
Louisiana Transportation Research Center

Final Report 568

Development of Wave and Surge Atlas for the Design and Protection of Coastal Bridges in South Louisiana—Phase 2

by

D. Max Sheppard, Ph.D.
Mark S. Gosselin, Ph.D., P.E.
Philip E. Dompe, P.E.
Hüseyin Demir, Ph.D., P.E.
INTERA Incorporated



4101 Gourrier Avenue | Baton Rouge, Louisiana 70808
(225) 767-9131 | (225) 767-9108 fax | www.ltrc.lsu.edu

TECHNICAL REPORT STANDARD PAGE

| | | | |
|---|--------------------------------------|---|--|
| 1. Report No. FHWA/LA.16/568 | | 2. Government Accession No. | 3. Recipient's Catalog No. |
| 4. Title and Subtitle Development of Wave and Surge Atlas for the Design and Protection of Coastal Bridges in South Louisiana – Phase 2 | | 5. Report Date September 2016 | 6. Performing Organization Code |
| 7. Author(s) D. Max Sheppard, Ph.D., Philip E. Dompe, P.E., Mark S. Gosselin, Ph.D., P.E., and Hüseyin Demir, Ph.D. | | 8. Performing Organization Report No. | |
| 9. Performing Organization Name and Address INTERA Incorporated 2114 NW 40 th Terrace, Suite A-1 Gainesville, FL 32605 | | 10. Work Unit No. | 11. Contract or Grant No. LRTC No. 15-1ST SIO No. 30000118 |
| 12. Sponsoring Agency Name and Address Louisiana Department of Transportation and Development P.O. Box 94245 Baton Rouge, LA 70804-9245 | | 13. Type of Report and Period Covered Final Report March 2016 | 14. Sponsoring Agency Code |
| 15. Supplementary Notes | | | |
| <p>16. Abstract</p> <p>This report summarizes the work performed by INTERA Incorporated (INTERA), for the Louisiana Department of Transportation and Development (DOTD) on Phase 2 of LTRC project No. 10-4ST, “Development of Wave and Surge Atlas for the Design Protection of Coastal Bridges in South Louisiana.” In Phase 1 (LTRC project No. 10-4ST, Final Report 528), a Level III storm surge/wave analysis provided the design water level and wave parameters needed to compute wave loads. This analysis entailed (1) hindcasting 50 of the most severe tropical storms and hurricanes that have affected Louisiana coastal waters over the past 160 years including hindcasting alternative paths for a select number of those storms resulting in a total of 124 hindcasts, and (2) performing extreme value analyses on water elevation and wave heights throughout the area covered by the model to obtain 100-year design meteorological /oceanographic (met/ocean) conditions. The results from the extreme value analyses provided the data for the Wave and Surge Atlas. The atlas is presented in a geographic information system (GIS) database for ease of access and use. Those data provided the input to determine the vulnerability of selected DOTD coastal bridges to design storm surge and wave loads. This phase includes (1) developing parameters for additional return periods (5-, 10-, 25-, and 50-year) from the results of the hindcasted storms in Phase 1; (2) develop a Surge/Wave Atlas for maximum values of the actual hurricane/tropical storm-induced water elevation, wave height and peak period and wind speed for the study area over the past 160 years; (3) developing a Surge/Wave Atlas for maximum values of the actual + path shifted hurricane/tropical storm-induced water elevation, wave height and peak period, and wind speed for the study area over the past 160-years; (4) Developing an AASHTO Wave Load Calculation Program (Visual Basic Program) based on the AASHTO Guide Specifications [1]; (5) providing a training session for DOTD employees so that DOTD will be able to update or modify the program as needed for future code changes; and (6) computing the forces and moments on the remaining spans on the bridges determined to be vulnerable and generate PDF files of the bridge information and the forces and moments along the entire bridge, and adding PDF pop-ups to the locations of all bridges determined to be vulnerable.</p> | | | |
| 17. Key Words Wave Forces, Storm Surge, Wave Atlas | | 18. Distribution Statement Unrestricted. This document is available through the National Technical Information Service, Springfield, VA 21161. | |
| 19. Security Classif. (of this report) | 20. Security Classif. (of this page) | 21. No. of Pages 163 | 22. Price |

Project Review Committee

Each research project will have an advisory committee appointed by the LTRC Director. The Project Review Committee is responsible for assisting the LTRC Administrator or Manager in the development of acceptable research problem statements, requests for proposals, review of research proposals, oversight of approved research projects, and implementation of findings.

