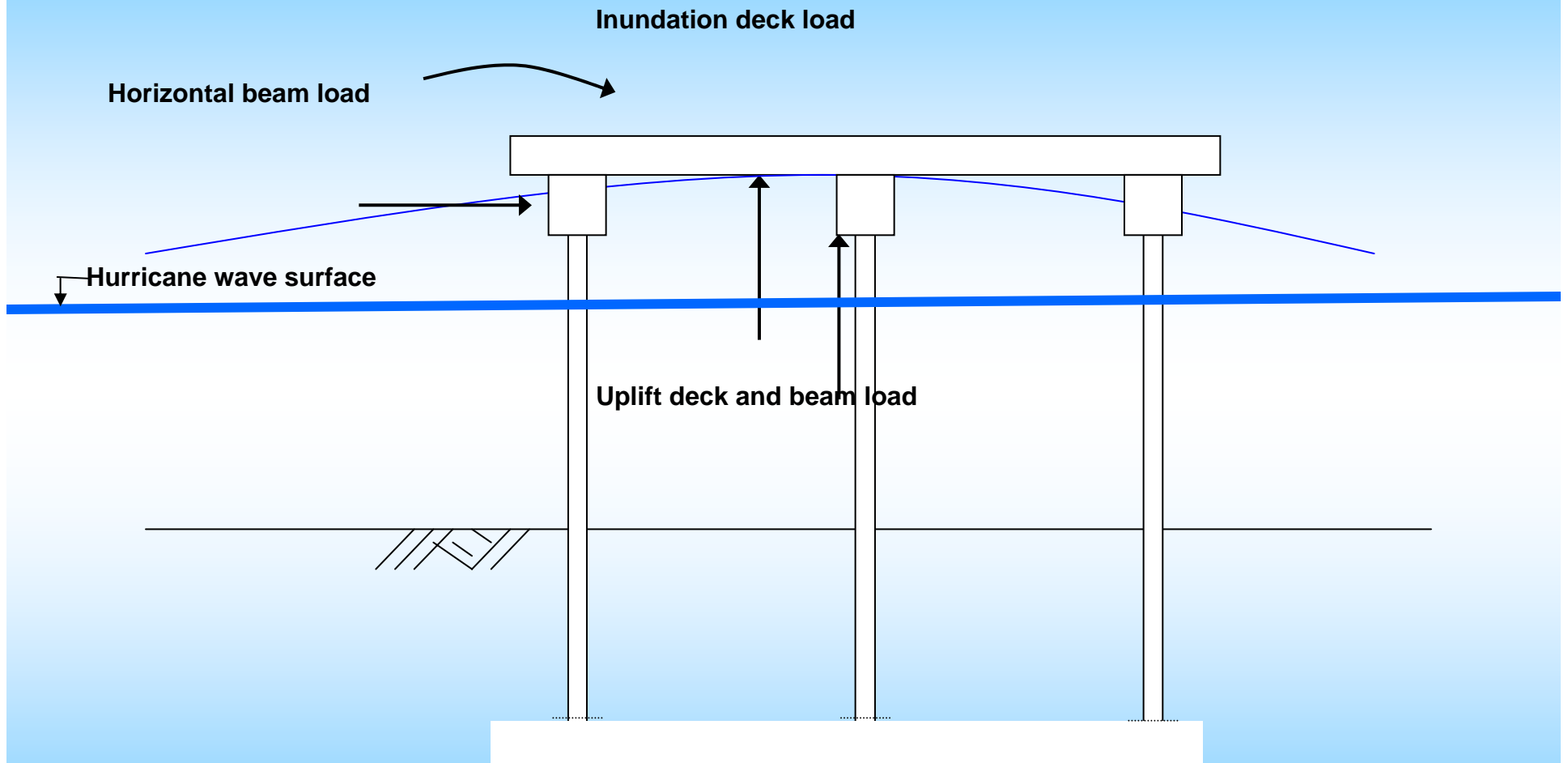
Storm Scenarios

| Scenario | Wind Return Period (years) | Wind Speed (mph) | Surge Return Period (years) | Surge (ft NAVD88) |
|----------|----------------------------|------------------|-----------------------------|-------------------|
| 1 | 100 | 182 | 100 | 10.7 |
| 2 | 100 | 182 | 50 | 8.8 |
| 3 | 50 | 173 | 50 | 8.8 |

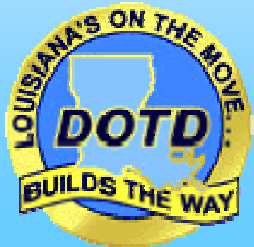
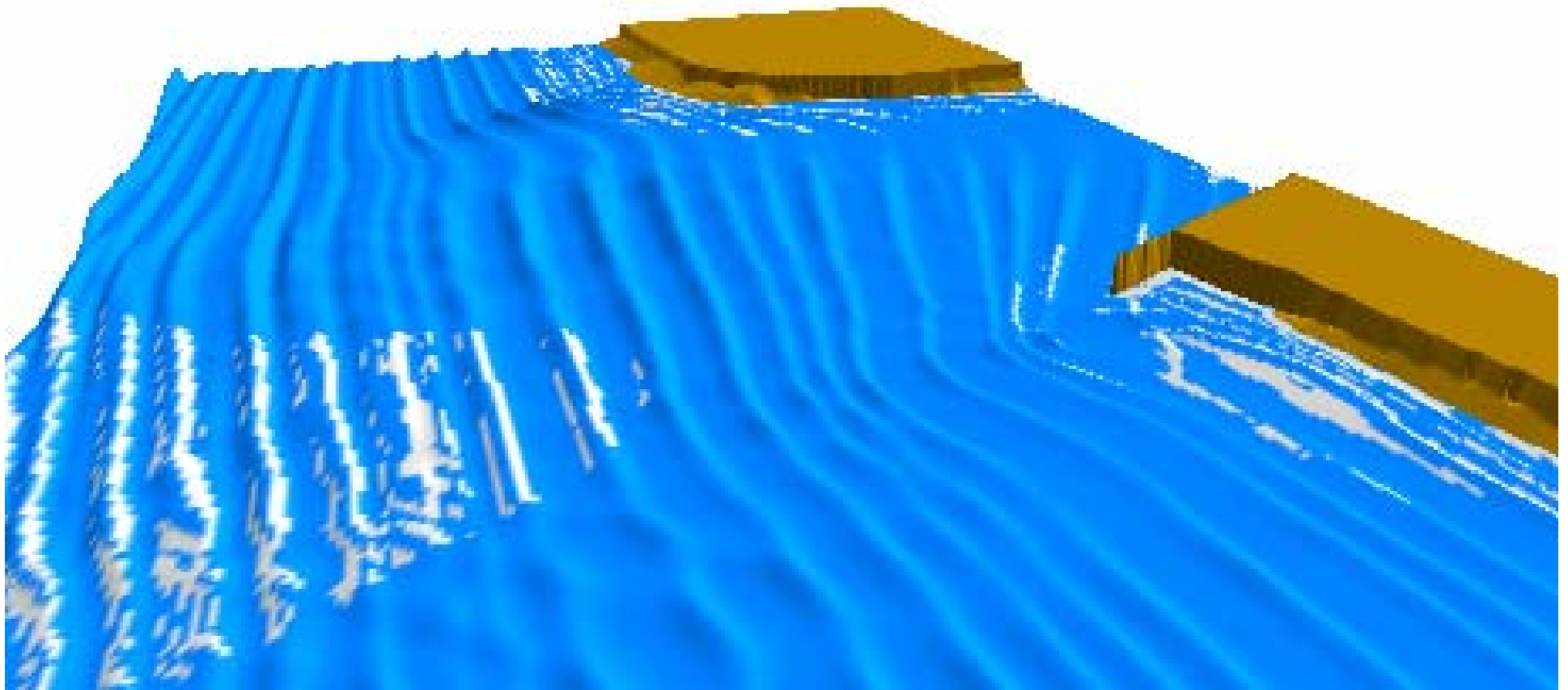


