



Louisiana DOTD Experience with CPT for Driven Pile Design

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Louisiana Transportation Research Center

LTRC

<http://www.ltrc.lsu.edu>

Louisiana CPT Systems



- **Twenty-ton truck**

- LTRC - Research
- DOTD - Production
 - 220' max depth

- **Minicone truck**

- LTRC - Research
 - 40' max depth

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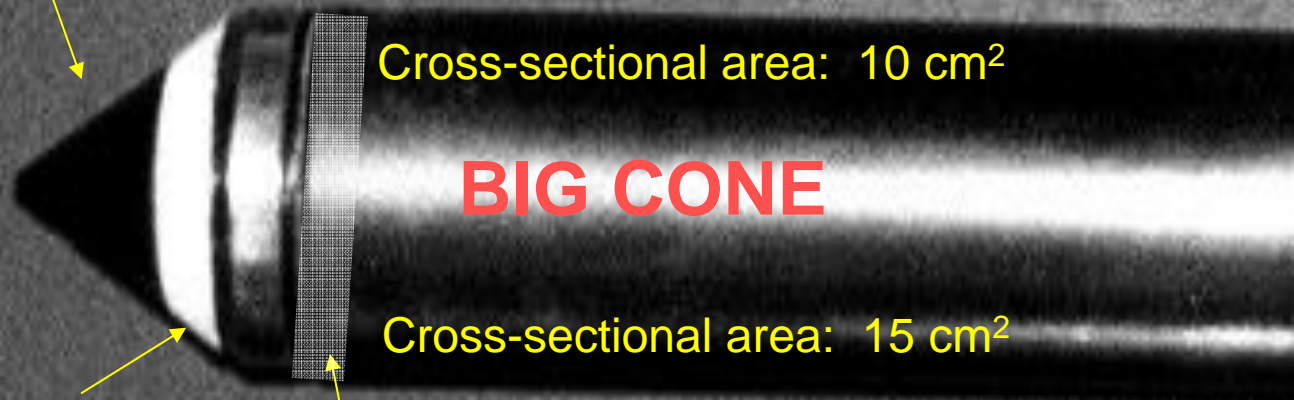
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Cross-sectional area: 2 cm²



MINICONE

60 degree
cone tips



Cross-sectional area: 10 cm²

BIG CONE

Cross-sectional area: 15 cm²

Ceramic
stone allows
porewater to
enter, U1

U2
Location, if
available

U3 Located at
end of sleeve,
If available

Available Cones:

- Friction
- Piezo

Available Cones:

- Friction
- Piezo
 - Single, U2
 - Dual, U1 & U3
- Seismic
- Conductivity

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CPT Technology In Louisiana

■ Primary Uses

- Load carrying capacity of piles
 - end-bearing piles (mid 80's to present)
 - friction piles (2000 to present)
- Soil classification (stratification)
- Soil improvement (relative density)

■ Current Research Effort

- Consolidation parameters
- Subgrade Resilient modulus (mini-cone)

CPT Experience for Driven Piles

End-bearing Piles : Locating dense sand layers

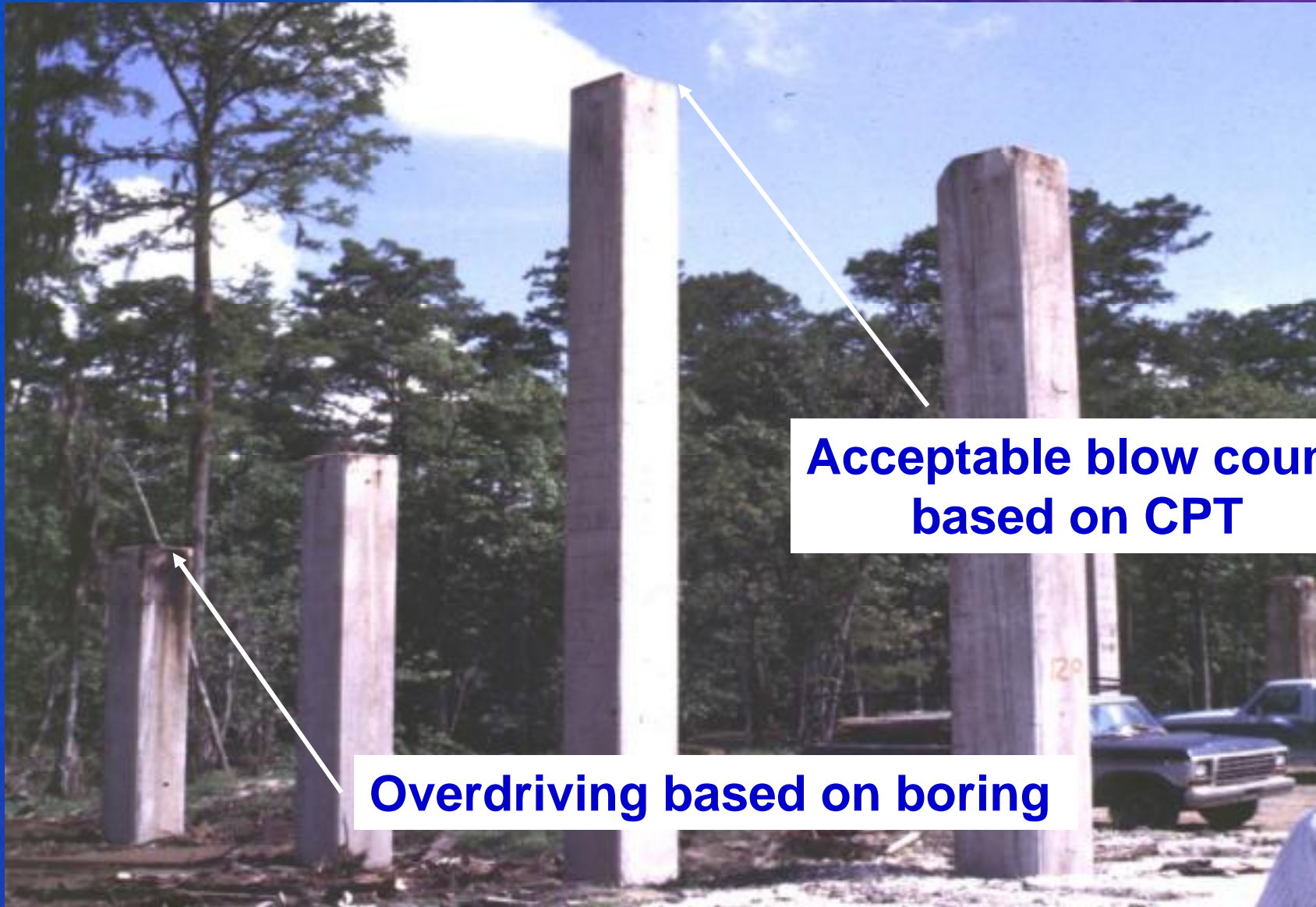
Precast Concrete Piles

Timber Piles

Steel Pipe Piles



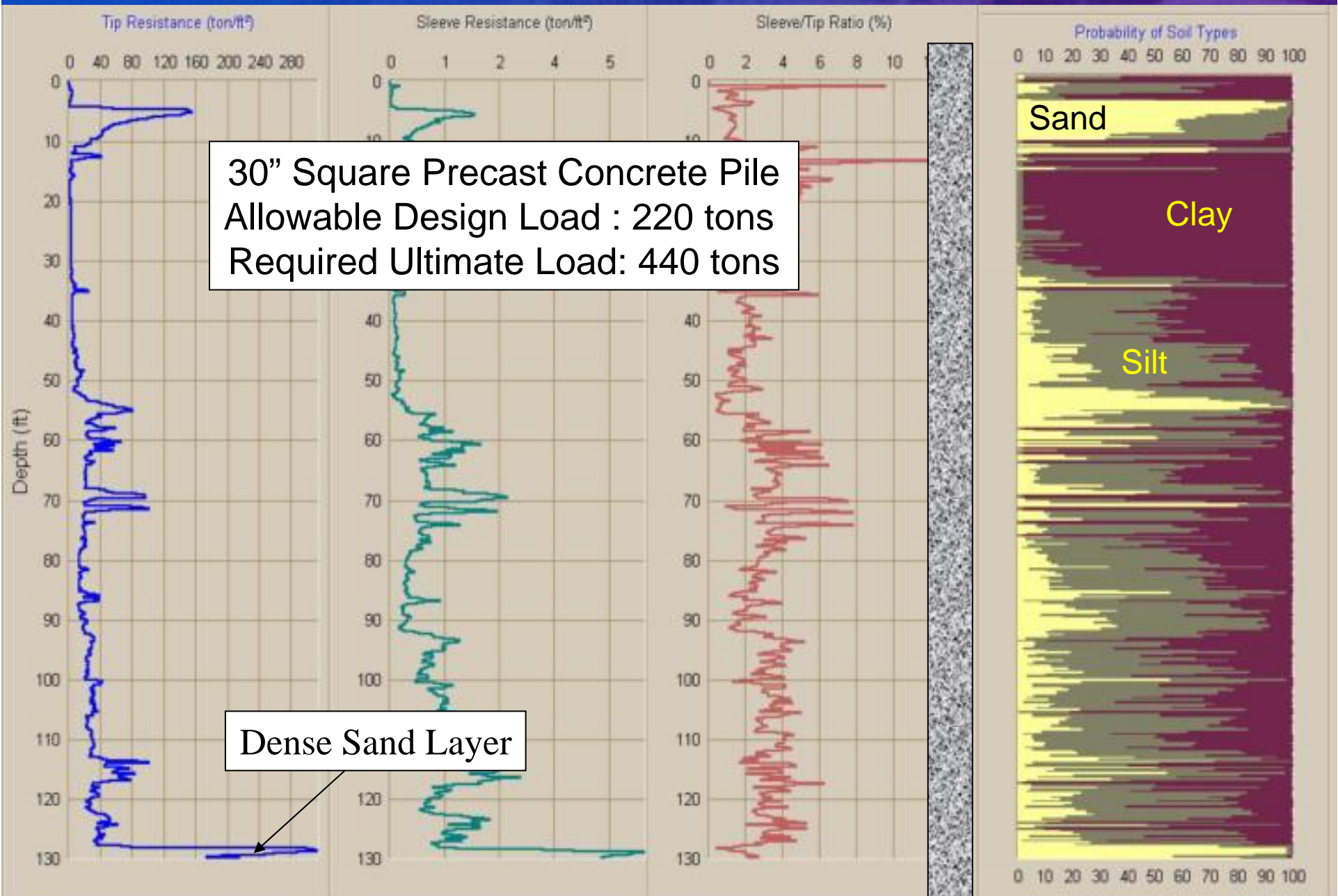
I-310 New Orleans

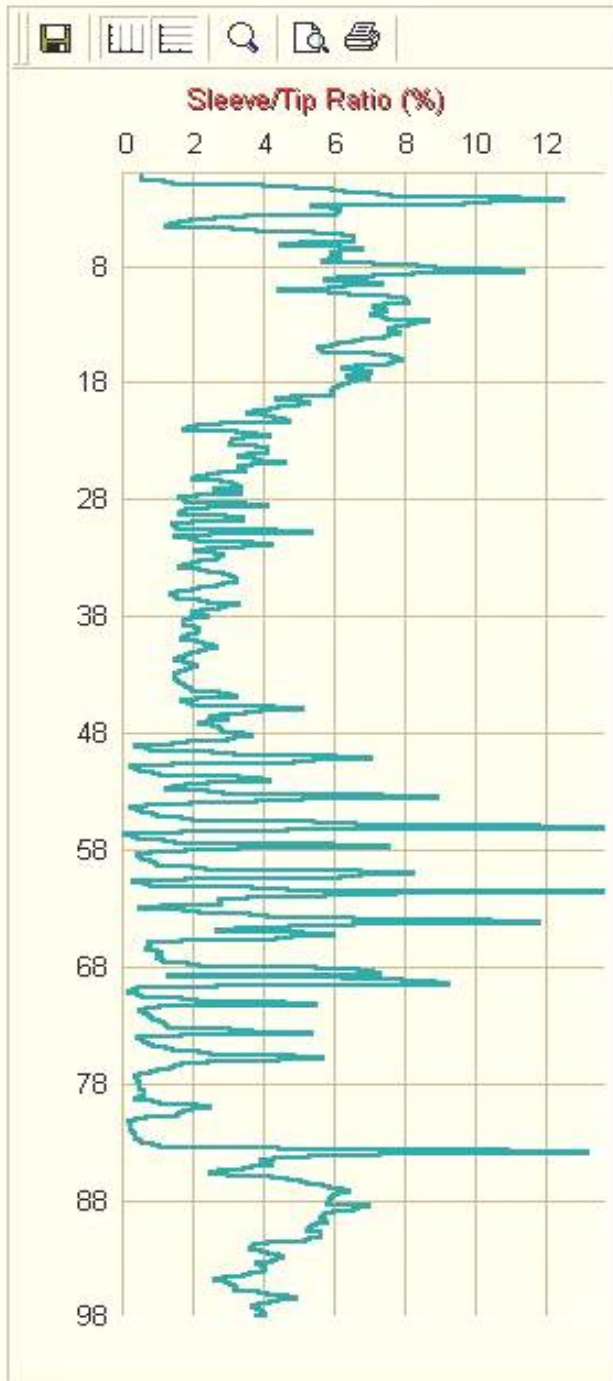
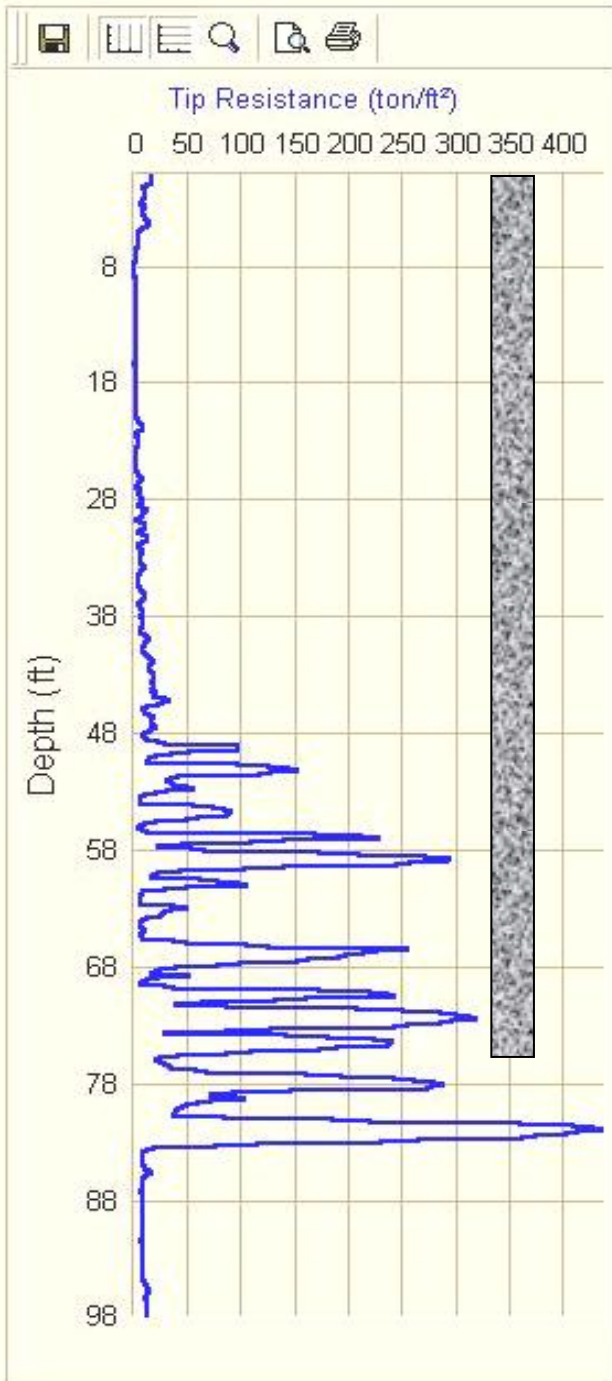