LTRC appreciates the dedication of the following Project Review Committee Members in guiding this research study to fruition.

LTRC Manager

Walid R. Alaywan, Ph.D., P.E.
Sr. Structures Research Manager

Members

Jenny Fu, P.E.
Stephanie Cavalier, P.E.
Ray Mumphrey, P.E.
Steve Lee, P.E.
Arturo Aguirre, P.E.

Directorate Implementation Sponsor

Janice P. Williams, P.E.
DOTD Chief Engineer

**Development of Wave and Surge Atlas for the Design and Protection of
Coastal Bridges in South Louisiana Phase 2**

by

D. Max Sheppard, Ph.D.

Mark S. Gosselin, Ph.D., P.E.

Philip E. Dompe, P.E.

Hüseyin Demir, Ph.D., P.E.

INTERA Incorporated
2114 NW 40th Terrace, Suite A-1
Gainesville, FL 32605

LTRC Project No. 15-1ST
SIO No. 30000118

conducted for

Louisiana Department of Transportation and Development
Louisiana Transportation Research Center

The contents of this report reflect the views of the author/principal investigator who is responsible for the facts and the accuracy of the data presented herein. The contents do not necessarily reflect the views or policies of the Louisiana Department of Transportation and Development or the Louisiana Transportation Research Center. This report does not constitute a standard, specification, or regulation.

September 2016

ABSTRACT

This report summarizes the work performed by INTERA Incorporated (INTERA), for the Louisiana Department of Transportation and Development (DOTD) on Phase 2 of LTRC project No. 10-4ST, “Development of Wave and Surge Atlas for the Design Protection of Coastal Bridges in South Louisiana.” In Phase 1 (LTRC project No. 10-4ST, Final Report 528), a Level III storm surge/wave analysis provided the design water level and wave parameters needed to compute wave loads. This analysis entailed (1) hindcasting 50 of the most severe tropical storms and hurricanes that have affected Louisiana coastal waters over the past 160 years including hindcasting alternative paths for a select number of those storms resulting in a total of 124 hindcasts, and (2) performing extreme value analyses on water elevation and wave heights throughout the area covered by the model to obtain 100-year design meteorological /oceanographic (met/ocean) conditions. The results from the extreme value analyses provided the data for the Wave and Surge Atlas. The atlas is presented in a geographic information system (GIS) database for ease of access and use. Those data provided the input to determine the vulnerability of selected DOTD coastal bridges to design storm surge and wave loads. This phase includes (1) developing parameters for additional return periods (5-, 10-, 25-, and 50-year) from the results of the hindcasted storms in Phase 1; (2) develop a Surge/Wave Atlas for maximum values of the actual hurricane/tropical storm-induced water elevation, wave height and peak period and wind speed for the study area over the past 160 years; (3) developing a Surge/Wave Atlas for maximum values of the actual + path shifted hurricane/tropical storm- induced water elevation, wave height and peak period, and wind speed for the study area over the past 160- years; (4) Developing an AASHTO Wave Load Calculation Program (Visual Basic Program) based on the AASHTO Guide Specifications [1]; (5) providing a training session for DOTD employees so that DOTD will be able to update or modify the program as needed for future code changes; and (6) computing the forces and moments on the remaining spans on the bridges determined to be vulnerable and generate PDF files of the bridge information and the forces and moments along the entire bridge, and adding PDF pop-ups to the locations of all bridges determined to be vulnerable.

ACKNOWLEDGMENTS

This study was made possible by funding from the Louisiana Transportation Research Center (LTRC). In-kind services were provided by the Louisiana Department of Transportation Bridge Design Section.