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Wave Loads Diagram

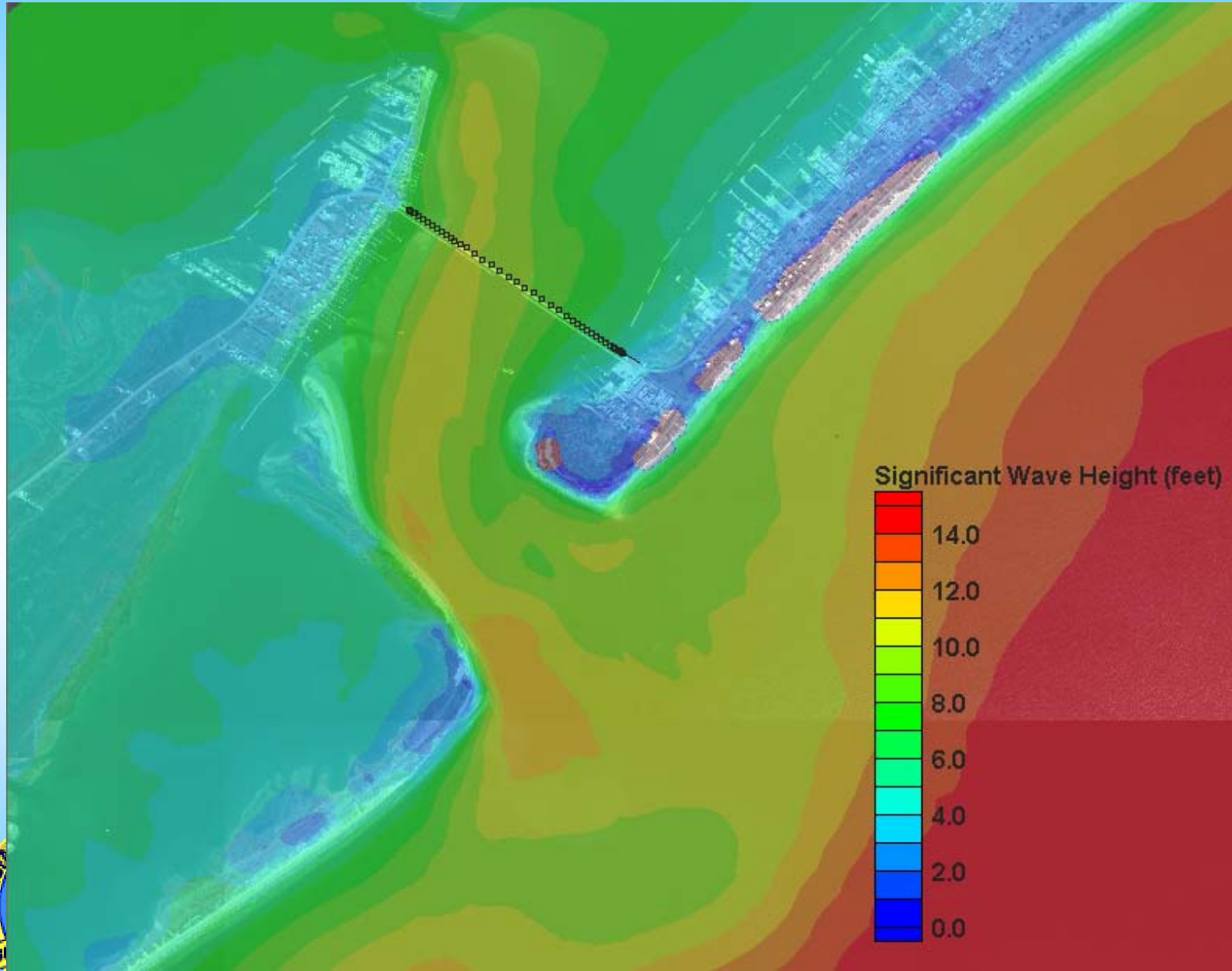


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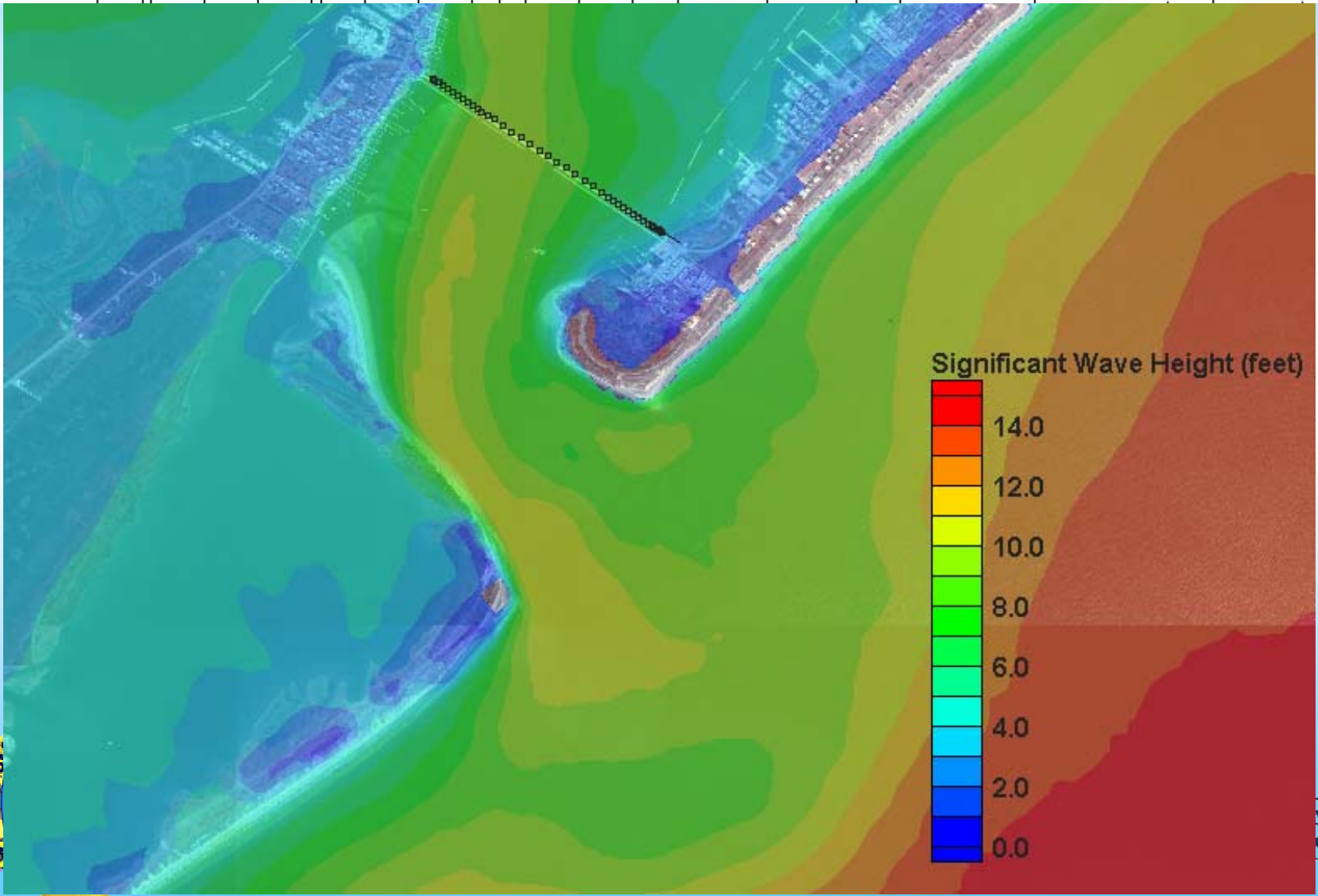
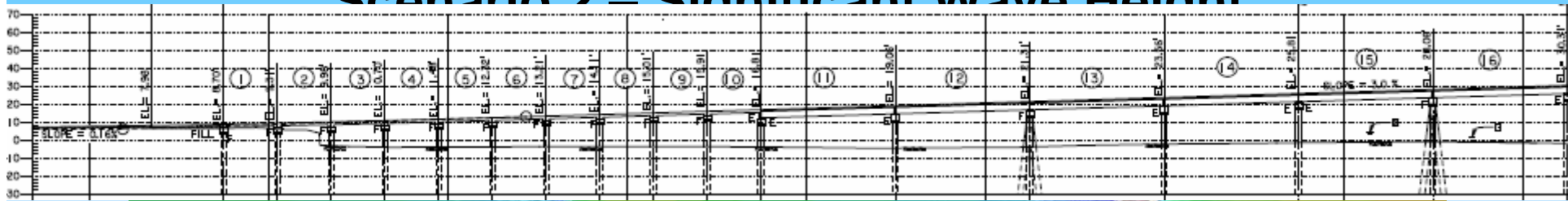
Scenario 1 – Significant Wave Height



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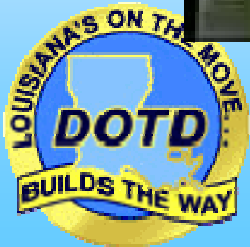
Scenario 2 - Significant Wave Height