**Acceptable blow count
based on CPT**

Overdriving based on boring

Typical South Louisiana CPT log





Soil Types - Robertson86

V. Stiff Clay with layers of Sand
Clay with layers of Stiff Clayey Sand
Clay
Stiff Clayey Sand and Clay
Stiff Clayey Sand with layers of Silt Sand
Stiff Clayey Sand with layers of Sand
Stiff Clayey Sand with layers of Sand
Stiff Clayey Sand with layers of Silt Sand
Stiff Clayey Sand with layers of Sand
Sand to Clayey Sand
Stiff Clayey Sand and Gravel
V. Stiff Clay and Gravel
Gravel with layers of V. Stiff Clay
Gravelly Sand to Sand
Gravel with layers of Stiff Clayey Sand
V. Stiff Clay with layers of Stiff Clayey Sand

LTRC Research Project

Evaluation of Bearing Capacity of Piles From Cone Penetration Test Data 1998

Principal Investigator: Hani Titi, Ph.D, P.E.

Co-Principal Investigator: Murad Abufarsakh, Ph.D., P.E.

Project Manager: Mark Morvant, P.E.

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DOTD Test Piles

- **Precast concrete piles with CPT data**
 - 60 piles identified
- **Test loaded to failure**
 - 40 piles identified
- **Skin friction is the major component**
 - 34 piles analyzed
- **Implementation Phase (2001)**
 - 17 piles added to database
 - Total 51 test piles analyzed

Load Capacity Methods

- **Load Test Results**
- **Traditional Static Analysis**
 - Alpha method (Tomlinson)
- **CPT Methods:**
 - Schmertmann
 - de Ruitter and Beringen
 - Bustamante and Ganeselli (LCPC/LCP)
 - Tumay and Fakhroo
 - Aoki and De Alencar
 - Philipponnat
 - Price and Wardle
 - Penpile

SOIL CLASSIFICATION

Soil-CPT Program

STATISTICAL APPROACH

1. Zhang, Z., and Tumay, M.T., “Statistical to Fuzzy Approach Toward CPT Soil Classification”, JGE, ASCE, Vol. 125, No. 3, 1999.

EXPERIMENTAL APPROACH

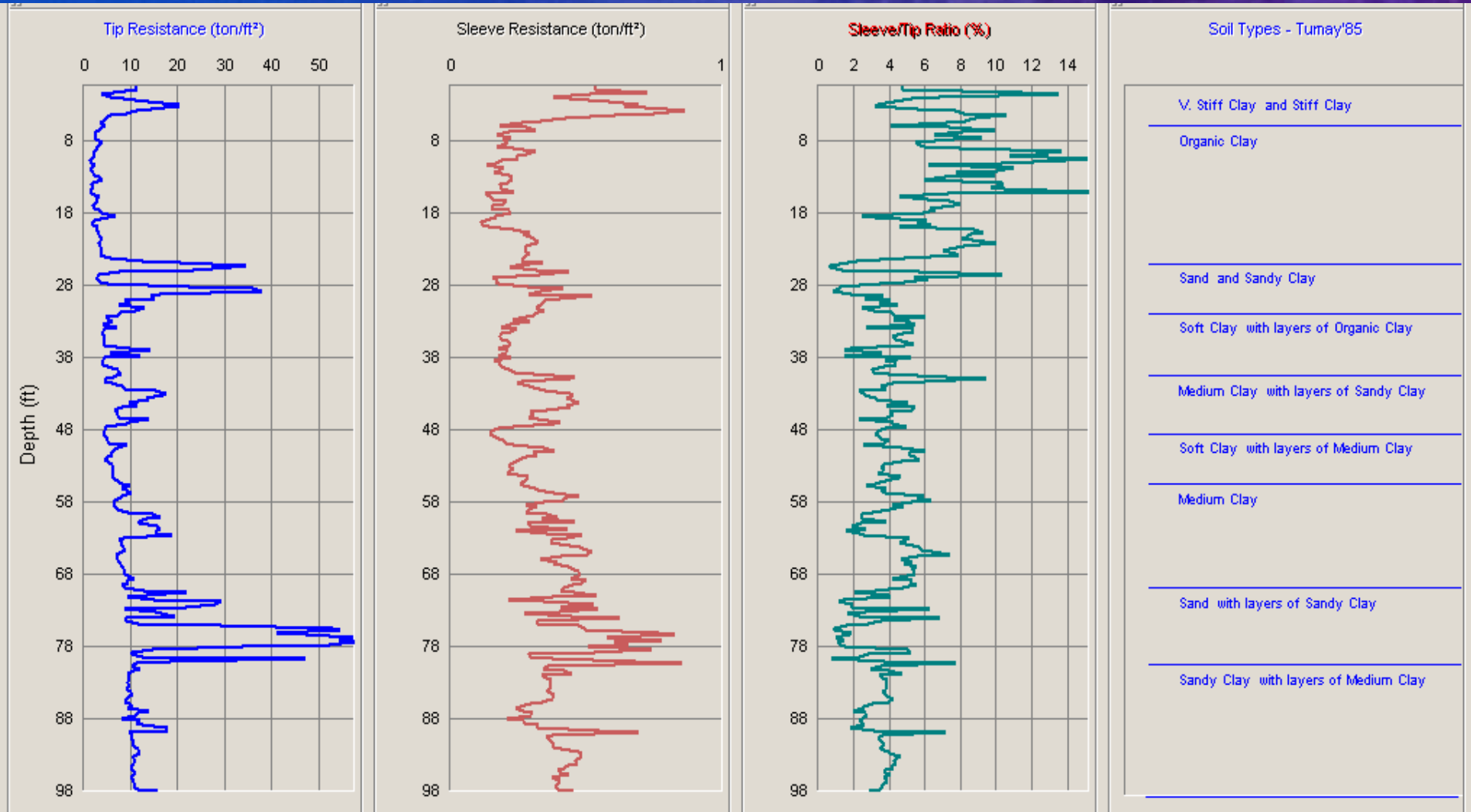
2. Tumay, M., Report No. FHWA/LA/LSU-GE85-02, LTRC, 1985
3. Robertson, P.K., Campanella, R.G., Gillespie, D., and Greig, J., “*Use of Piezometer Cone Data*”, ASCE, 1986

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Soil Classification: General Soil Type

Tumay, M.T (Modified Schmertmann)



Project Number: 829-10-0013

Project Title: Bayou Lafourche Bridge @ Clotilda

Station: 125+50

Offset: 35' rt

Elevation (ft): 15

Parish: Lafourche

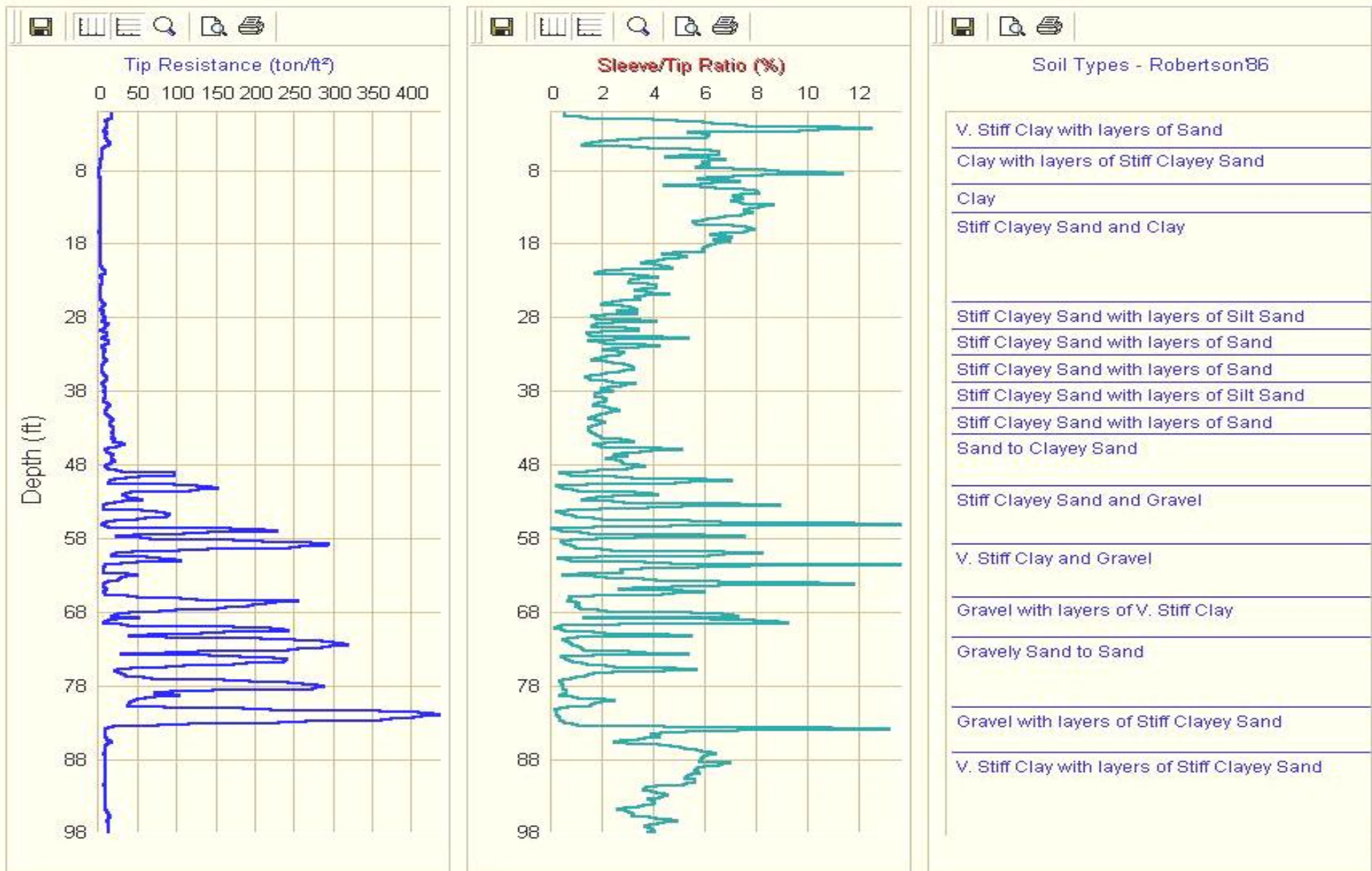
Date: 5/25/99

Latit.:

Longit.:

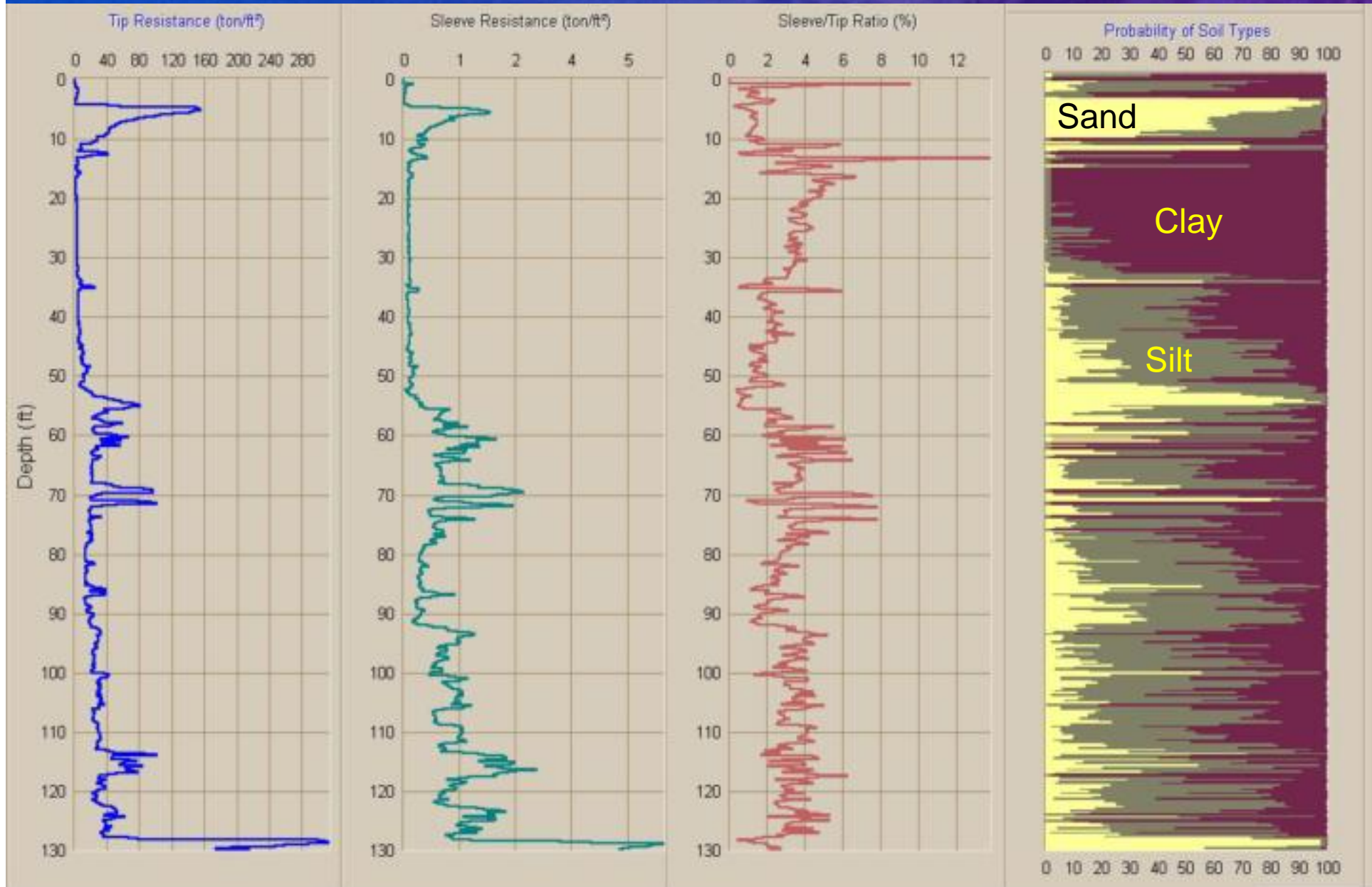
Soil Classification: General Soil Type

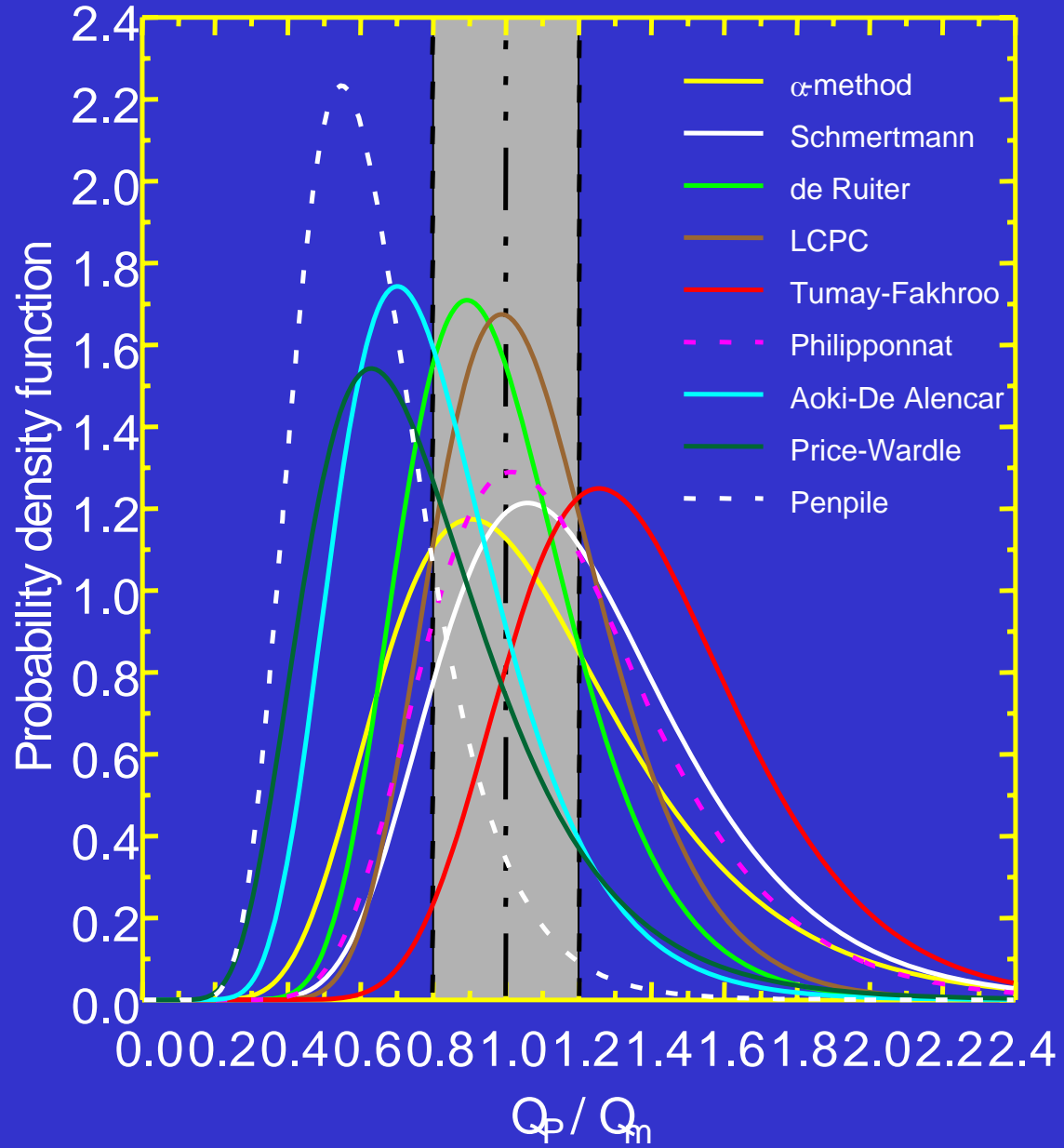
Robertson (no pore pressure)