IMPLEMENTATION STATEMENT

The hurricane-wave and storm-surge-induced damage experienced by a number of large and expensive bridges in the Gulf Coast states during the past decade led to the creation of the AASHTO document *Guide Specifications for Bridges Vulnerable to Coastal Storms*. This document provides guide specifications for calculating hurricane generated wave and storm surge loads on bridge superstructures for both design of new bridges and evaluation of existing bridges. Phase 1 of this study in LTRC project No. 10-4ST (Final Report 528) was conducted to apply the AASHTO specification to evaluate DOTD's existing coastal bridges to discern their current vulnerability to this type of loading. The study identified 18 bridges as potentially vulnerable. In addition to the vulnerability assessment, this study produced a Wave and Surge Atlas, transmitted to the Department, which contains 100-year wave and storm surge conditions at Louisiana's coastal bridges. The atlas provides a GIS interface to present and access the data. This tool allows DOTD to rapidly identify 100-year wave and storm surge conditions along Louisiana's coastal waterways enabling acquisition of design wave and surge parameters for evaluation of existing bridges or design of new bridges.

While the 100-year meteorological/oceanographic (met/ocean) conditions are appropriate for most designs, there are many issues encountered by DOTD engineers where other frequency met/ocean information (e.g., 5-, 10-, 25-, 50-year return interval values) are more appropriate. Phase 2, the subject of this report, developed a Wave and Storm Surge Atlas for the 5-, 10-, 25-, 50-year return intervals. Those data provide more appropriate conditions to design a temporary facility (a detour bridge) or to design retrofits for bridges, where the service life is approaching the design life.

To facilitate application of the Wave and Storm Surge Atlas, this phase also included development of a calculator that follows the AASHTO document *Guide Specifications for Bridges Vulnerable to Coastal Storms* to calculate the wave forces. The user inputs met/ocean data and structure dimensions and the calculator does the appropriate conversion of the wave data, calculates the forces, and presents the data in a formatted report.

TABLE OF CONTENTS

| | |
|---|-----|
| ABSTRACT..... | iii |
| ACKNOWLEDGMENTS | v |
| IMPLEMENTATION STATEMENT | vii |
| TABLE OF CONTENTS..... | ix |
| LIST OF FIGURES | xi |
| INTRODUCTION | 1 |
| OBJECTIVE | 3 |
| SCOPE..... | 5 |
| METHODOLOGY | 7 |
| Additional Wave and Surge Atlases | 7 |
| Additional Return Periods..... | 7 |
| Maximum Values..... | 7 |
| Wave Force Calculator | 9 |
| Wave and Surge Atlas..... | 21 |
| Detailed Surge/Wave Forces..... | 23 |
| Design Conditions for the Detailed Wave Loading..... | 23 |
| Method of Analysis..... | 24 |
| DISCUSSION OF RESULTS..... | 27 |
| CONCLUSIONS..... | 29 |
| ACRONYMS, ABBREVIATIONS, AND SYMBOLS | 31 |
| REFERENCES | 33 |
| APPENDICES | 35 |
| APPENDIX A Storm surge and wave GIS database | 37 |
| Wave Force Calculator Example – Bridge Recall Number 003440 | 54 |
| APPENDIX B | 65 |
| Detailed Wave Force calculations | 65 |

LIST OF FIGURES

| | |
|---|----|
| Figure 1 Wave force calculator flow chart | 11 |
| Figure 2 Wave force calculator interface data input..... | 14 |
| Figure 3 Wave force calculator superstructure information table | 15 |
| Figure 4 Wave force calculator sea state table..... | 16 |
| Figure 5 Wave force calculator control buttons..... | 17 |
| Figure 6 Wave force calculator results table project information..... | 18 |
| Figure 7 Wave force calculator results table data input..... | 19 |
| Figure 8 Wave force calculator results table data input..... | 20 |
| Figure 9 Wave force calculator results table intermediate steps..... | 20 |
| Figure 10 Screenshot of storm wave and surge atlas GIS geodatabase..... | 22 |

INTRODUCTION

The infrastructure in low-lying coastal areas subject to tropical storms and hurricanes is potentially vulnerable to the elevated water levels, high velocity flows, and wave conditions that accompany these types of storms. It is imperative that those responsible for the design and maintenance of this infrastructure have the most accurate information as practical about these conditions. In particular, coastal roadways and bridges are potentially vulnerable to this type of loading. A number of large and expensive bridges in the Gulf Coast states were destroyed by storms during the past decade. Most of this destruction was attributed to hurricane storm surge and wave forces. In this document, environmental parameters associated with tropical storms and hurricanes are referred to as met/ocean, conditions.