New Bridge Layout



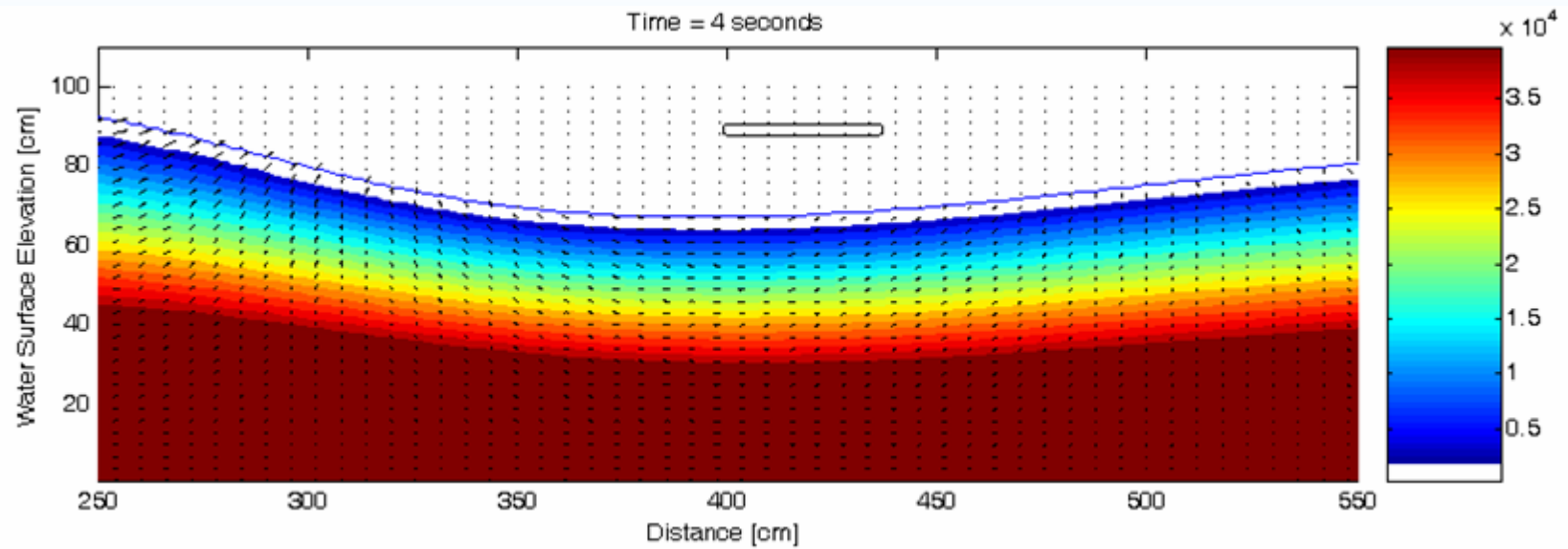
Cuomo & Tirindelli



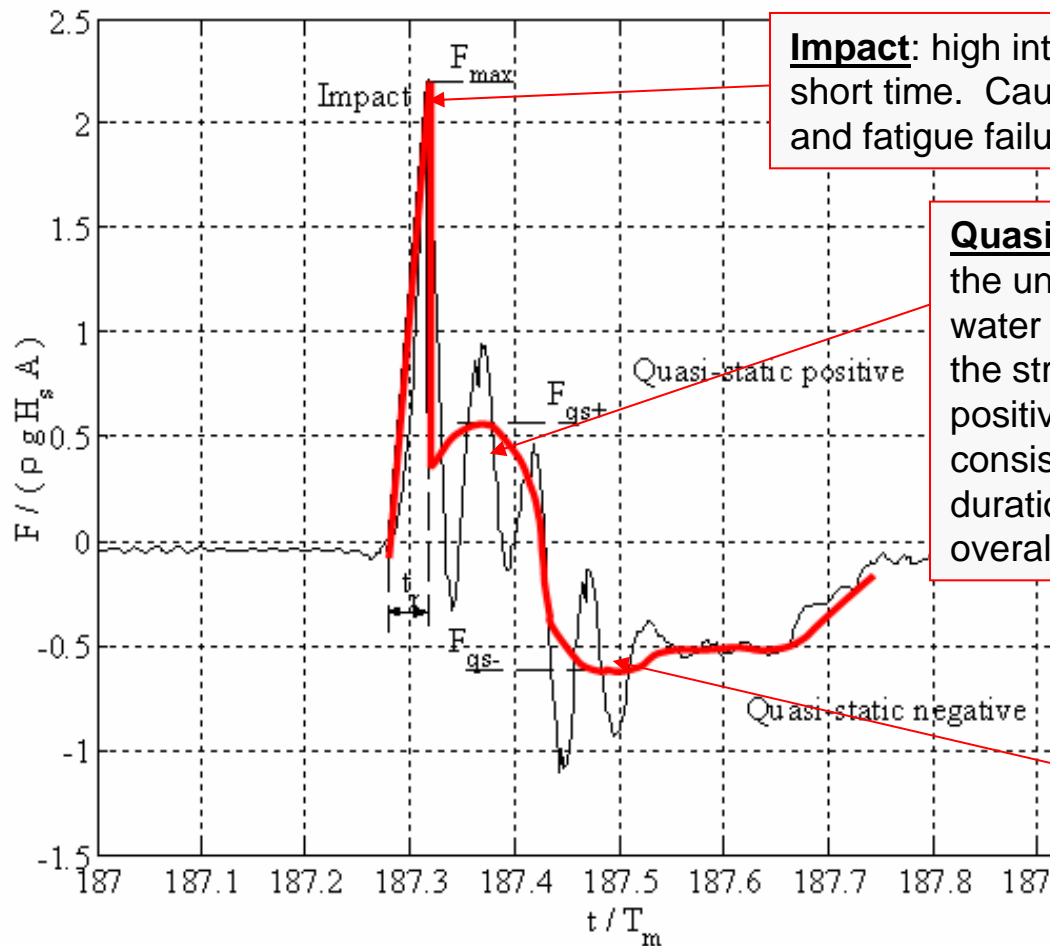
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Time = 4 seconds



Vertical Wave Loading



Impact: high intensity forces acting on limited areas, for a short time. Causes severe local damage, local yielding and fatigue failure

Quasi-static +: As the wave propagates along the underside of a deck, a difference between water level under the deck and that alongside the structure gives rise to the quasi-static positive force. The magnitude of this force is consistently lower than any initial impact, but its duration is of order $0.25 \cdot T_m$. Important for overall structure stability

Quasi-static -: Finally, the wave surface falls below soffit level and moves inward below the deck, reducing the contact area with the wave. A quasi-static negative force is increased when the wave inundates the deck



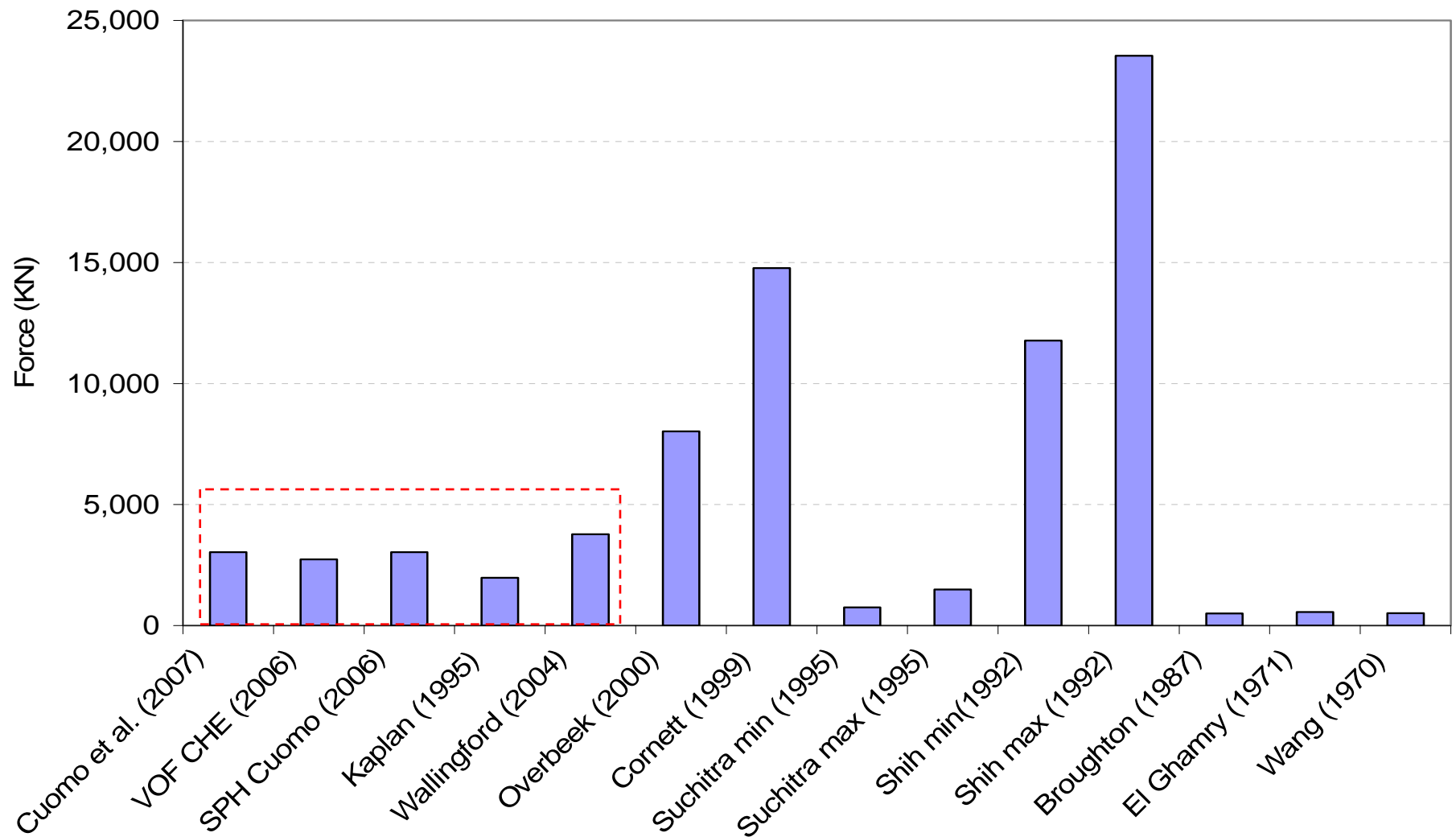
All 3 types of loading must be considered



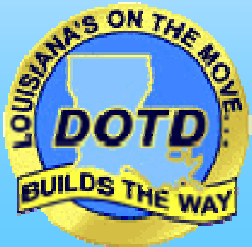
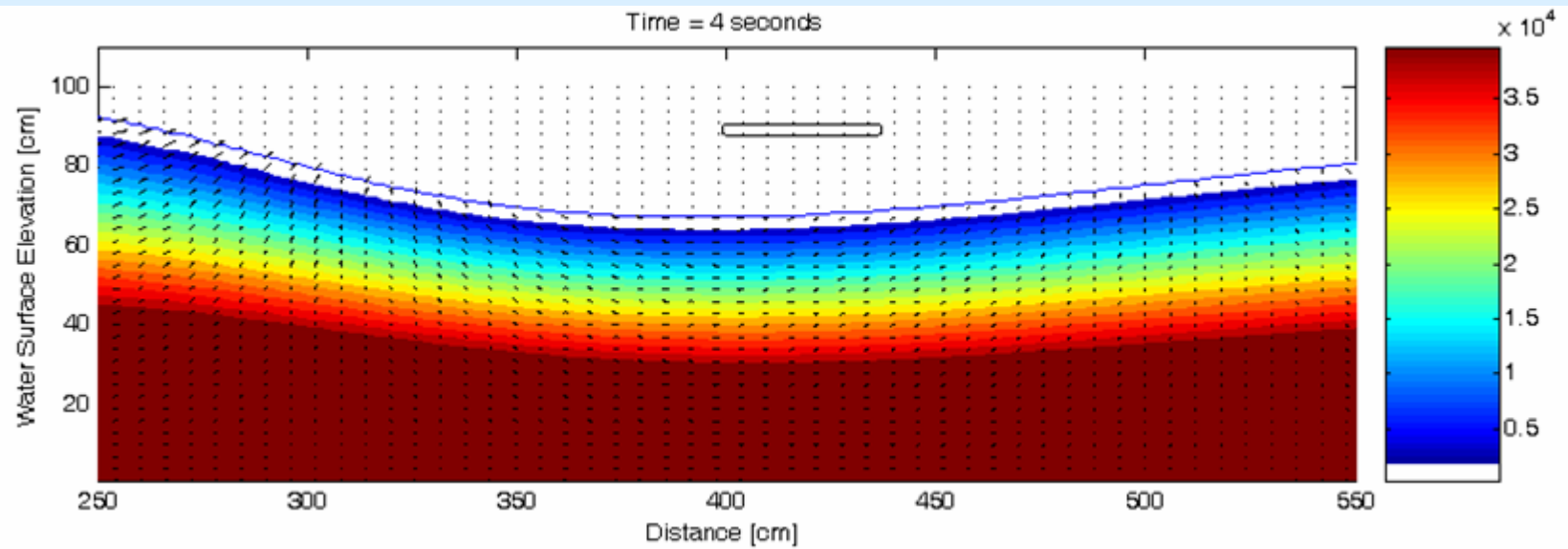
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Evaluations of Uplift Loading