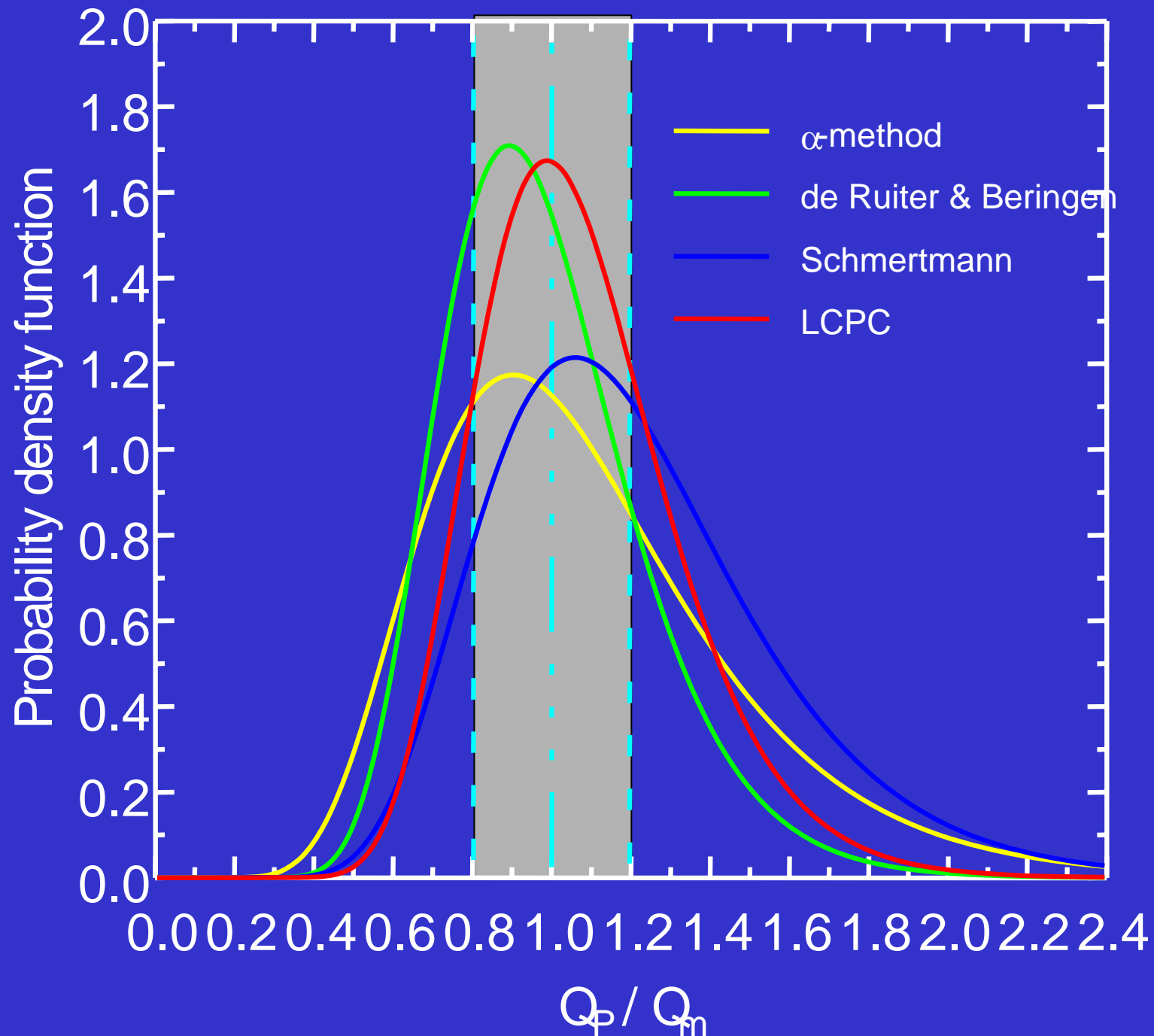


Soil Classification: Statistical Method

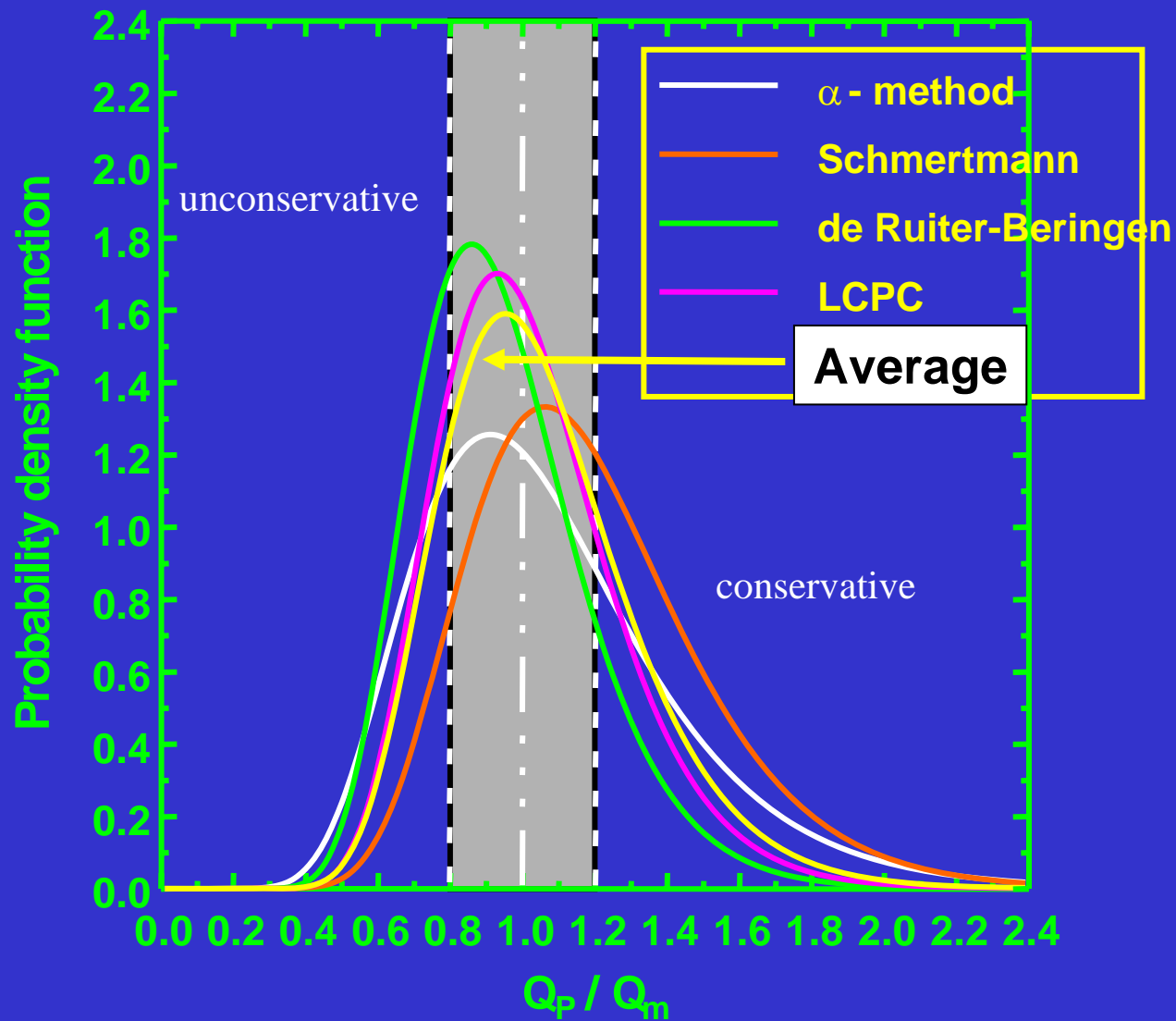
Zhang, Z., and Tumay, M.T



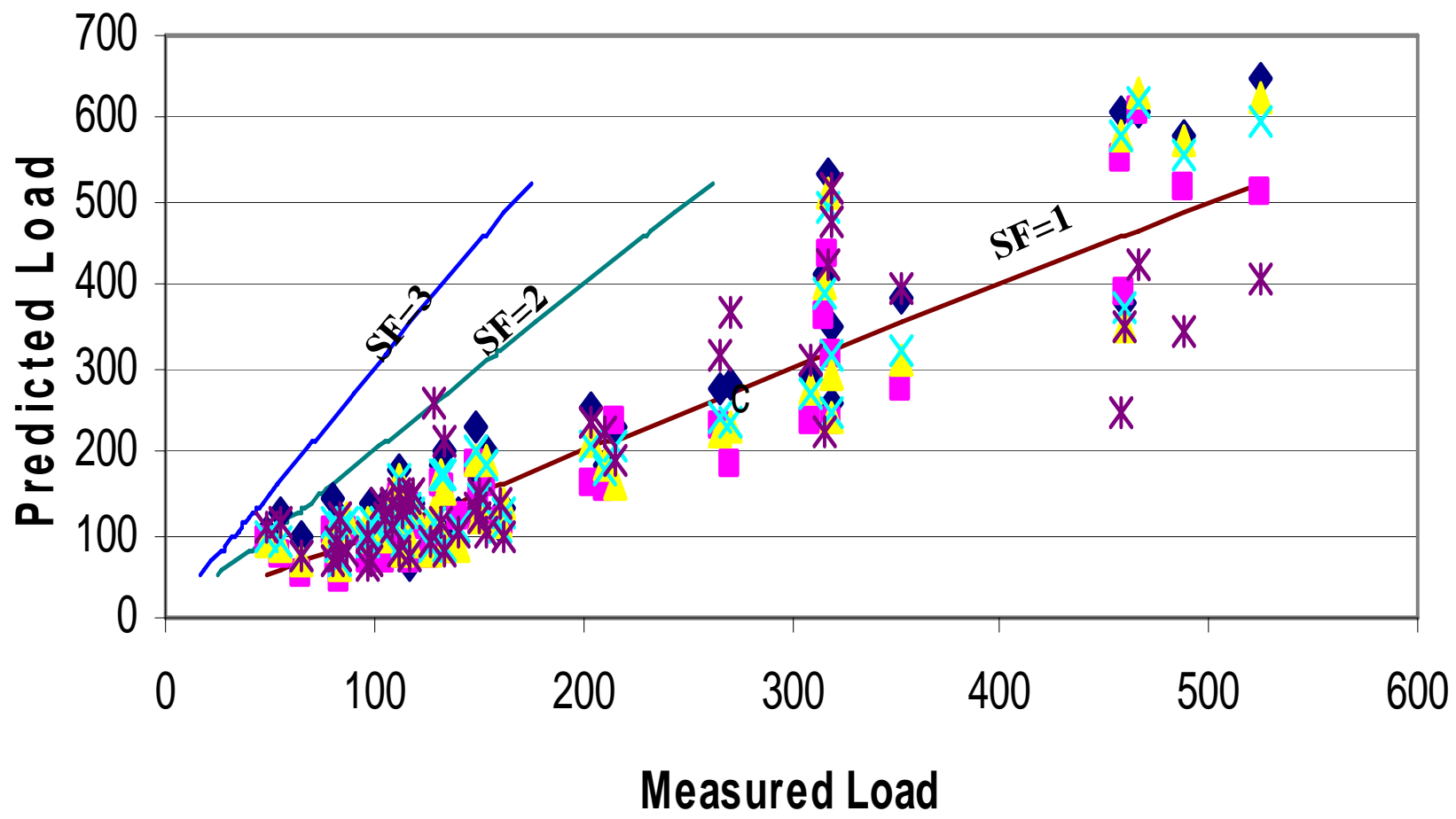




Statistical Analysis: Including Average of Methods



◆ Schmertman ■ deRueter ▲ LCPC × Ave * Alpha



DOTD Design Policy for Piles

- **AASHTO Design Codes**

 - WSD (foundations)

 - LFD (structures)

 - LRFD (proposed)

- **Safety Factor on Load Capacity (WSD)**

 - 3.0 without test piles

 - 2.0 with test pile verification

- **Current Policy: CPT for Design**

 - CPT included into bid plans

 - Plan length determined by CPT Average of Methods

 - Boring Data Correlation

 - Test Pile Correlation

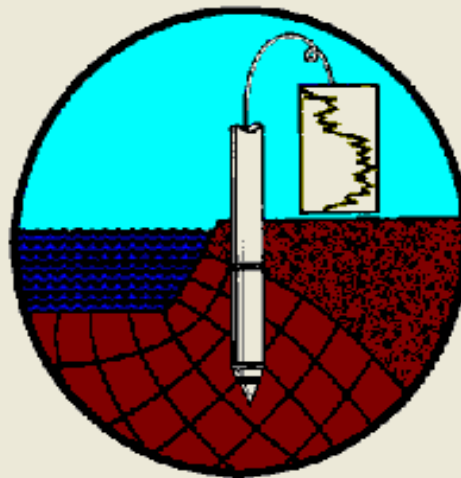
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Pile Design Program

Louisiana Transportation Research Center

LOUISIANA PILE DESIGN by CONE PENETRATION TEST PROGRAM



Version 2.3

Available for download: www.ltrc.lsu.edu

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<http://www.ltrc.lsu.edu>

Open Cone Data File

Open | List | About

View Data File

Open Data File

Plot Cone Data

m-MilliVolts

English Units

SI Units

Number of Lines: 10

Pile Design - Cone Penetration Analysis Program

Notes on Reading Cone Data Files:

- The program reads cone data files in ASCII format with extensions *.dat, *.txt, and *.prn
- Cone data in each line should be in the order: depth, tip resistance, and sleeve resistance
- The readings in each line can be separated by either commas or spaces
- The program discards other readings in the line such as pore pressure readings when a piezocone is used. If the data file was scanned, data should be modified to contain only one depth reading.
- Before plotting the data, the user should specify the number of data lines in the file (Default = 10 data lines)
- The user should specify the units of the data (default = English Units)
The SI unit of Tip and Sleeve resistance is MPA (MN/m²) and the English unit is TSF (tonf/ft²)

Start | Favorites: Mark Morvant ... | Microsoft PowerPoint | Open Cone Data File | 10:11 AM

E:\CPT files\MonroeCpt2.txt

```
05-01-2000
3:16pm
JD STA # 111+10 ON CL
RM TESTPILE # 1
JL N 18 ST. EXT. SEG 2
JN 742-37-0010
ID F5CKEV539
EL 68.2
GW 125TKN

0.02 0.0110 0.0024
0.04 0.0195 0.0024
0.06 0.0244 0.0061
0.08 0.0293 0.0049
0.10 0.0281 0.0073
0.12 0.0281 0.0342
0.14 0.0269 0.0635
0.16 0.0269 0.0867
0.18 0.0244 0.1160
0.20 0.0244 0.1355
0.22 0.0232 0.1550
0.24 0.0220 0.1697
0.26 0.0208 0.1660
0.28 0.0195 0.1611
0.30 0.0208 0.1550
0.32 0.0208 0.1453
0.34 0.0220 0.1367
0.36 0.0208 0.1282
0.38 0.0208 0.1233
0.40 0.0208 0.1221
```

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sequential, or other similar damages arising out of, or
inability to use the software or the information
anying documentation, even if the LaDOTD/LTRC has
damages, or of claims by any other party.

View Data File

Open Data File

Plot Cone Data

Units in Data File

- m-MilliVolts
- ft-tsft
- m-MPa

Num. of Title Lines

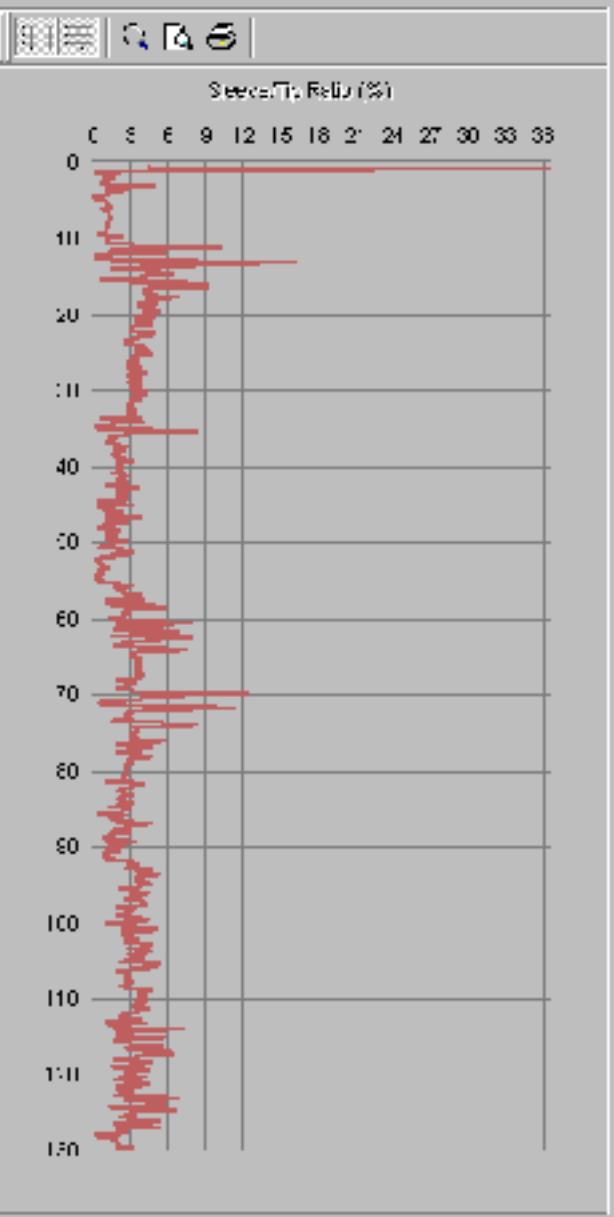
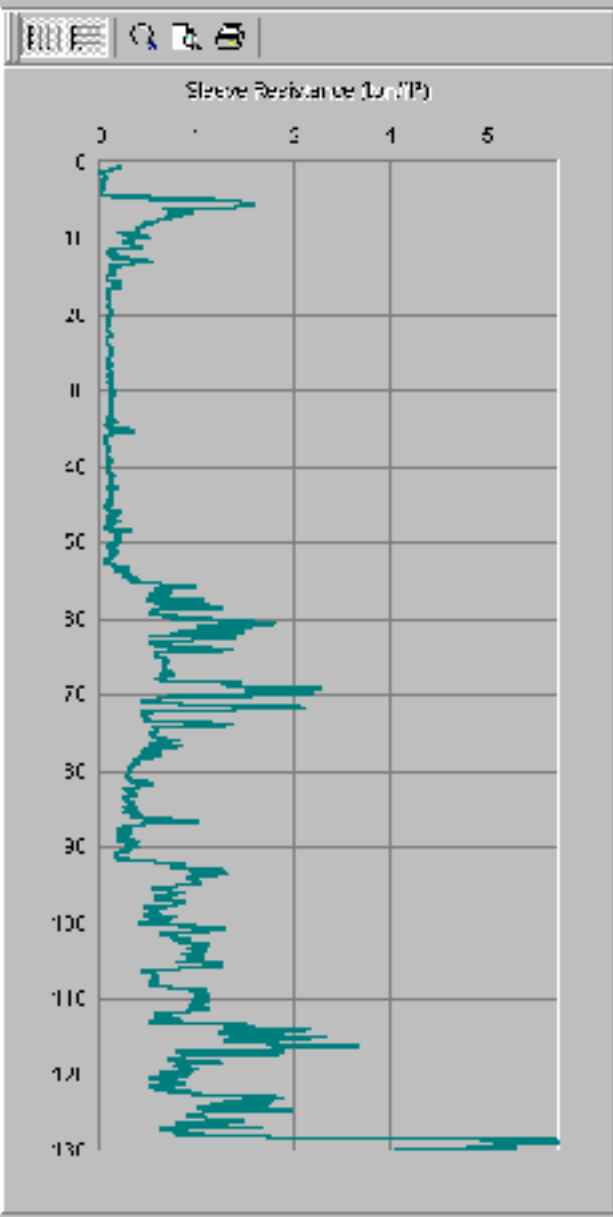
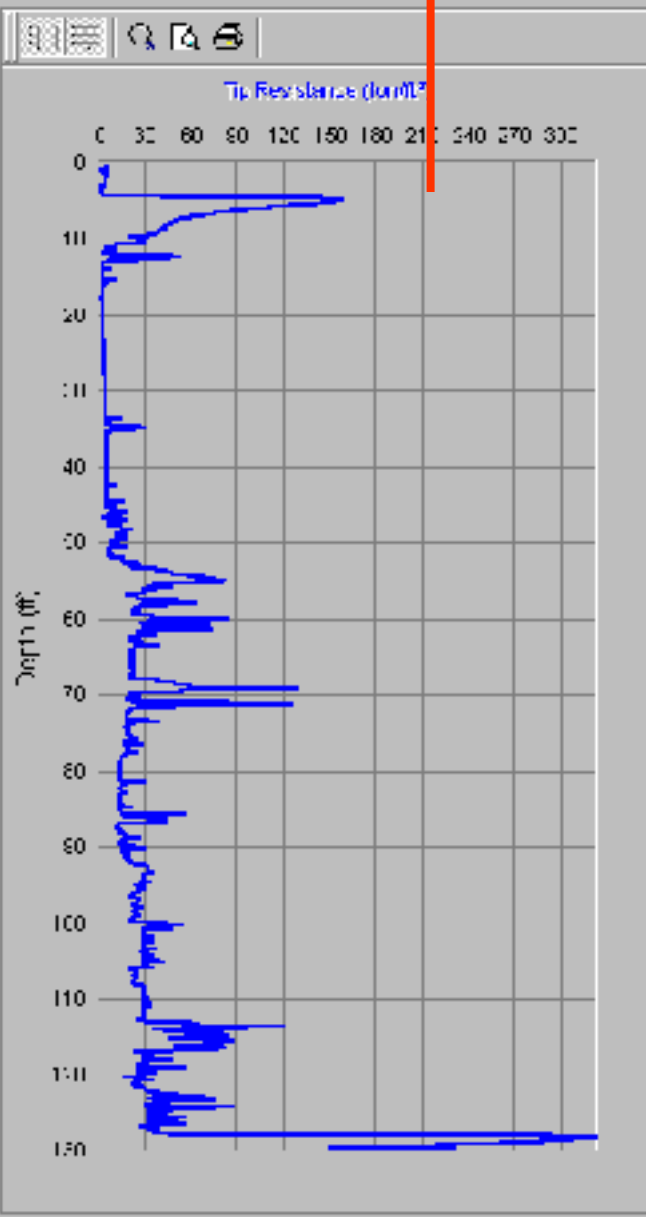
Does not use pore pressure