In order for the met/ocean information to be useful, the frequency of occurrence must also be known. That is, estimates of its probability of occurrence each year must be known. With this information, the desired structure life, and the acceptable level of risk, design conditions can be established. Common design frequencies for coastal bridges are 1 and 2 percent chances of occurrence each year (referred to as 100-year and 50-year return intervals, respectively).

The first phase of this project (LTRC Project No. 10-4ST, Final Report 528) developed met/ocean data for the 100-year return period (1 percent chance) in South Louisiana and present the data in a GIS platform referred to as the Wave and Surge Atlas. The second phase, the subject of this report, develops met/ocean data for the 5-, 10-, 25-, and 50-year (20, 10, 4, and 2 percent chance) in South Louisiana, extracts the maximum met/ocean data occurring during all of the hindcasted storms (50 storms), extracts the maximum met/ocean data occurring during all of the hindcasted storms and shifted paths (124 storms), and presents those data in a GIS platform.

Phase 2 also develops a wave force calculator that solves the wave force equations in the AASHTO *Guide Specifications for Bridges Vulnerable to Coastal Storms*. The calculator converts the met/ocean data extracted from the Wave and Surge Atlas based on the methods presented on the AASHTO Specifications, solves the wave and surge parametric equations presented in the AASHTO Specifications, and tabulates the input data and results in an EXCEL spreadsheet.

This phase of the study also evaluates all the spans of the bridges identified as vulnerable during Phase 1. Those results both tabulated and plotted in PDF format are included in the Wave and Surge Atlas.

OBJECTIVE

This research intended to develop and extend the previously completed wave and surge atlases for the design and evaluation of coastal bridges in South Louisiana. Specifically to (1) establish met/ocean data for the 5-, 10-, 25-, and 50-year return interval events (20, 10, 4, and 2 percent chance of occurrence) in South Louisiana, extract the maximum met/ocean data for all the hindcasted storms, extract the maximum met/ocean data for all the hindcasted and shifted storms, and to present the results in a Surge/Wave GIS Database (Wave and Surge Atlas); (2) develop a wave force calculator to solve the parametric equations in the AASHTO *Guide Specifications for Bridges Vulnerable to Coastal Storms*; and (3) evaluate (for the 100-year met/ocean conditions) all of the spans on the DOTD bridges identified as vulnerable and include the results in both tabulated and plotted PDF format in the Wave and Surge Atlas.

SCOPE

This study included seven tasks intended to develop a set of GIS coverages generated from the results of all the events hindcasted in Phase 1, development of a wave force calculator, and a detailed analysis of the bridges identified as potentially vulnerable in Phase 1. The first task developed a Surge/Wave Atlas for return intervals of 5 years, 10 years, 25 years, and 50 years. To maintain consistency in the outcome, the analyses employed the same methodology and computer program(s) used in Phase 1 for the return interval of 100 year. The second task developed a Surge/Wave Atlas for maximum values of the actual hurricane/tropical storm-induced water elevation, wave heights and peak period, and wind speed for the study area over the past 160 years. The third task developed a Surge/Wave Atlas for maximum values of the actual and path shifted hurricane/tropical storm-induced water elevation, wave height and peak period, and wind speed for the study area over the past 160 years. The fourth task developed an AASHTO Wave Load Calculation Program (Visual Basic Program) based on the AASHTO Guide Specifications [1]. The program allows the designers to input metrological/oceanographic parameters from the Surge/Wave Atlas and the bridge superstructure information, and simply click a “compute button” to obtain all wave loads calculated in accordance with the equations in AASHTO’s *Guide Specifications for Bridges Vulnerable to Coastal Storms*. The fifth task provided a training session for DOTD employees so that DOTD will be able to update or modify the program as needed for future code changes. Task six evaluated the forces and moments on the remaining spans on the bridges determined to be vulnerable and generated PDF files of the bridge information and the forces and moments along the entire bridge, added PDF pop-ups to the Wave/Surge Atlas at the locations of all bridges determined to be vulnerable. The PDF pop up at each bridge location displays PDF files of the bridge and loading information, and plots of bridge low chord elevation, maximum 100-year storm water elevation, and 100-year wave crest elevation. Finally, task seven prepared a final report documenting the entire research effort. The report includes guidelines regarding the application and/or limitations of the Surge/Wave Atlas. The following section details the methods employed to meet the target scope.