Hs = 3.4m; Tp = 7.4s; Depth = 15.3m; Clearance = 1.4m; Deck length = 7.4m;
Deck Width = 11.6m

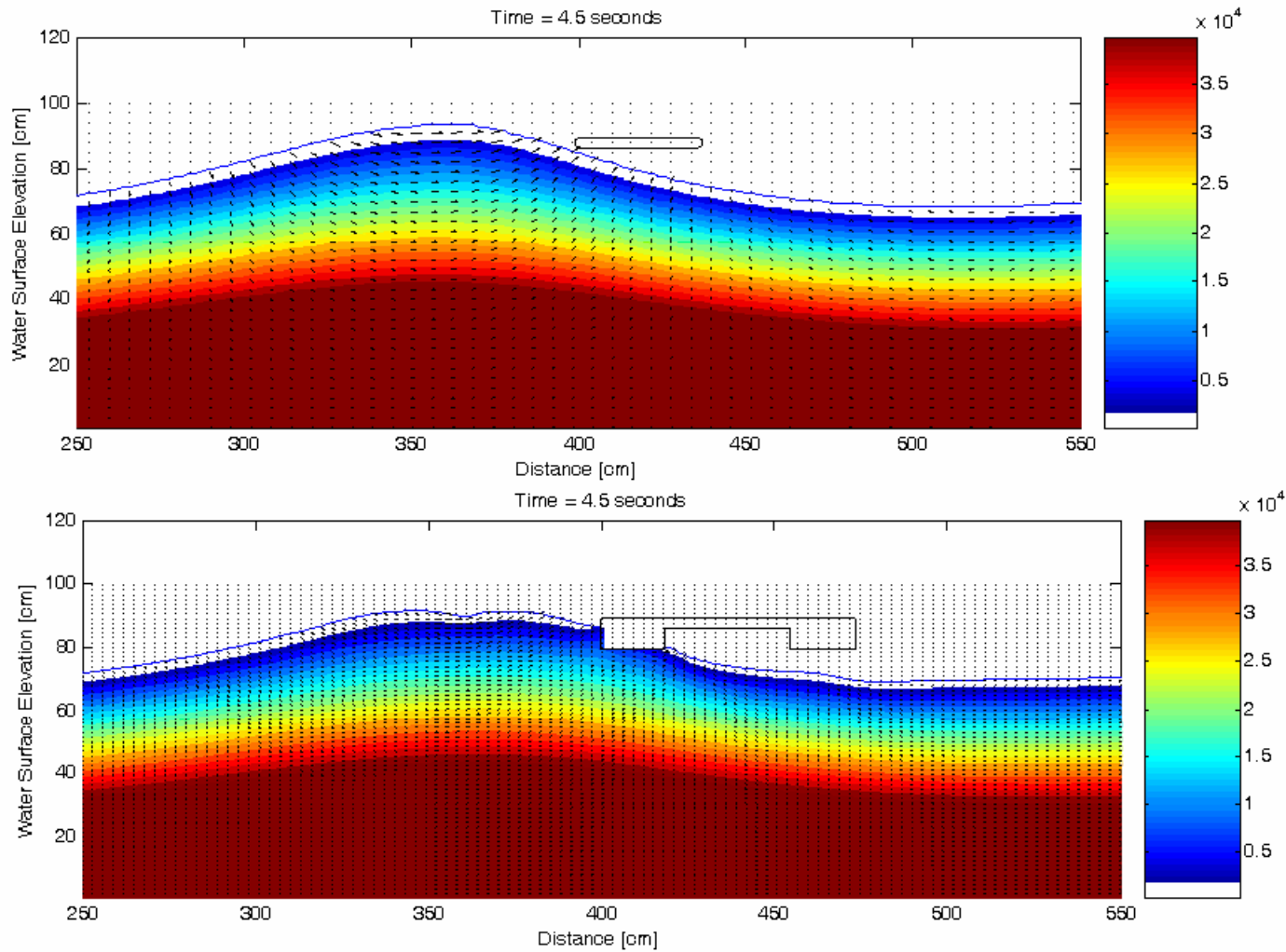


Dynes/cm²

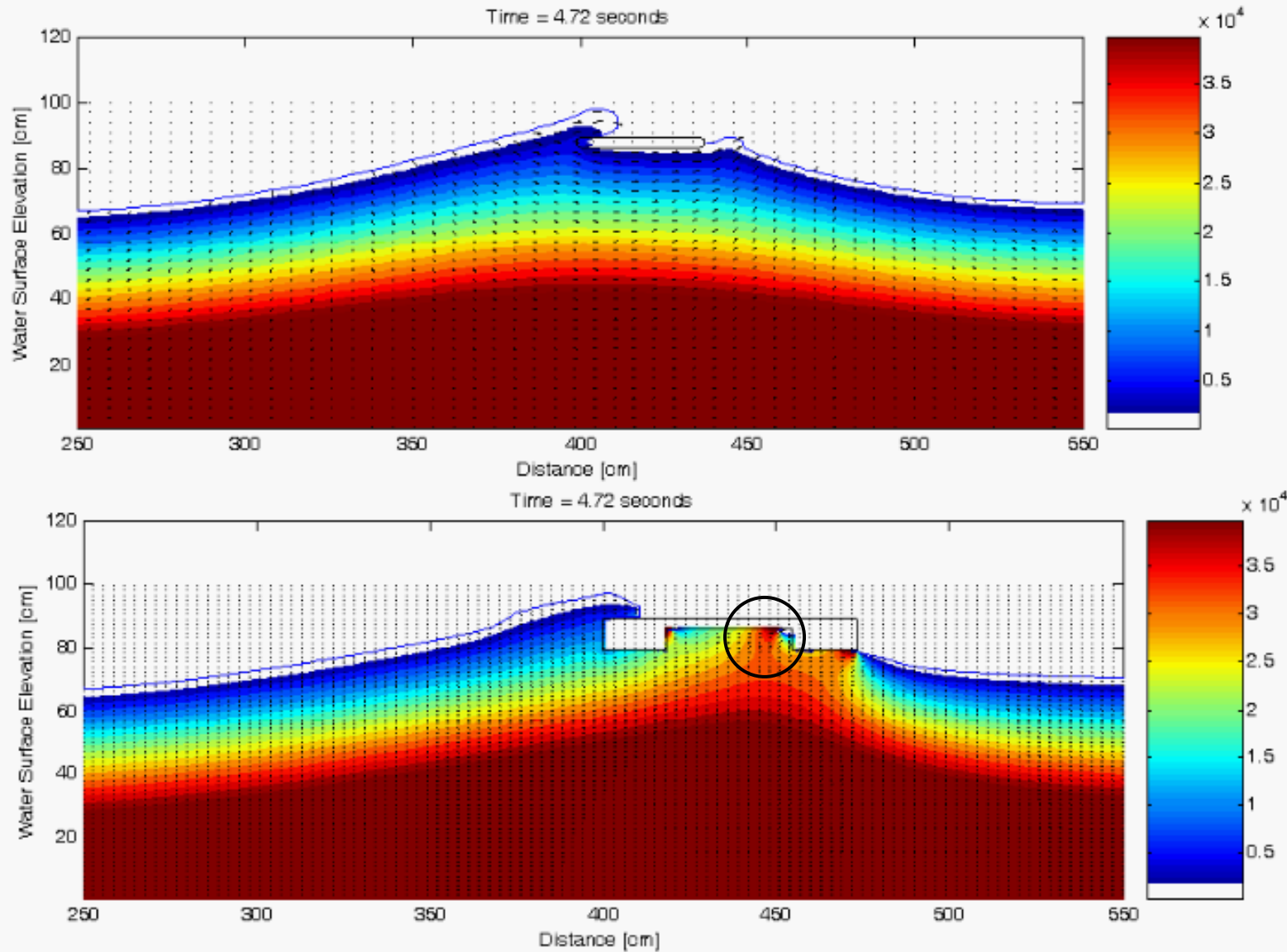


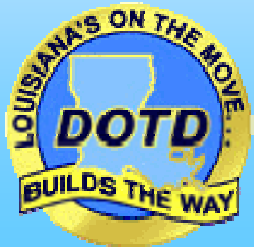
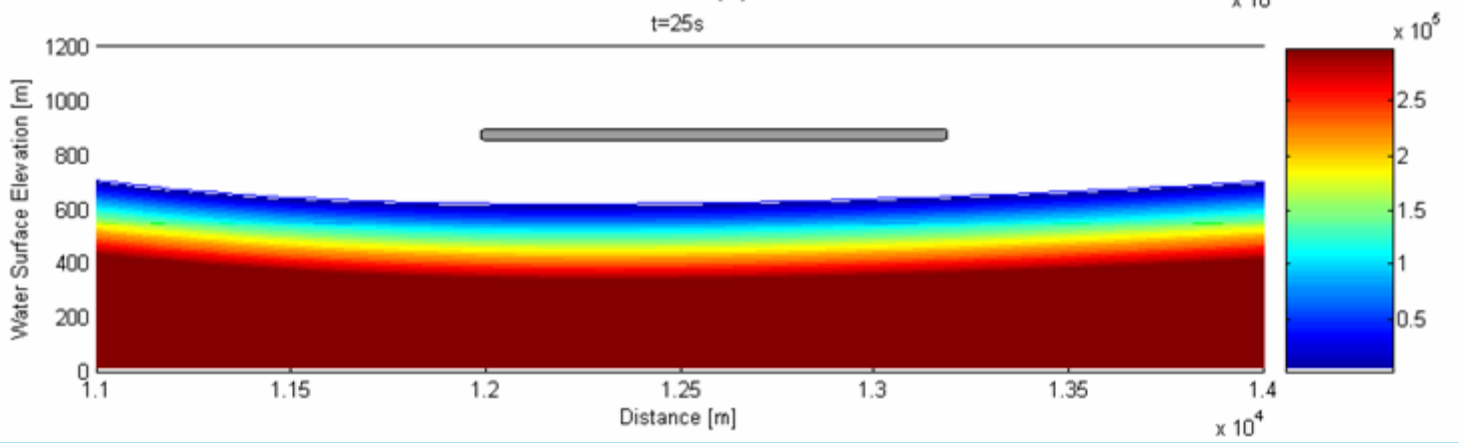
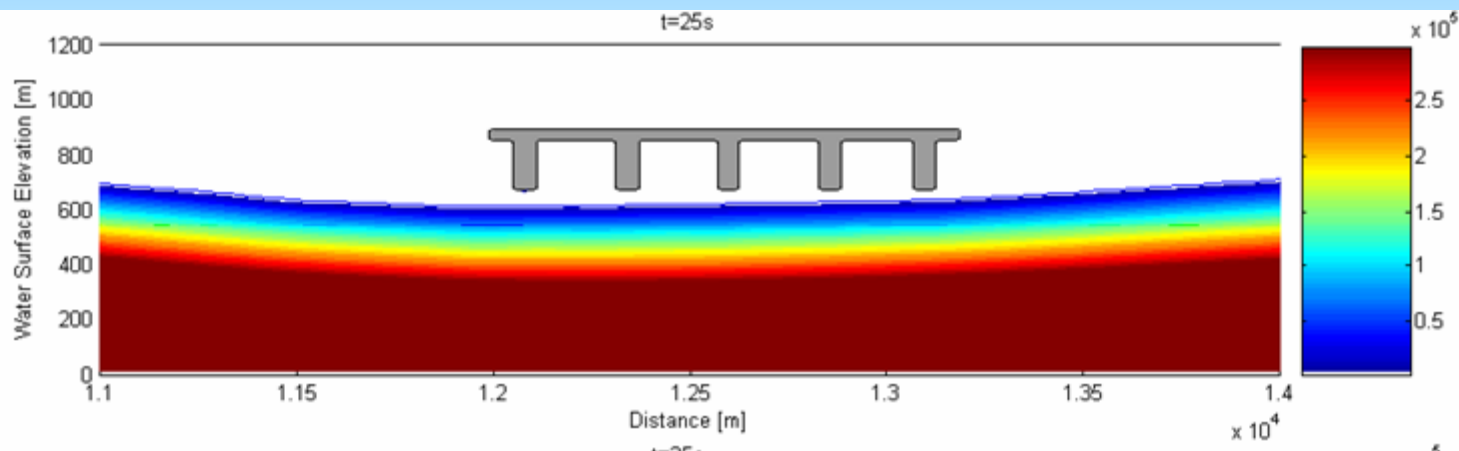