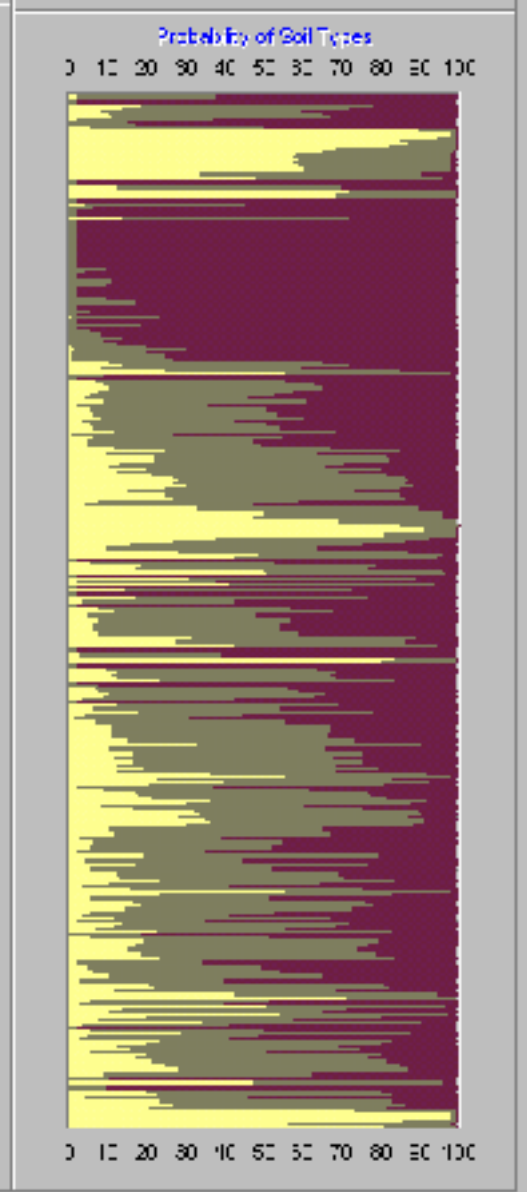
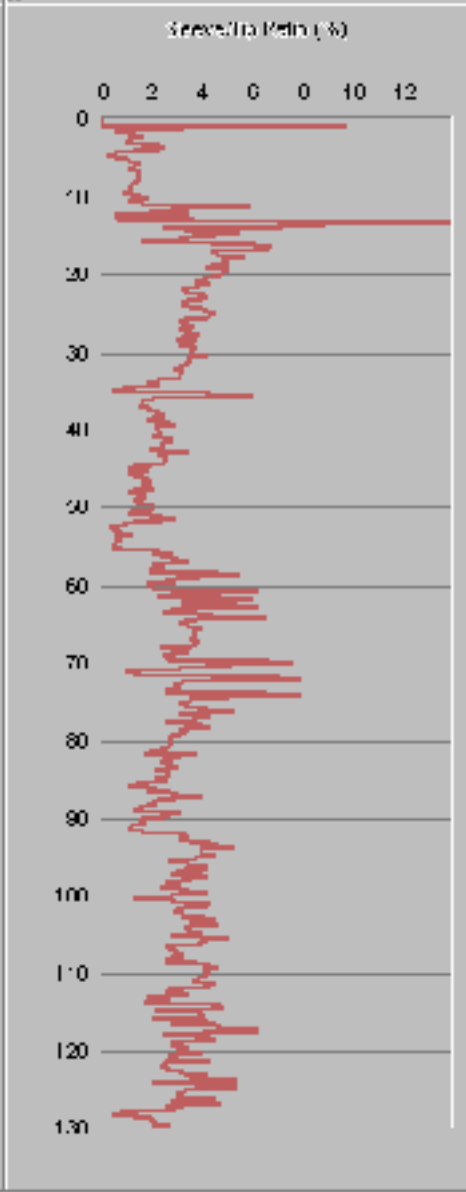
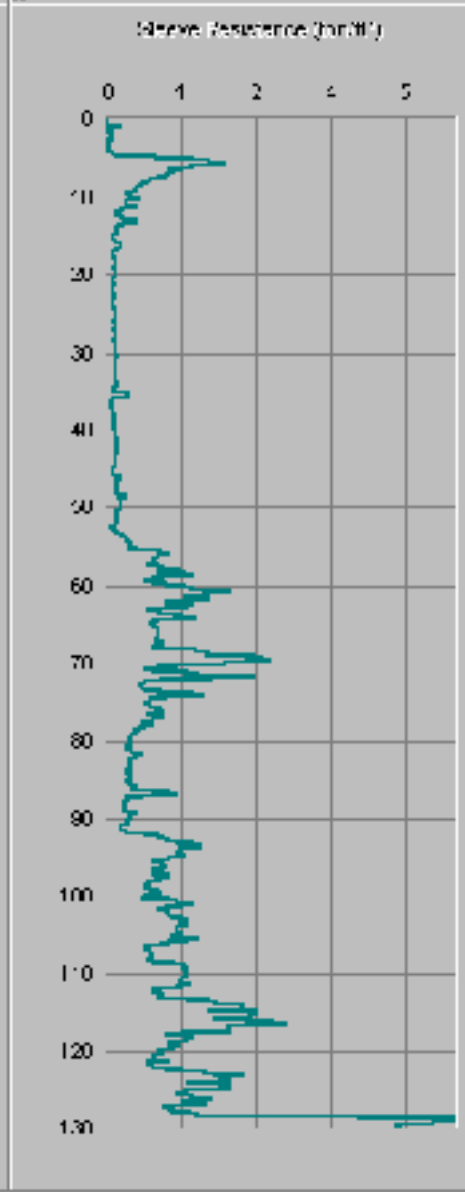
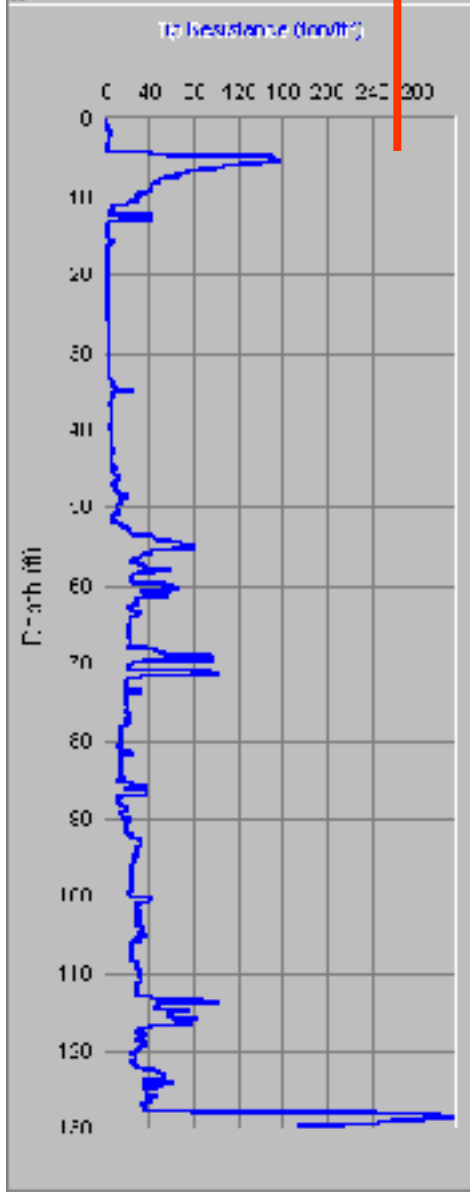
Sleeve