METHODOLOGY

Phase 1 developed a 100-year Wave and Surge Atlas for southern Louisiana based on a 100-year return period interval and employed that data to evaluate the potentially vulnerable bridges (LTRC Project No. 10-4ST, Final Report 528). This phase of the study extends Phase 1 by developing the met/ocean data for 5, 10, 25, and 50 years (20, 10, 4, and 2 percent chance), extracting the maximum met/ocean conditions for all of the hindcasted storms (50 storms) and the maximum conditions for all of the hindcasted storms and the shifted path storms (124 storms) in South Louisiana. The results are presented in a Wave and Surge Atlas for each event. Additionally, Phase 2 calculates the forces on the remainder of the spans on the bridges identified as vulnerable in Phase 1 including the results in the Wave Atlas and develops a wave force calculator to solve the AASHTO wave force equations.

Additional Wave and Surge Atlases

This section presents the analysis of the Phase 1 hindcast simulations to develop the 5-, 10-, 25-, and 50-year (20, 10, 4, and 2 percent chance) and the maximum met/ocean conditions for all of the hindcasted storms and the maximum conditions for all of the hindcasted storms and the shifted path storms.

Additional Return Periods

For consistency, development of the 5-, 10-, 25-, and 50-year (20, 10, 4, and 2 percent chance) return period conditions followed the methodology from the Phase 1 study (LTRC Project No. 10-4ST, Final Report 528). This involved extracting the (1) maximum water surface elevation (storm surge plus local wind setup) and associated wave heights and (2) maximum wave heights and associated water surface elevations at each node location within the model domain from each simulated storm during Phase 1. Then applying extreme value analyses to obtain the 5-, 10-, 25-, and 50-year values of the (1) maximum water surface elevation (storm surge plus local wind setup) and associated wave heights and (2) maximum wave heights and associated water surface elevations at each node location within the model domain. Contour maps of 5-, 10-, 25-, and 50-year (1) maximum water surface elevations and associated wave heights, and (2) maximum wave heights and associated water surface elevations, and (3) steepness/depth limited wave periods for maximum waves are developed and converted to shapefiles. Separate GIS geodatabases for each return period are developed from the shape files.

Maximum Values

The Wave and Surge Atlas developed during Phase 1 of the project incorporated 100-year surge and wave conditions. The 100-year conditions originated from extreme value analyses

on actual or synthetic data. Notably, accuracy of the different return interval values increases with larger data sets for extreme value analyses. The methodology applied in the development of the atlas started with hindcasting the actual major storms that affected the study area over the last 160 years. Shifting the paths of all of these storms to the right and left of the point of landfall and hindcasting these shifted path storms created two additional data sets. All three hindcasts (actual and two shifted paths) excluded astronomical tides so that they could be added later with up to 1,000 different phases with each storm. This step helped capture more of the natural variability (and further increased the sample size) and, therefore, the accuracy of the different return interval values. This procedure, however, prevents the computation of the precise storm of record properties (e.g., water surface elevations and wave heights). While the magnitude of the astronomical tide varies throughout the month, the peak values are relatively small along the Louisiana coast — the tide range is about 2 feet. One may obtain reasonably conservative estimates of the storm of record parameter values by applying the average root mean square (RMS) value of the astronomical tide for the month. A slightly more conservative approach would include applying the maximum tide value for the month.

Maximum Values Actual Hindcasted Storms. The process involves extracting the maximum water surface elevation (storm surge plus local wind setup) and associated wave heights and maximum wave heights and associated water surface elevations at each node location within the model domain from each actual storm hindcasted during Phase 1. Water surface elevations are adjusted by adding the average RMS tide elevation from the one-month, tide-only ADCIRC simulation performed during Phase I. The next step determines the maximum and associated parameters at each node location within the model boundary for each of the actual storms. Note that the storm that produced the value of one parameter is not necessarily the storm that produced the maximum value of another parameter. Those data are then used to construct contour maps of maximum water surface elevations (and associated wave heights and periods), and maximum wave heights (and associated periods), associated water surface elevations, and steepness/depth-limited wave periods for maximum waves. Finally, the GIS geodatabase is populated by converting the contour maps to shapefiles. This GIS geodatabase represents the actual hurricane/tropical storm Surge/Wave Atlas.

Maximum Values Actual Hindcasted and Shifted Storms. The maximum values for the actual hindcasted and shifted storms generally follow the methodology outlined for the actual hindcasted with the exception that the analysis will consider actual and path shifted hurricanes/tropical storms.