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Influence of Down-standing Beams



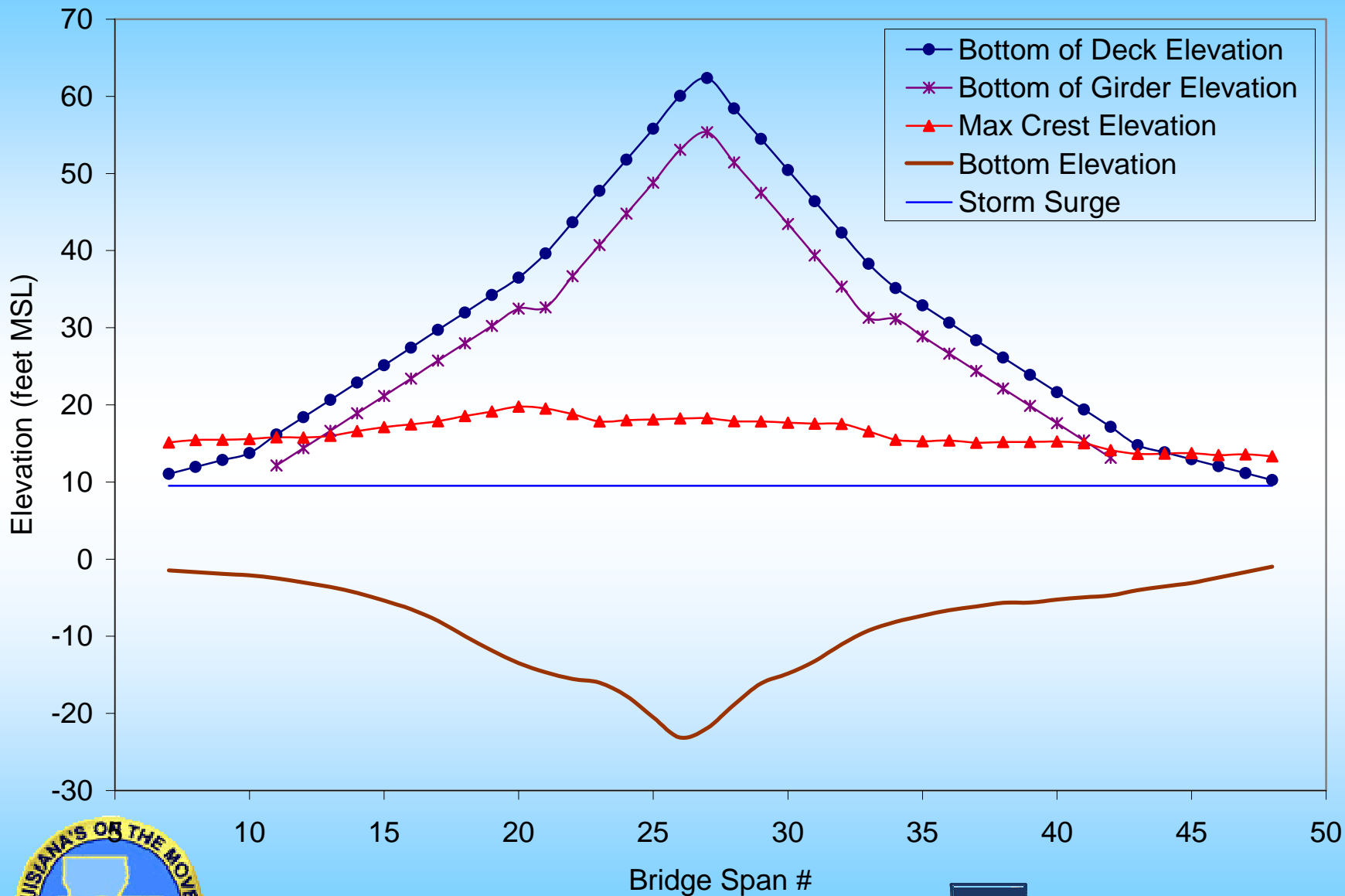
Wave Loading Design Criteria Influence of Down-standing Beams



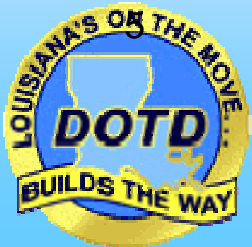
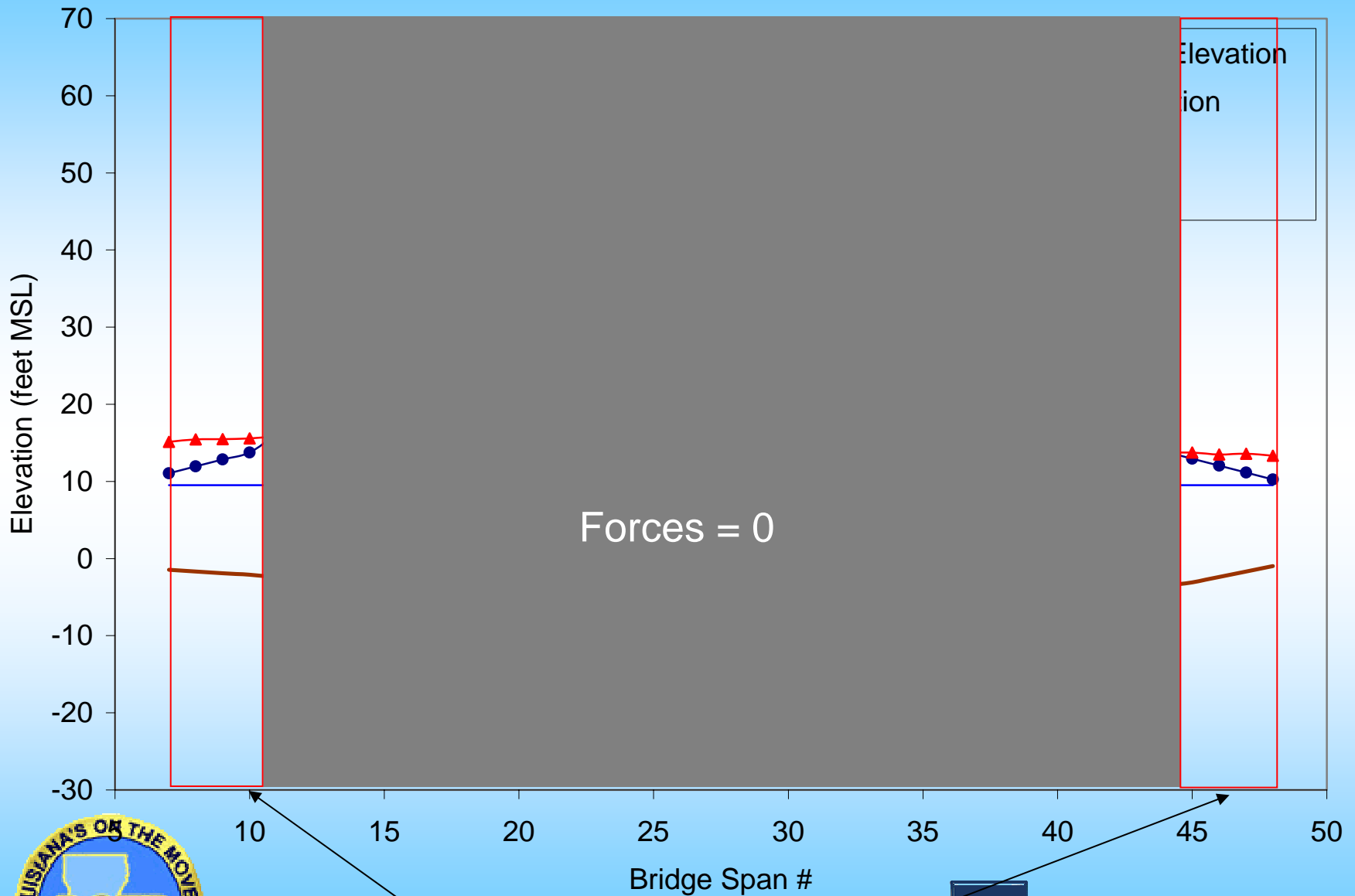