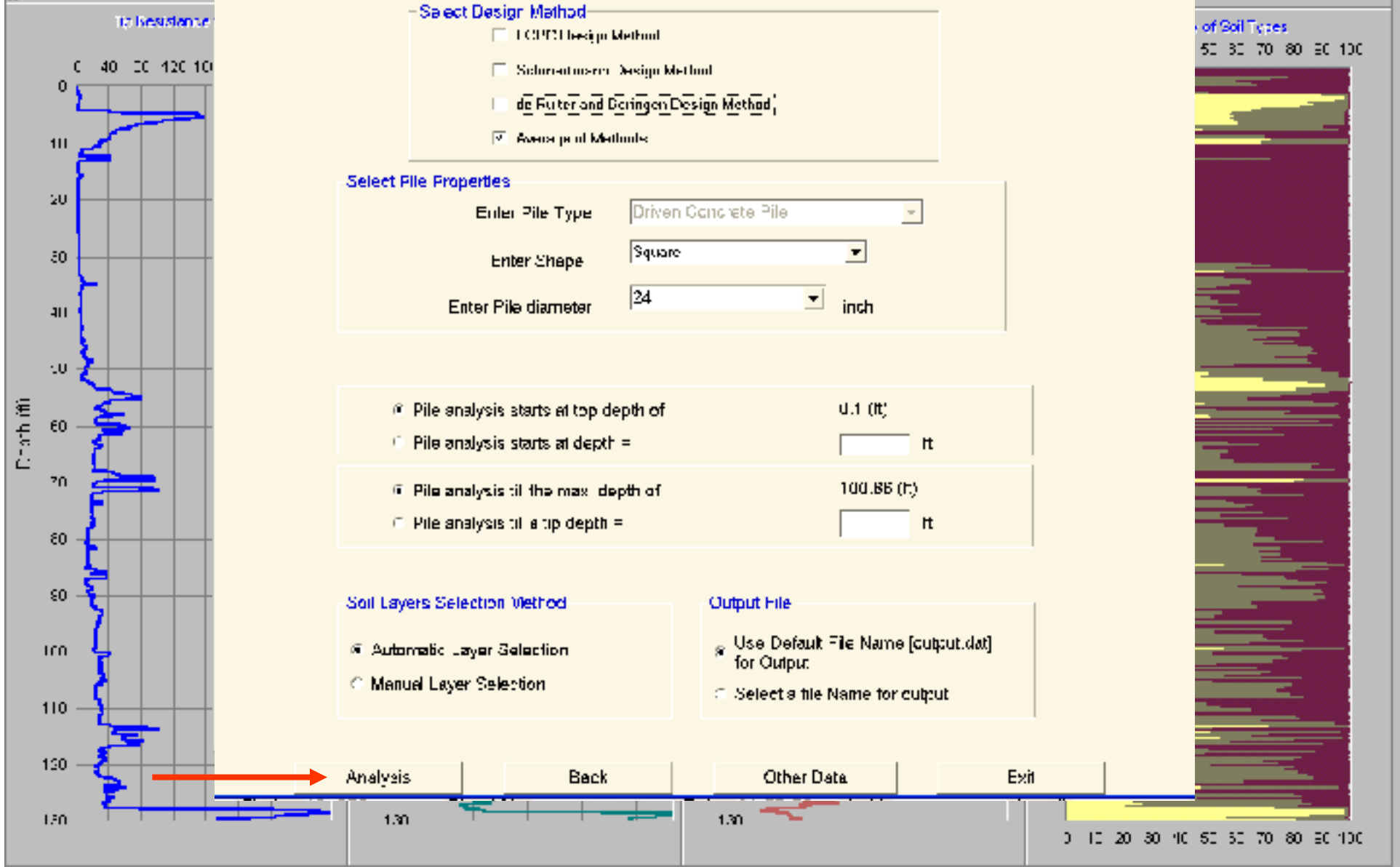
Tip

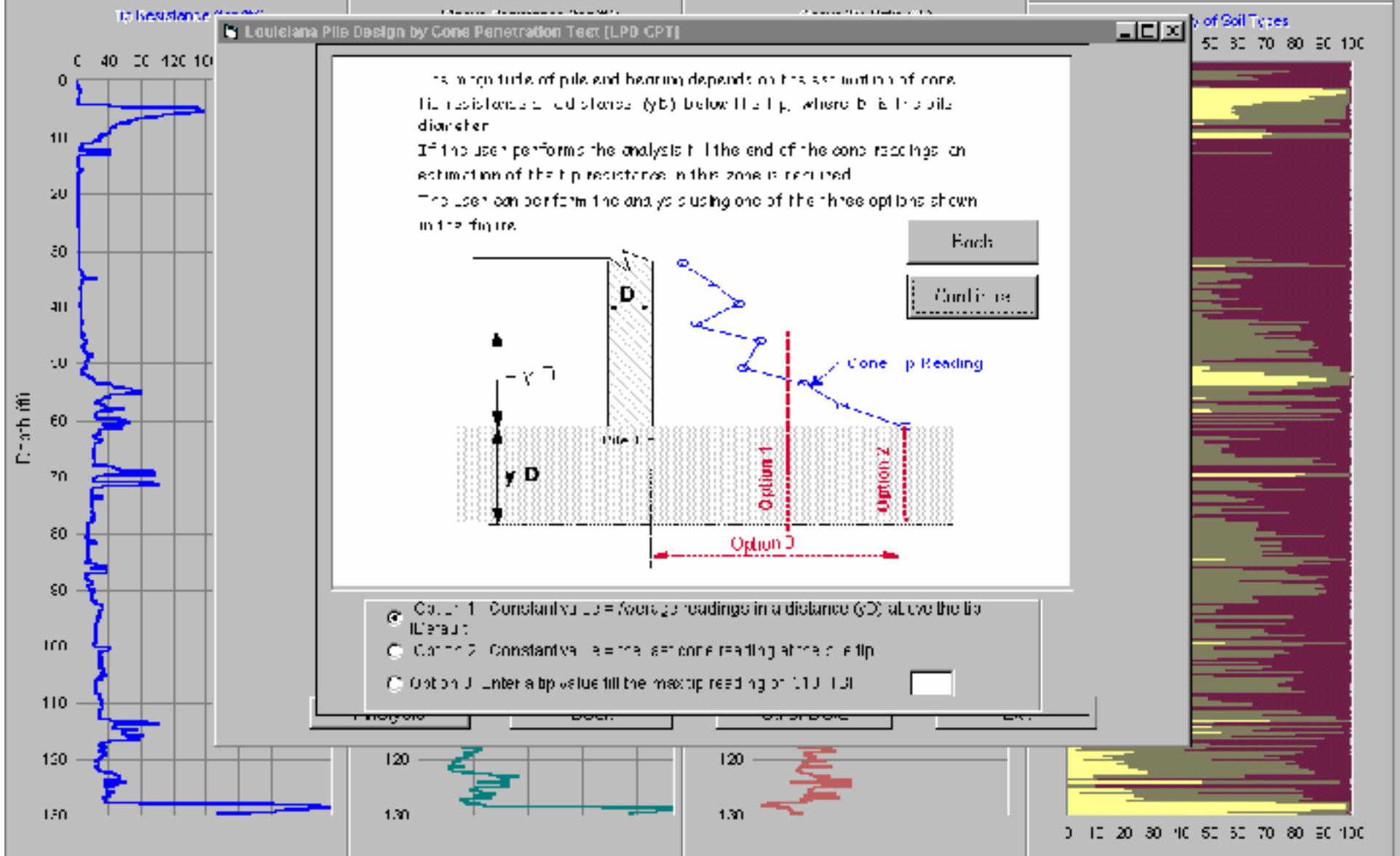
Depth

Louisiana Pile Design by Cone Penetration Test Program









Louisiana Pile Design by Cone Penetration Test [LPD-CPT]

The magnitude of pile end bearing depends on the penetration of cone tip resistance at a distance (yD) below the tip, where b is the pile diameter.

If the user performs the analysis at the end of the cone readings, an estimation of the tip resistance in this zone is required.

The user can perform the analysis using one of the three options shown in the figure.

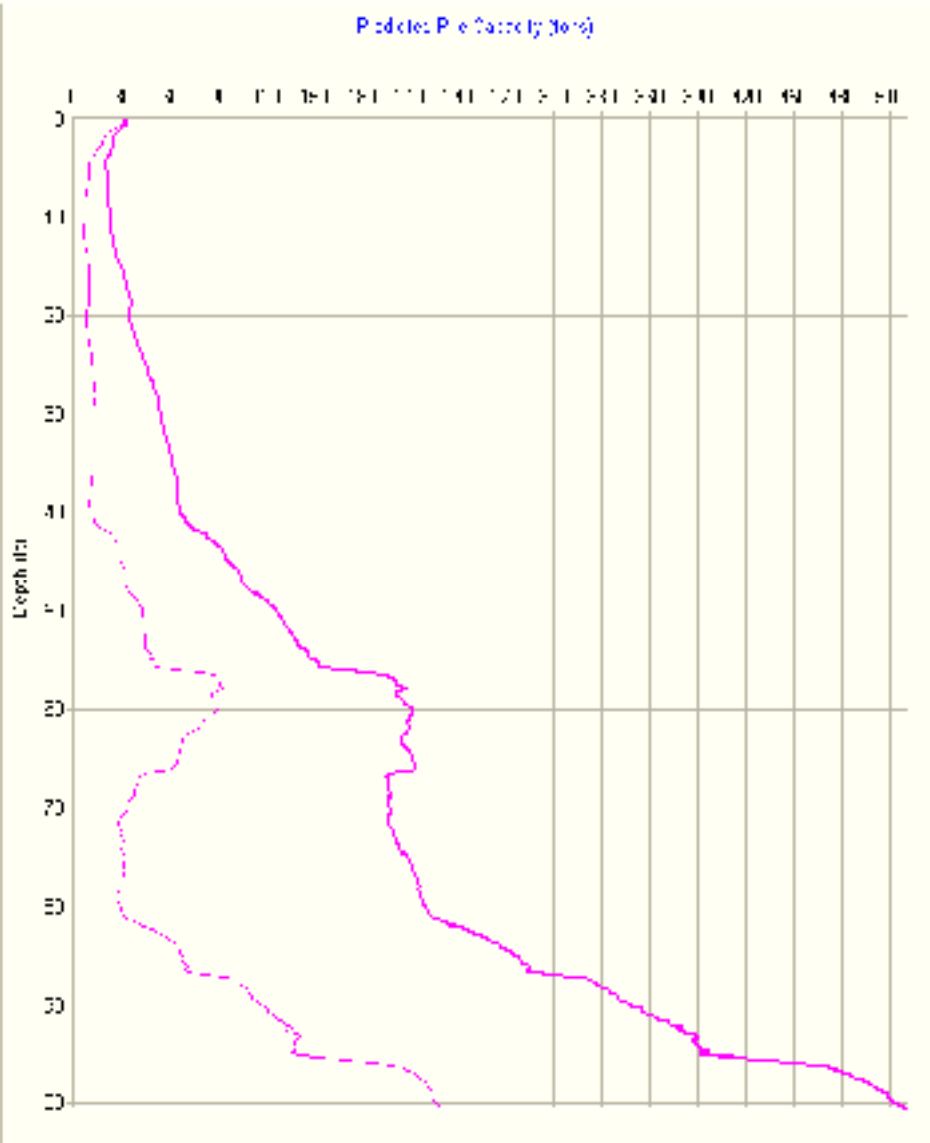
Option 1 - Constant value = average readings in a distance (yD) above the tip (Default)

 Option 2 - Constant value = the last cone reading above the tip

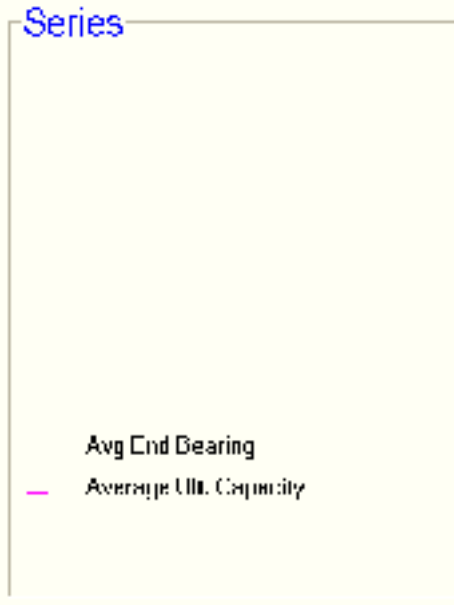
 Option 3 - Enter a tip value till the max tip reading of CPT log

Pile Shape: Square

Pile Diameter: 24



- ### Design Method
- LCPC Method
 - Schmertmann Method
 - de Ruiter and Beringen Method
 - Average of Methods
- ### Pile Capacity
- End Bearing
 - Frictional Capacity
 - Ultimate Capacity



Bayou Lafourche Bridge at Clotilda Mathews, LA Lafourche Parish

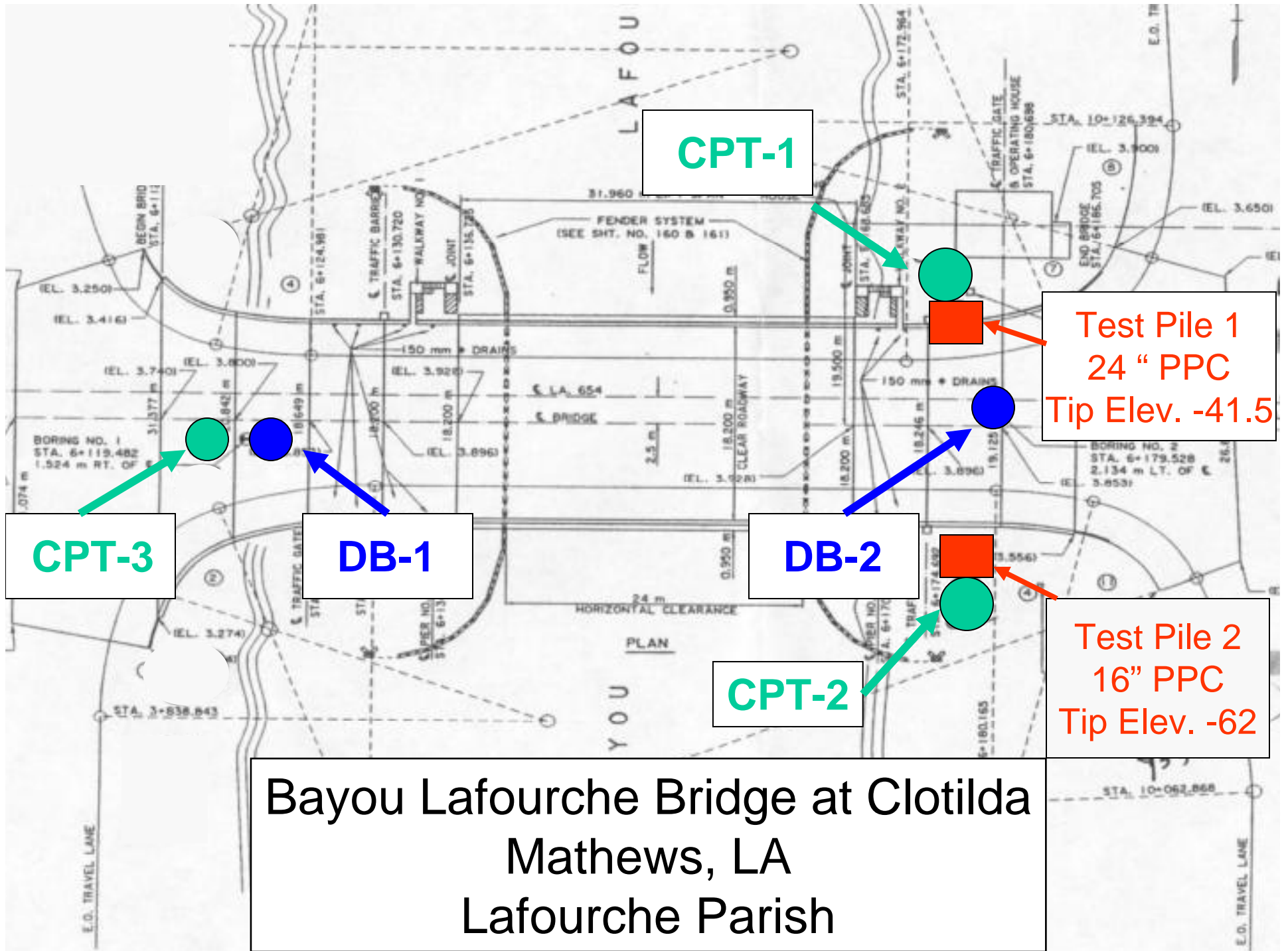


Foundation:	24 " Test Pile	16" Test Pile
Required Load:	105.6 Tons	102.2 Tons
Tip Elevation:	-41.5	-62.2
Load Results:	66 Tons	> 154 Tons (Did not fail)