Wave Force Calculator

This section details the methodology employed to develop an AASHTO wave load calculation program (Visual Basic Program) based on the AASHTO Guide Specifications [1]. The program allows the designers to input metrological/oceanographic parameters from the Surge/Wave Atlas and the bridge superstructure information to obtain all wave loads calculated in accordance with the equations in AASHTO's *Guide Specifications for Bridges Vulnerable to Coastal Storms*.

Figure 1 presents the flow chart designed to guide the development of the code. As the flow chart illustrates, the logic starts by checking the input values (wave parameters) to ensure they do not violate the limits of the AASHTO equations. If so, the wave parameters are adjusted and noted. The logic continues checking to confirm the design wave parameters can physically exist, as required by the AASHTO code, noting any changes to the wave parameters. Once the design wave parameters are confirmed, the wave forces are calculated and the results are printed.

The code, developed in Visual Basic, includes three modules with 17 commented subroutines. Module1 contains the “Reset_Wave_Forces” subroutine, which resets all the inputs in the spreadsheet to the default values and clears the output table. Module2 contains the subroutine that creates reports for the selected spans. Module3 contains the main subroutine “Calc_Wave_Forces()” that sets the variable declarations, initiates the variables, reads the user specified variables, initiates the wave force subroutines, and writes the results to the EXCEL spreadsheet. There is a separate subroutine for each force component, the slamming force, and the trapped air factor. Table 1 lists each of the Module3 subroutines along with the description and reference (where applicable) to the AASHTO Specifications.

An EXCEL spreadsheet provides access to the Wave Force Calculator. Figure 2 presents an overview of the calculator, which includes the superstructure information, the sea state data, the control buttons, the project information, the definition figure, and the results table. The spreadsheet includes an “Instructions” sheet that details these components.

Figure 3 presents the superstructure information table and details the superstructure information required in the calculations. Information required for the superstructure are the span number, span length, span width, girder height, girder type, number of girders, deck thickness, railing height, percent of trapped air, and the clearance. The girder height is employed for all forces except the horizontal force associated with the maximum vertical force — for slab structures, the girder height is zero. Girder type is only employed in the horizontal force associated with the maximum vertical force equation and is limited to the

five girder types, a slab, and box girder available through the drop down menu. For slabs, the height of the slab (and deck) are input as the deck thickness. Trapped air accounts for the volume of air trapped between the girders and is estimated as a percentage of the height of the girders — typically 100%.

Figure 4 presents the sea state table and describes the information necessary to populate the table. These data are provided in the Wave and Surge Atlas. Figure 5 presents the control buttons and details the action each button initiates. Figure 6 presents the project information table, which provides the data to populate the reports generated by the calculator. Figure 7 through Figure 9 present the results table, which lists the results, input data, and intermediate and steps of the wave force parametric equations. Both INTERA and DOTD vetted the Wave Force Calculator by testing the program for a wide range of coastal conditions and bridge superstructure geometries.