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Scenario 1 – Max Crest Elevations



Scenario 1 – Max Crest Elevations



Inundated Deck Slabs



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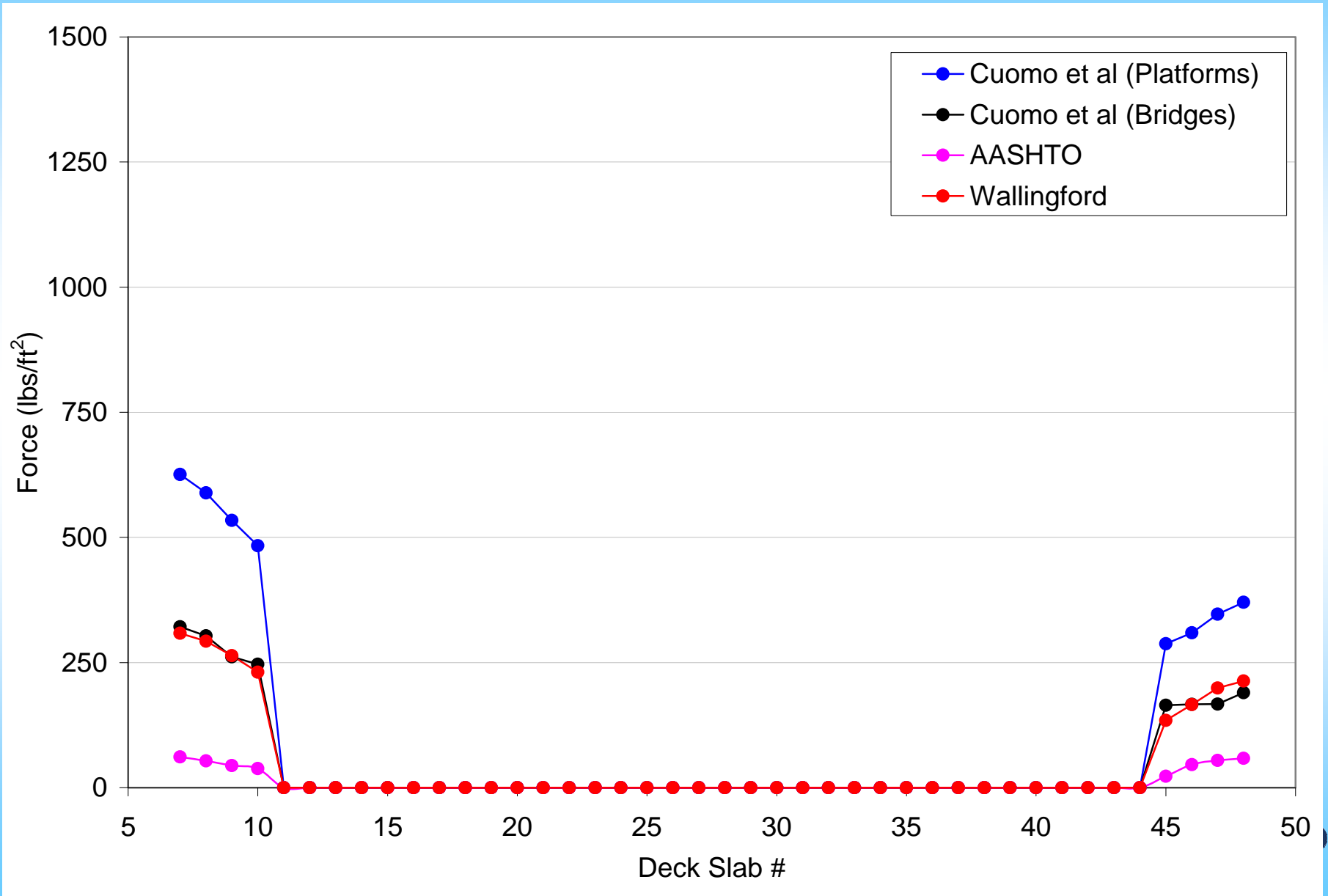
Methods Used for Wave Force Evaluation

| Method | Year | Typology | Direct. of Force | Type of Force | Use |
|----------------|-------|-----------------|-----------------------|----------------------------|--------------|
| Cuomo et al. | 2007a | Experimental | Horizontal / Vertical | QS ^[1] / Impact | Quantitative |
| Cuomo et al. | 2007b | Experimental | Vertical | QS / Impact | Quantitative |
| AASHTO | 2007 | Experimental | Horizontal / Vertical | QS / Impact | Quantitative |
| Wallingford | 2004 | Experimental | Horizontal / Vertical | QS | Quantitative |
| VOF | 2004 | Numerical | Horizontal / Vertical | QS / Impact | Qualitative |
| Kaplan et al. | 1995 | Semi-analytical | Horizontal / Vertical | QS / Impact | Qualitative |
| Morison et al. | 1950 | Semi-analytical | Horizontal | QS / Impact | Quantitative |