CPT Application: Correlate with load test to determine pile order lengths

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Bayou Lafourche Bridge at Clotilda Mathews, LA Lafourche Parish

GR. SA. 086-102	117	28	4	12	0.82		YLD	①	10
GR. SA. 086-102	114	27	7	12	0.82		YLD	①	10
GR. SA. 086-102	118	-	25	1		0.78	M.S.	②	15
GR. SA. 086-102	117	-	29	4		0.75	M.S.	③	20
GR. ELASTIC SI. W/ TR. ORG.	116	30	28	27	0.72		YLD	④	20
GR. CL. W/ TR. ORG.	117	43	21	27	0.72		YLD	⑤	25
GR. SA. 086-102	118	-	N.	P.		0.77	YLD	⑥	25
GR. SA. 086-102	115	-	N.	P.		0.73	M.S.	⑦	30
GR. SA. 086-102	112	34	40	13	0.82		YLD	⑧	30
GR. ELASTIC SI. W/ TR. ORG.	103	34	21	23	0.58		YLD	⑨	35
GR. LEAN CL. W/ TR. ORG.	117	31	47	24	0.60		YLD	⑩	40
GR. ELASTIC SI.	74	54	83	42	0.78		YLD	⑪	45
GR. SA. 086-102	122	-	N.	P.		0.74	M.S.	⑫	45
GR. STR. GR. SA. W/ TR. ORG.	116	-	N.	P.					50
GR. LEAN CL.	116	45	39	18	0.50		M.S.	⑬	50
GR. CL. W/ TR. ORG.	112	47	74	42	1.18		YLD	⑭	55
GR. SA. 086-102	111	43	78	47	0.85		YLD	⑮	60
GR. SA. W/ TR. ORG.	121	-	N.	P.					65
GR. CL. W/ TR. ORG.	113	45	77	45	1.13		YLD	⑯	70
GR. SA. 086-102	118	-	N.	P.		0.71	M.S.	⑰	75
GR. CL.	111	37	83	47	0.83		YLD	⑱	80
GR. ELASTIC SI.	108	51	26	57					85
GR. CL. W/ TR. ORG. PSA	110	44	74	44	1.07		YLD	⑲	90
GR. CL. W/ TR. ORG. 10H.	113	37	-	-	1.24		M.S.		95
GR. ELASTIC SI. W/ TR. ORG.	102	51	83	45	1.32		YLD	⑳	100
GR. STR. + LOT. NO.	95	73	79	33	2.41		YLD	㉑	105
GR. CL. W/ TR. ORG.	117	52	67	28	1.25		YLD	㉒	110
GR. SA. W/ TR. SA. + TR. ORG.	118	-	32	5		0.68	M.S.	㉓	115
GR. SA. 086-102	-	-	N.	P.					120
GR. CL. W/ TR. ORG.	104	63	82	4	1.25		YLD	㉔	120
GR. SA. W/ TR. ORG.	115	-	N.	P.					130
GR. ELASTIC SI. W/ TR. ORG.	109	42	63	27					130
GR. CL. W/ TR. SA.	106	48	80	47	1.71		YLD	㉕	130
GR. ELASTIC SI. W/ TR. ORG.	103	47	-	-	1.5		YLD	㉖	130
GR. ELASTIC SI. W/ TR. ORG.	106	37	66	47	2.13		YLD	㉗	150



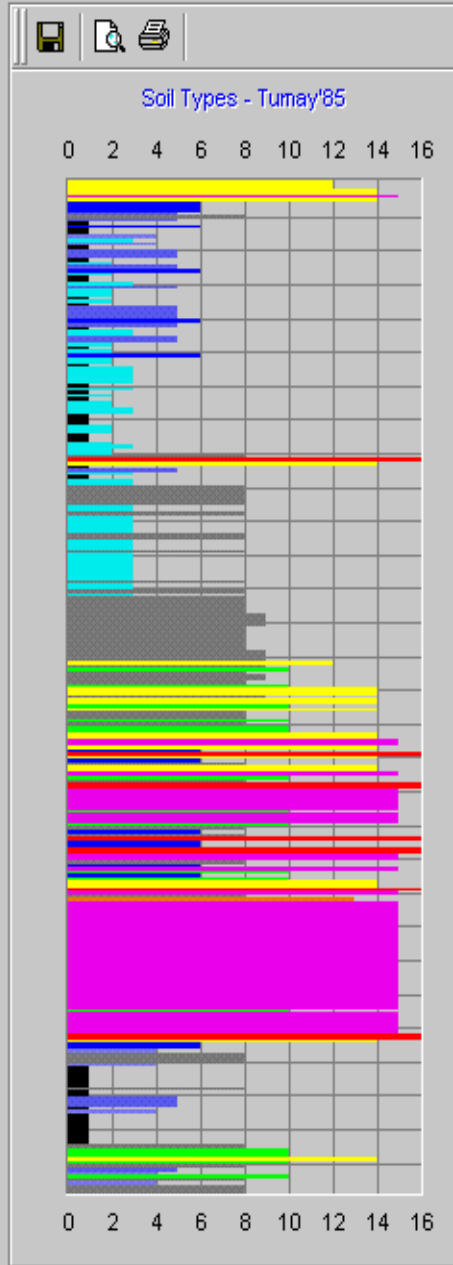
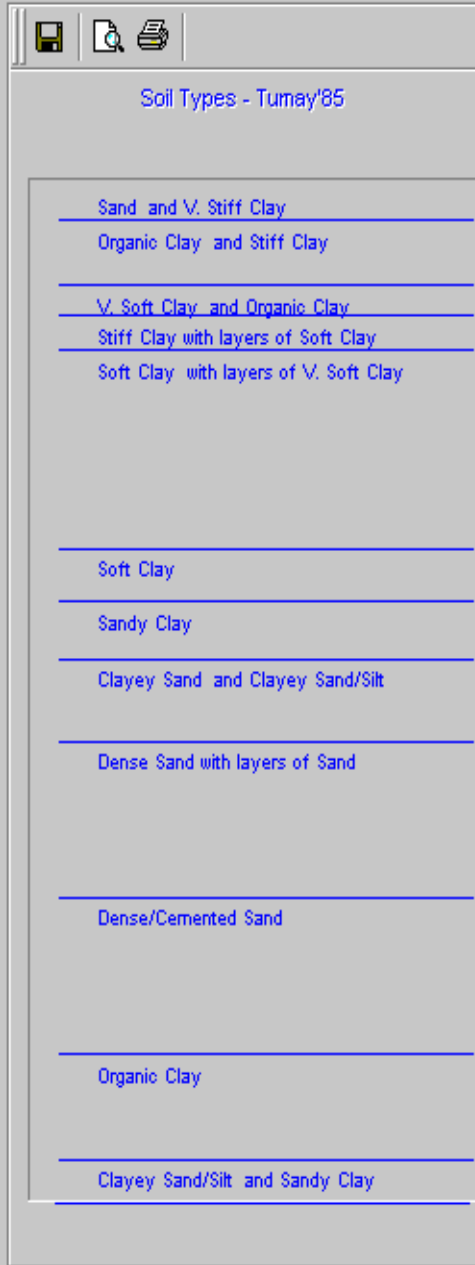
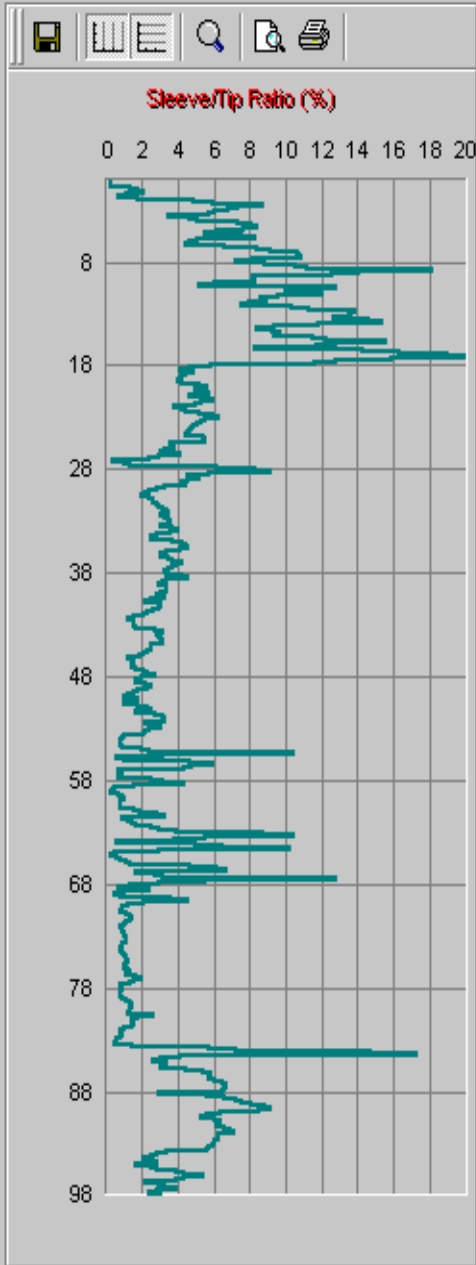
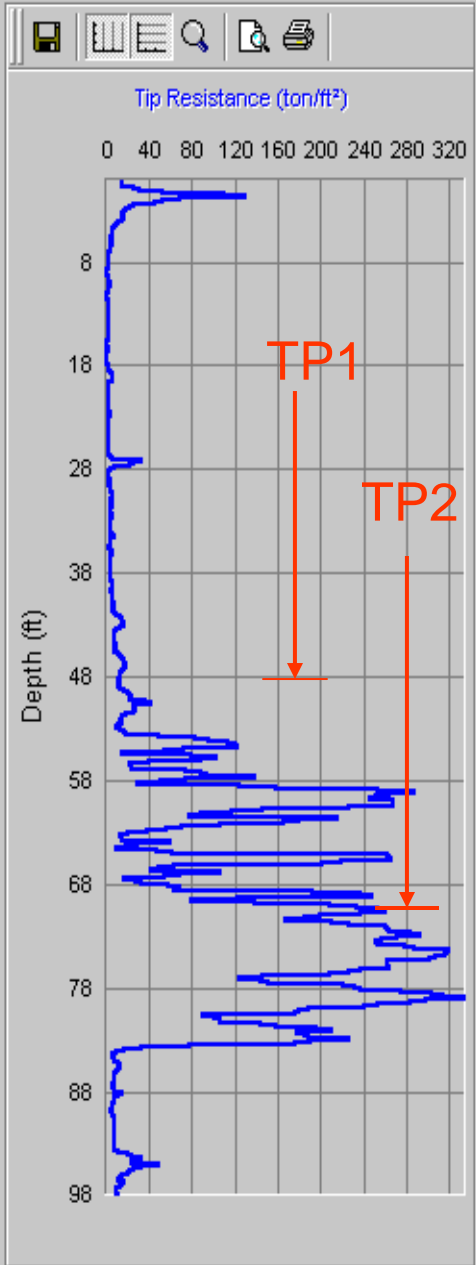
TP 1