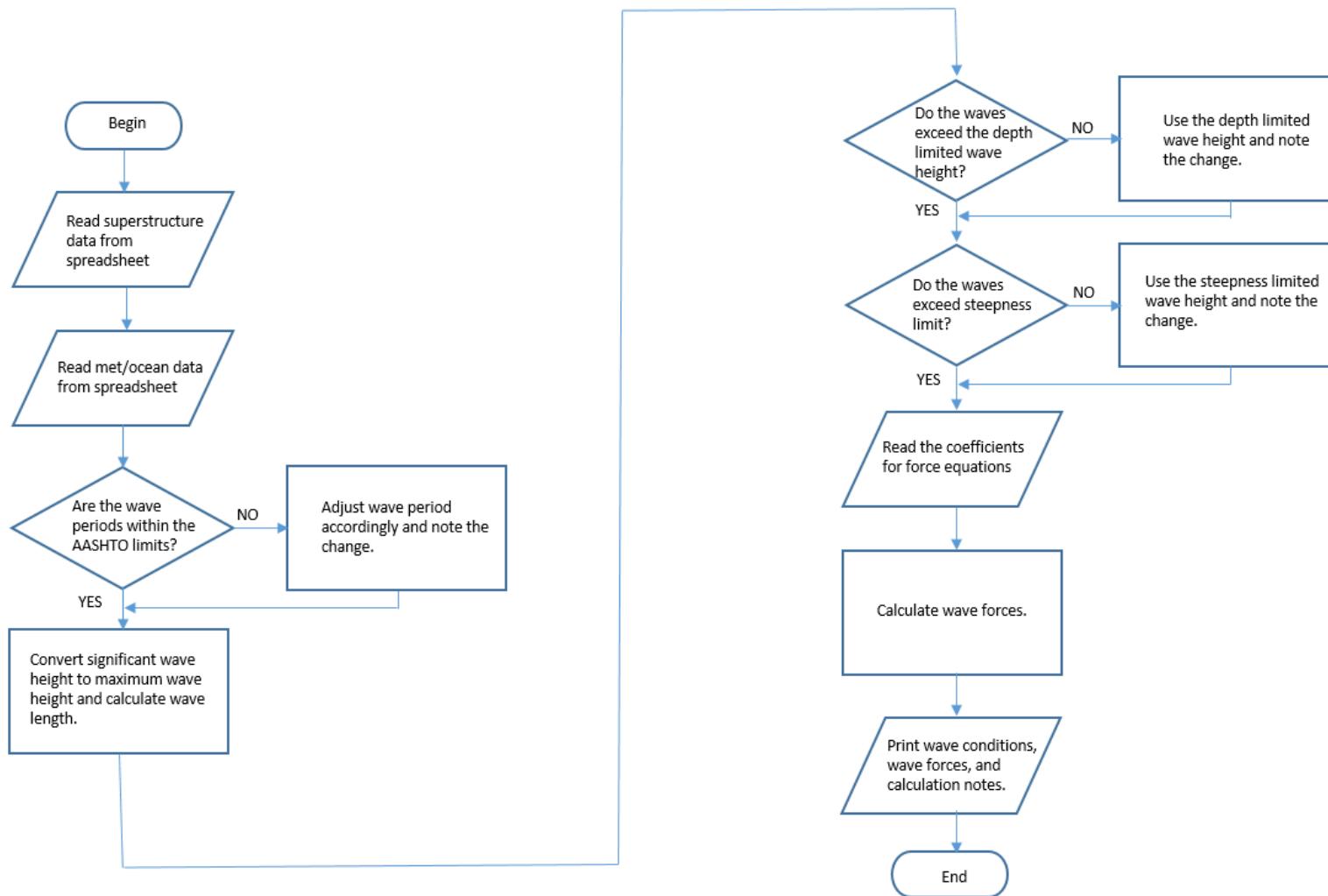


Figure 1
Wave force calculator flow chart

Table 1
Wave calculator subroutines

| Subroutine | Purpose |
|-----------------------------|---|
| Calc_Wave_Forces | Initialize Variables, reads in user specified variables, calls the wave force calculation subroutines, and writes the results to the results table |
| Calculate_Sea_State | The Calculate_Sea_State subroutine employs equations 6.2.2.4-7 through 6.2.2.4-10 to calculate the design wave parameters based on the input information. The routine also checks for depth and steepness limiting criteria. It also checks at the period is within the range of applicability for the equations. |
| Check_Input_Errors | The Check_Input_Errors subroutine ensures that the input variables have all been entered. The routine displays a message box if the cells have been left blank. |
| Create_Calculation_Notes | The Create_Calculation_Notes subroutine creates notes to be included in the output tables that indicate if any of the limits have been exceeding regarding wave height or peak period. |
| Determine_Fhav | The Determine_Fhav subroutine employs equations 6.1.2.2.3-1 through 6.1.2.2.3-3 to calculate the quasi-static horizontal force, Fhav, associated with the maximum vertical force. |
| Determine_Fhav_Coefficients | The Determine_Fhav_Coefficients subroutine assigns the coefficients employed in the calculation of the associated horizontal force at time of maximum vertical force. |
| Determine_FHmax | The Determine_FHmax subroutine employs equations 6.1.2.3.1-1 through 6.1.2.3.1-4 to calculate the maximum horizontal force, FHmax. |
| Determine_Fslam | The Determine_Fslam subroutine employs equations 6.1.2.2.2-1 through 6.1.2.2.2-4 to calculate the slamming force, Fslam. |