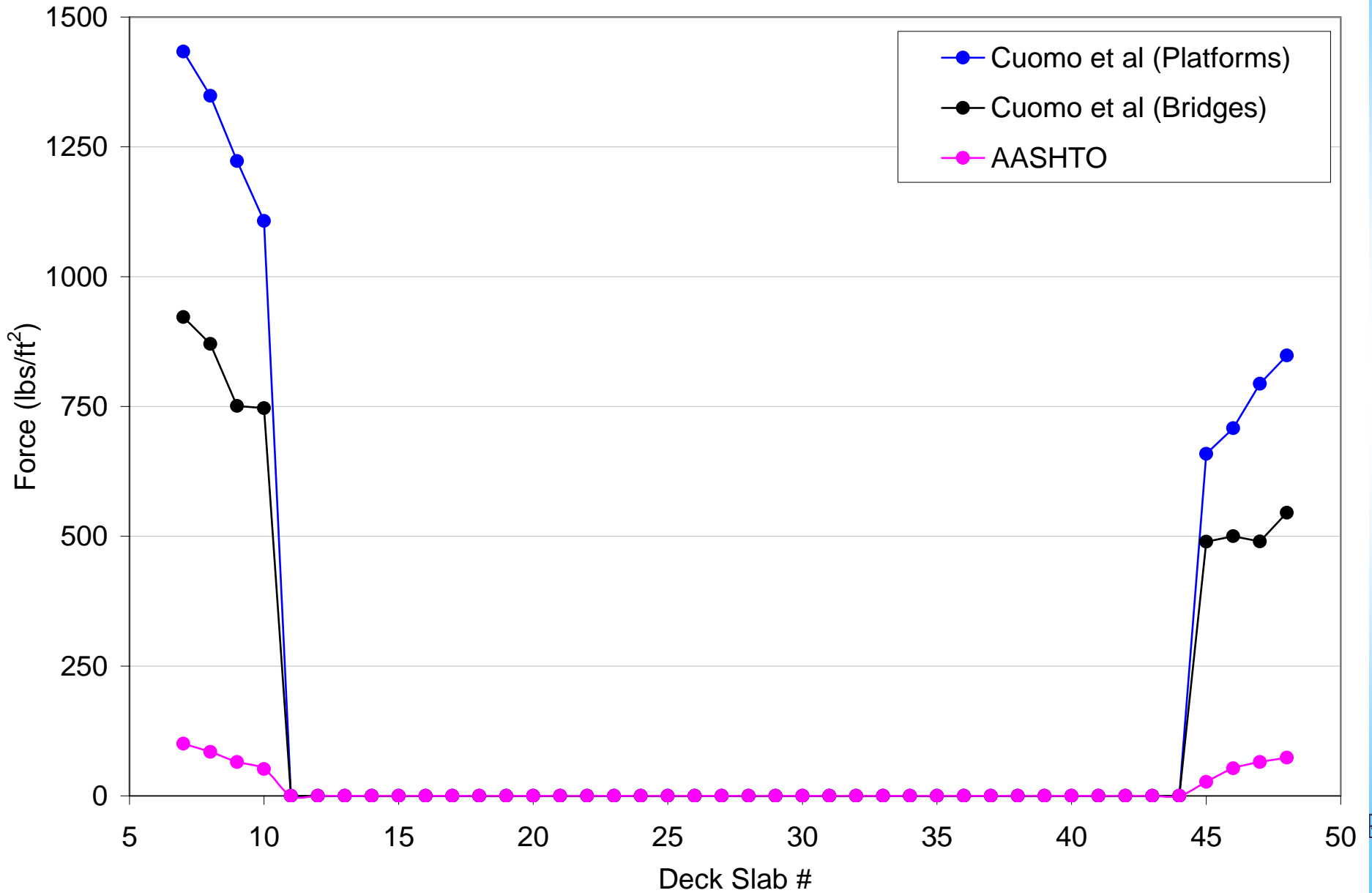


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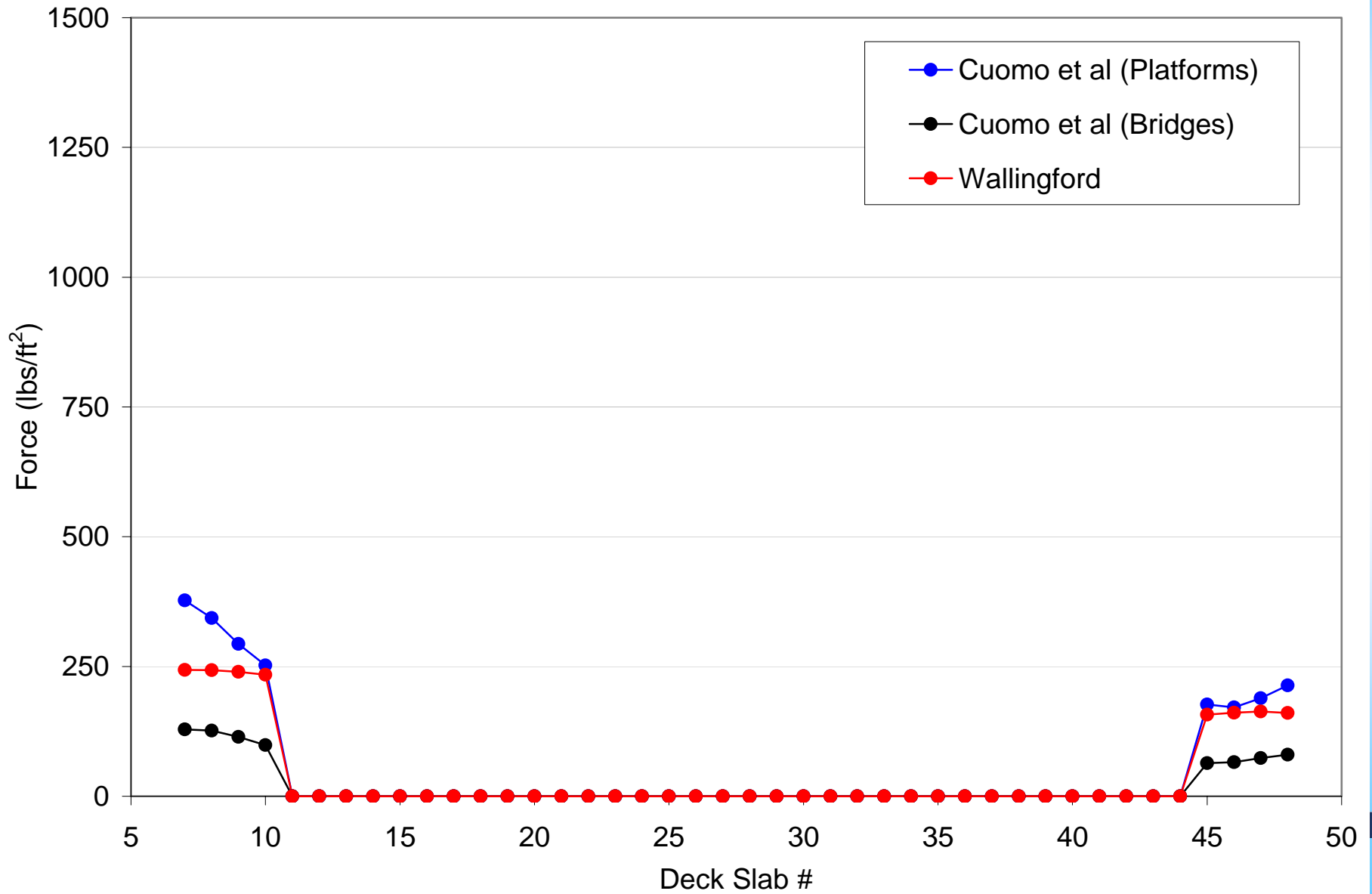
Vertical Quasi-Static Positive Forces on Deck Slabs for Scenario 1



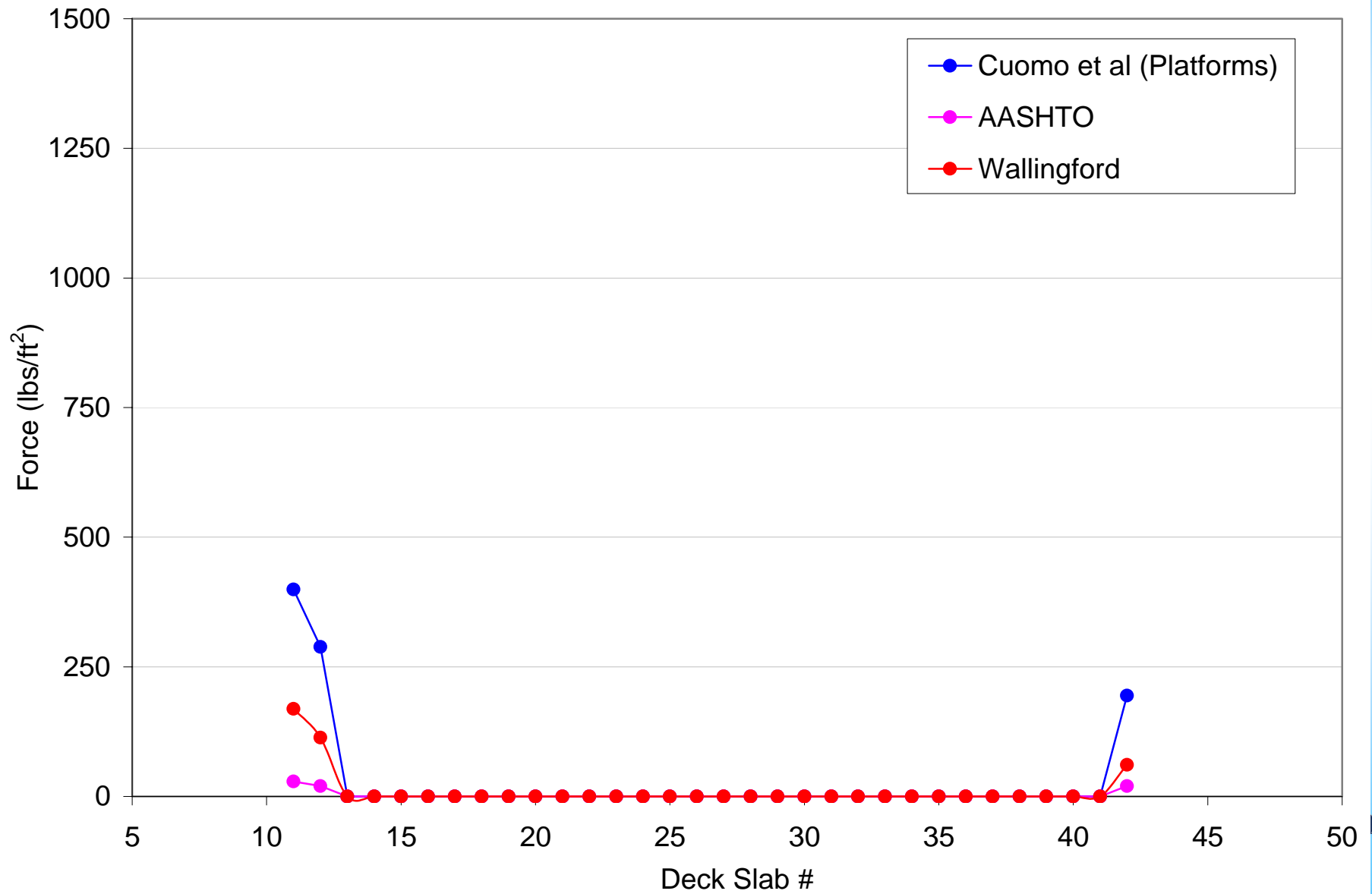
Vertical Impact Forces on Deck Slabs for Scenario 1