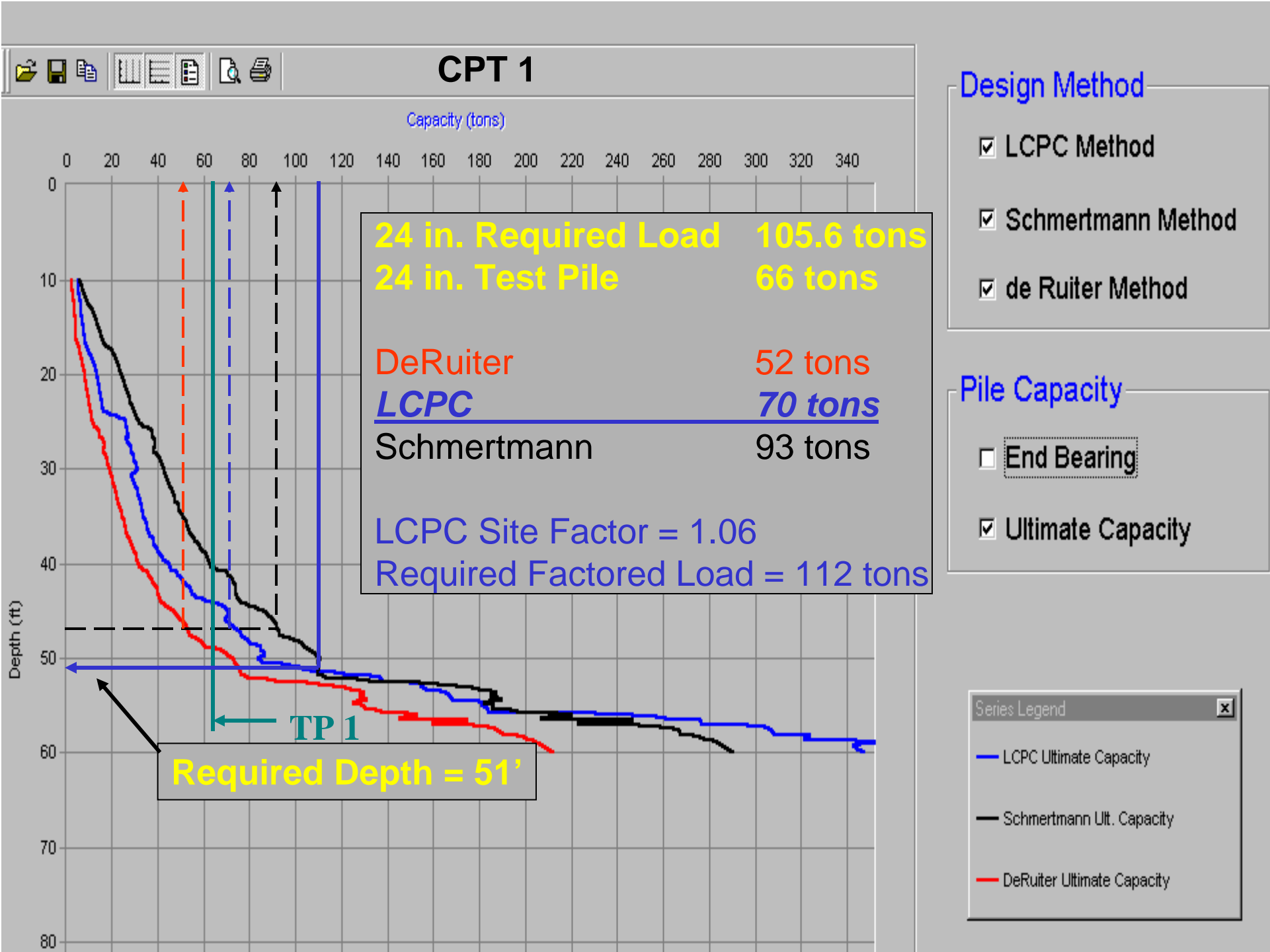


TP 2

GR. SA. 086-102	122	-	30	5		0.68	M.S.	①	10
GR. LEAN CL.	117	-	32	7	0.97		YLD	②	10
GR. SA. 086-102	120	-	36	11		0.85	M.S.	③	15
GR. SA. 086-102	117	28	37	13	0.63		YLD	④	15
GR. SA. 086-102	117	28	40	13	0.57		YLD	⑤	20
GR. SA. 086-102	117	34	37	13	0.47		YLD	⑥	25
GR. CL. W/ TR. ORG.	113	34	67	38	0.42		YLD	⑦	25
GR. LEAN CL. W/ TR. ORG.	114	47	47	20	0.47		YLD	⑧	30
GR. SA. 086-102	117	-	N.	P.		0.77	M.S.	⑨	30
GR. SA. 086-102	117	-	44	6		0.44	M.S.	⑩	35
GR. STR. GR. CL. W/ TR. ORG.	112	38	63	26	0.88		M.S.	⑪	40
GR. SA. 086-102	124	-	24	1					40
GR. SA. 086-102	124	-	31	6		0.60	YLD	⑫	45
GR. SA. 086-102	121	-	23	1		0.66	M.S.	⑬	50
GR. CL. W/ TR. SA.	115	81	63	32	-	-			55
GR. SA. 086-102	114	57	70	37	0.64		YLD	⑭	60
ALT. STR. GR. CL.	76	77	86	86	0.12		YLD	⑮	65
GR. SA. 086-102	-	-	N.	P.					65
ALT. STR. GR. CL. W/ TR. ORG.	-	-	57	34					70
GR. SA. 086-102	-	-	N.	P.					75
GR. SA. 086-102	-	-	N.	P.					80
GR. SA. 086-102	-	-	N.	P.					85
GR. SA. 086-102	-	-	N.	P.					90
GR. SA. 086-102	-	-	N.	P.					95
GR. SA. 086-102	-	-	N.	P.					100
GR. SA. 086-102	-	-	N.	P.					105
GR. SA. 086-102	-	-	N.	P.					110
GR. SA. 086-102	120	-	41	14		0.68	M.S.	⑯	115
GR. CL. W/ TR. ORG.	102	65	74	40	1.18		YLD	⑰	120
GR. SA. 086-102	-	-	N.	P.					120
GR. ELASTIC SI.	107	47	75	40	1.65		YLD	⑱	130
GR. ELASTIC SI. W/ TR. ORG.	110	47	-	-	1.81		YLD	⑳	130
GR. ELASTIC SI. W/ TR. ORG.	100	57	68	28	1.70		YLD	㉑	140
GR. ELASTIC SI. W/ TR. ORG.	71	65	56	14	1.62		YLD	㉒	140
GR. ELASTIC SI. W/ TR. ORG.	101	50	56	7	1.71		YLD	㉓	150

SPT N = 10



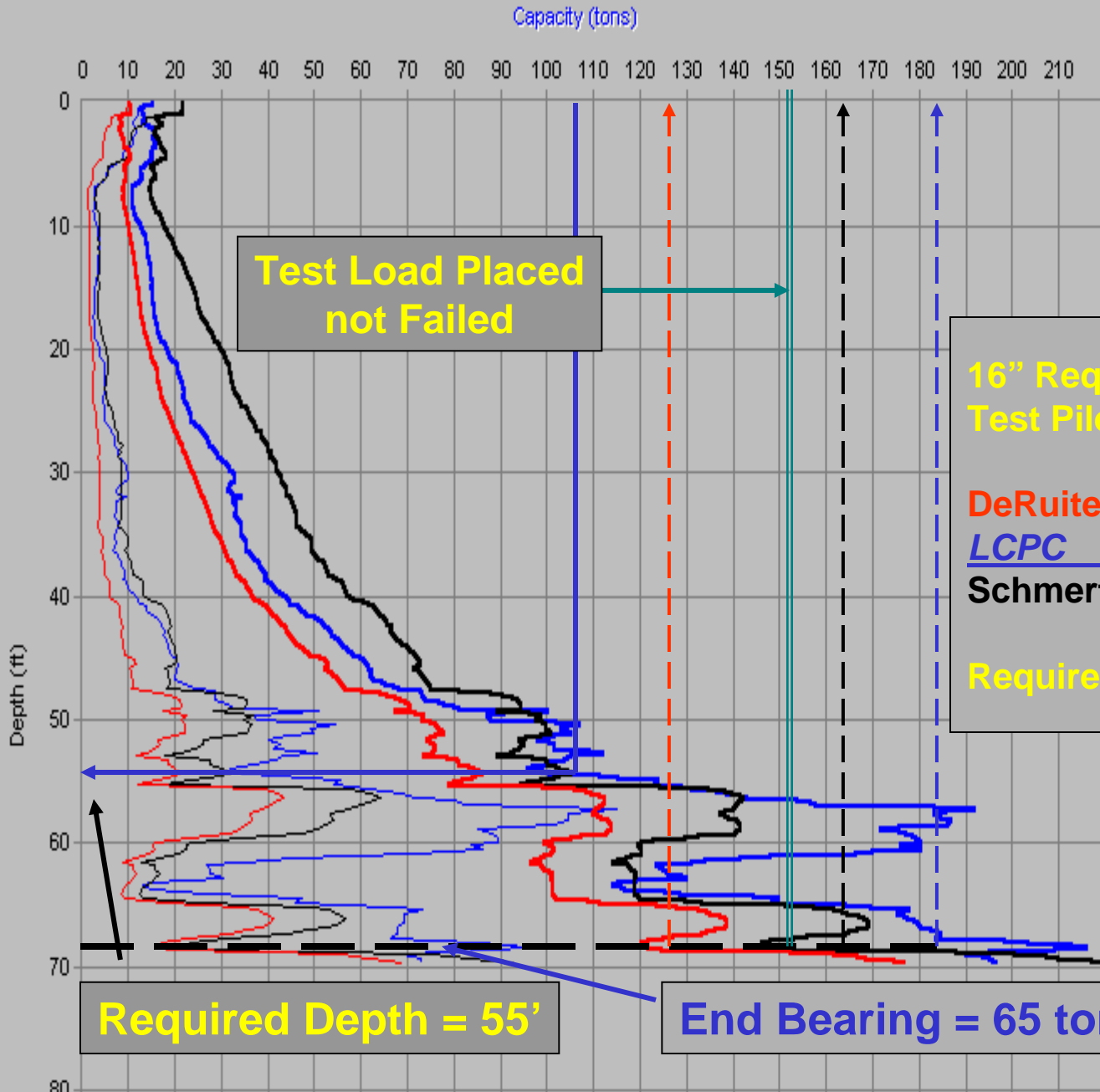




CPT 2

Design Method

- LCPC Method
- Schmertmann Method
- de Ruiter Method



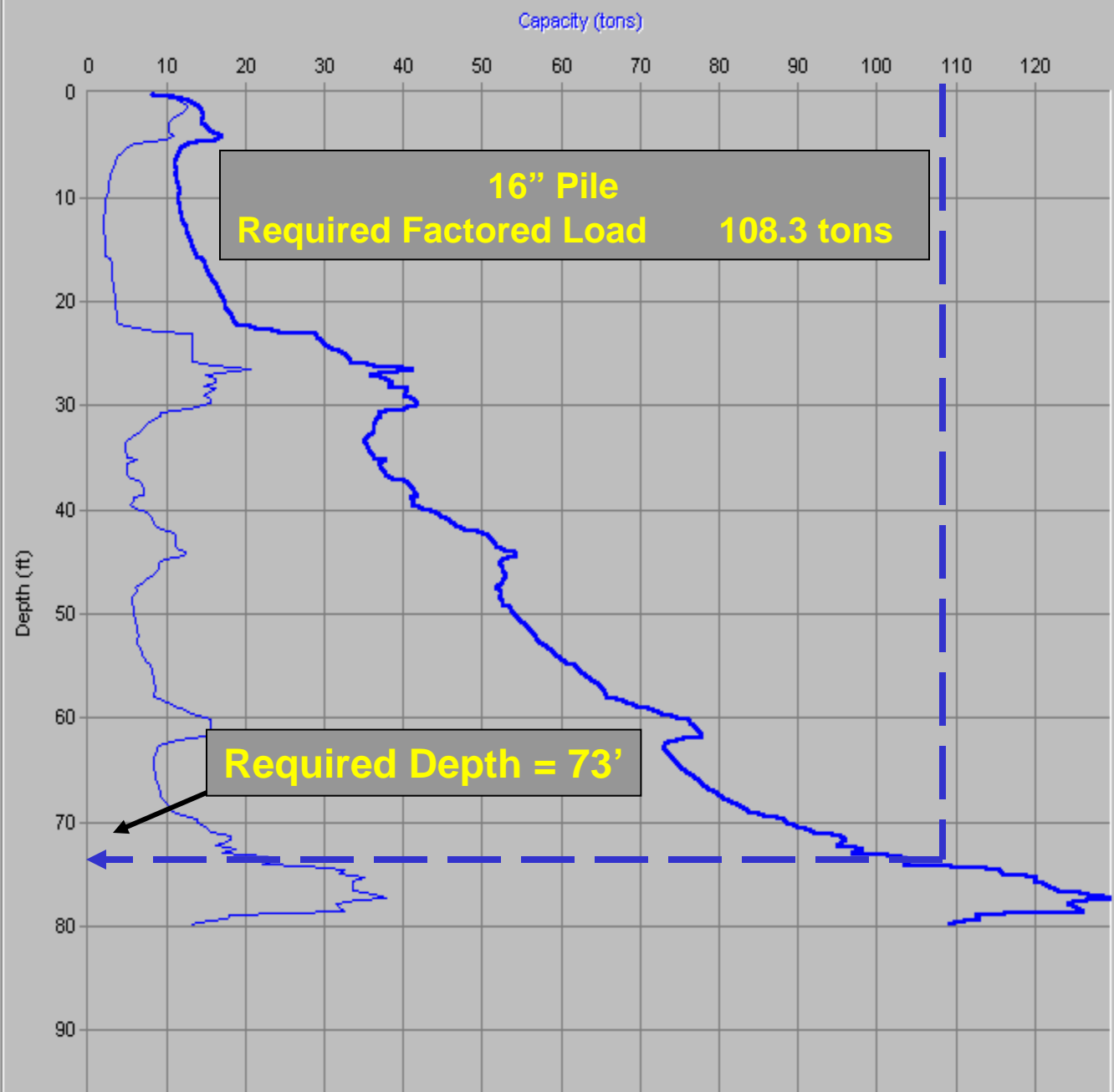
16" Required Load	102.2
Test Pile 2 Load	>154 tons
DeRuiter	125 tons
LCPC	188 tons
Schmertmann	163 tons
Required Factored Load	108.3 tons

Series Legend

- LCPC End Bearing
- LCPC Ultimate Capacity
- Schmertmann End Bearing
- Schmertmann Ult. Capacity
- DeRuiter End Bearing
- DeRuiter Ultimate Capacity



CPT 3



Design Method

- LCPC Method
- Schmertmann Method
- de Ruiter Method

Pile Capacity

- End Bearing
- Ultimate Capacity

Series Legend

- LCPC End Bearing
- LCPC Ultimate Capacity

Back

CPT vs. Conventional Drilling Method

Deep Foundations

	Conventional Soil Exploration	CPT
Manpower	Crew: 4-5	Crew: 2-4
Time	Field: 6-8 / week Lab: 2-4 weeks	6-8 / day
Equipment	Drilling rig 3 support vehicles Lab equipment	CPT truck 1 support vehicle

CPT vs. Conventional Drilling Method (1998)

Typical Cost		
	Conventional Soil Exploration	CPT
DOTD	\$45/ft - \$50/ft	\$14/ft
Consultant	\$50/ft - \$60/ft	\$28/ft

CPT Benefits

- **Cost Savings**
 - Field exploration costs savings
 - Laboratory costs savings
- **Time Savings**
- **Better subsurface stratification**
 - Finer Detail
 - More locations
- **Accuracy**
 - Reduced number of test piles
 - Shorter pile lengths



Questions ?

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