Vertical Quasi-Static Negative Forces on Deck Slabs for Scenario 1



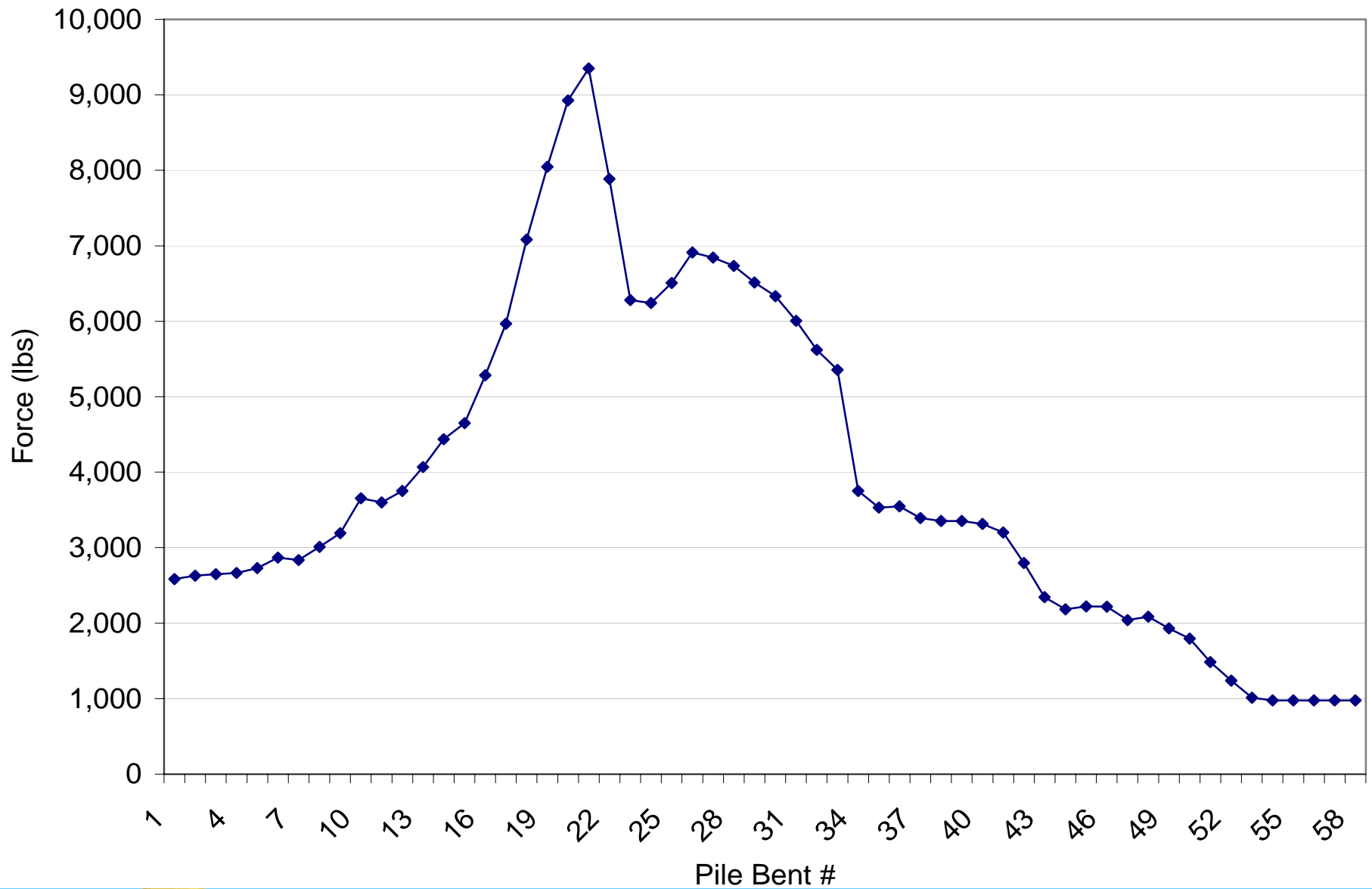
Horizontal Quasi-Static Forces on Girders for Scenario 1



F_i

OR

Horizontal Wave Forces on Piles for Scenario 1



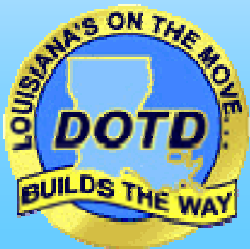
Vertical Wave Forces on Deck Slabs at Caminada Bridge for Scenario 1

| Bridge Span | Impact (lbs/ft ²) | QS+ (lbs/ft ²) | QS- (lbs/ft ²) |
|-------------|-------------------------------|----------------------------|----------------------------|
| 7 | 1180 | 470 | 250 |
| 8 | 1110 | 450 | 230 |
| 9 | 990 | 400 | 200 |
| 10 | 930 | 370 | 180 |
| 11-44 | 0 | 0 | 0 |
| 45 | 570 | 230 | 120 |
| 46 | 600 | 240 | 120 |
| 47 | 640 | 260 | 130 |
| 48 | 700 | 280 | 150 |

OR



| Bridge Elements | Computed Wave Loads | |
|-----------------|---------------------|------------|
| | Vertical | Horizontal |
| Deck slabs | 20 | n/a |
| Girders | 3 | 3 |
| Railings | n/a | 20 |
| Pile Caps | 33 | 33 |
| Piles | n/a | 52 |
| Columns | n/a | 13 |
| Footings | 13 | 13 |



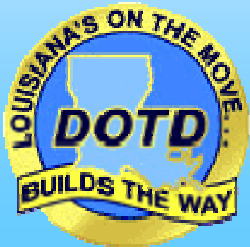
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European Research Foundation Grant 2008-2009

Vertical Wave Loads on Piers, Decks, and Bridges during Extreme Events

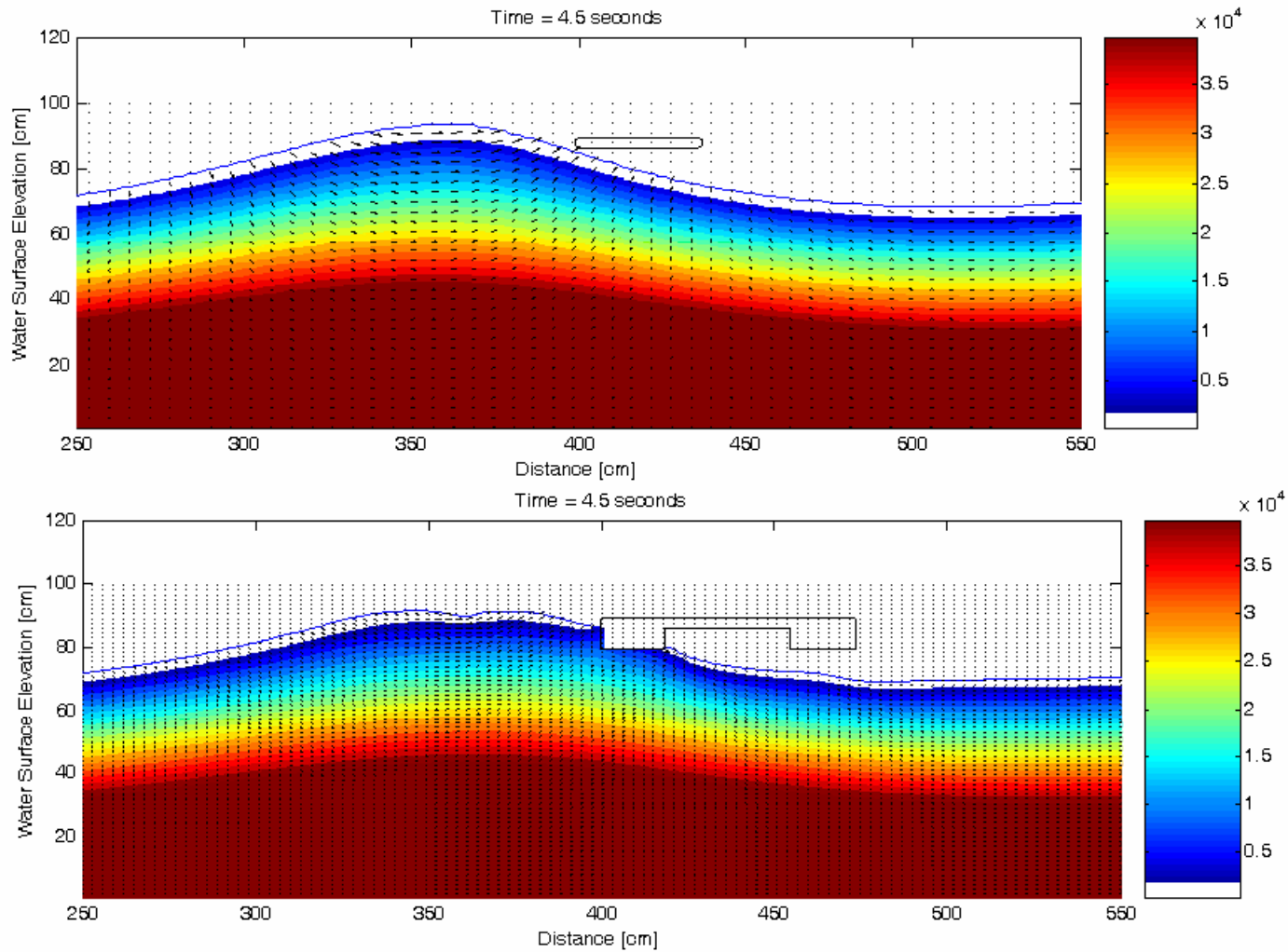
International Team of Scientists and Engineers

- **University of Bologna, Italy**
- **University of Rome, Italy**
- **University of Edinburgh, UK**
- **Coast & Harbor Engineering, USA**



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Influence of Down-standing Beams



ForschungsZentrum Küste

Gemeinsame Zentrale Einrichtung der Universität Hannover und der Technischen Universität Braunschweig

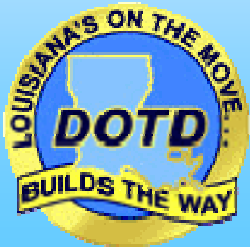
Merkurstrasse 11, 30419 Hannover, Germany



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Example Projects at Large Wave Channel

Low Crested and Submerged Breakwaters in Presence of Broken Waves, EU, 2002



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Example Projects at Large Wave Channel

Breaking Wave Impacts on Steep Fronted Coastal Structures (BWIMCOST, 2003)



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**Caminada Bridge (LA): Coastal Engineering Analysis & Numerical
Modeling in Support for Design
(*Hurricane Wind and Wave Action*)**

Hossein Ghara, P.E., M.B.A., State of Louisiana DOT&D

Artur Wagner, P.E., State of Louisiana DOT&D

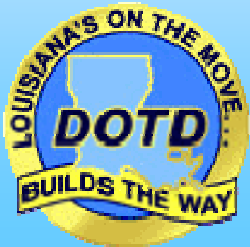
Jenny Fu, P.E., State of Louisiana DOT&D

Vladimir Shepsis, P.E., PhD Coast & Harbor Engineering, Inc.

Matteo Tirindelli, PhD Coast & Harbor Engineering, Inc.

Thank You

*New Orleans
February 21, 2008*



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