| Subroutine | Purpose |
|-----------------|--|
| Determine_Fvah | The Determine_Fvah subroutine employs equations 6.1.2.3.2-1 through 6.1.2.3.2-4 to calculate the vertical force, Fvah, associated with the maximum horizontal force. |
| Determine_FVmax | The Determine_FVmax subroutine employs equations 6.1.2.2.1-1 through 6.1.2.2.1-8 to calculate the maximum vertical force, FVmax. |
| Determine_Mtah | The Determine_Mtah subroutine employs equations 6.1.2.3.4-1 through 6.1.2.3.4-2 to calculate the moment, Mtah, associated with the maximum horizontal force. |
| Determine_Mtav | The Determine_Mtav subroutine employs equations 6.1.2.2.4-1 through 6.1.2.2.4-9 to calculate the moment, Mtav, associated with the maximum vertical force. |
| Determine_TAF | The Determine_TAF subroutine employs equations 6.1.2.2.1a-8 through 6.1.2.2.1a-10 to calculate the trapped air factor TAF. It also checks the range of percent trapped air via equations 6.1.2.2.1a-11&12. |
| Format_rows | The Format_rows subroutine formats the output table on the 'Calculator' spreadsheet. |

Figure 2 illustrates the Wave force calculator interface data input screen. The interface is divided into several sections:

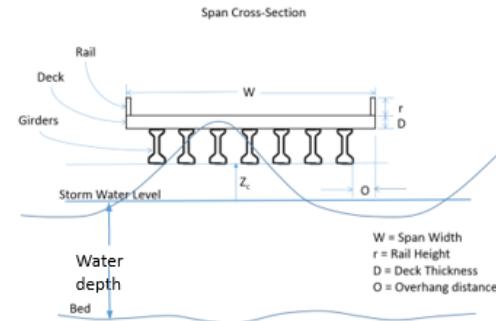
- Sea State Data**: Located at the top left, this section contains input fields for Significant Wave Height (H_s) [ft], Water Depth (d) [ft], and Wave Peak Period (T_p) [sec].
- Control Buttons**: A group of buttons including "Run Wave Force Calculator", "Create Reports", and "Reset Wave Force Calculator".
- Project Information**: A section for entering project details such as Project Name, Project Number, Recall Num., Parish, Structure Num., Route, Date, Designer, and Reviewer.
- Superstructure Information**: A table containing structural parameters like Span, Span Length, Span Width, Girder Height, Girder Type, Number of Girders, Deck Thickness, Girder Length, Overhang, Trapped Air, and Clearance Z_c [ft].
- Definition Figure**: A diagram showing a bridge's span cross-section. It includes labels for Rail, Deck, Girders, Storm Water Level, Water depth, Bed, and dimensions W (Span Width), r (Rail Height), D (Deck Thickness), and O (Overhang distance).
- Results**: A table displaying calculated results for three reports. The columns include Report, Span, Maximum Vertical Force (kip), Associated Horizontal Force (kips), Moment Associated with the Maximum Vertical Force (kip-ft), Maximum Horizontal Force (kips), Associated Vertical Force (kips), Moment Associated with the Maximum Horizontal Force (kip-ft), Equation Note 1, and Equation Note 2.
- Results/Input Data/Intermediate Steps**: A section at the bottom right where intermediate steps or input data can be reviewed.

| Report | Span | Maximum Vertical Force (kip) | Associated Horizontal Force (kips) | Moment Associated with the Maximum Vertical Force (kip-ft) | Maximum Horizontal Force (kips) | Associated Vertical Force (kips) | Moment Associated with the Maximum Horizontal Force (kip-ft) | Equation Note 1 | Equation Note 2 |
|--------|------|------------------------------|------------------------------------|--|---------------------------------|----------------------------------|--|---|--|
| 1 | 593 | 63 | 8949 | 148 | 494 | 8180 | | <i>Tp was within the limits for the equations in the AASHTO code - No changes to Tp</i> | <i>Equation steeper than AASHTO violated</i> |
| 2 | 593 | 63 | 8949 | 148 | 494 | 8180 | | <i>Tp was within the limits for the equations in the AASHTO code - No changes to Tp</i> | <i>Equation steeper than AASHTO violated</i> |
| 3 | 593 | 63 | 8949 | 148 | 494 | 8180 | | <i>Tp was within the limits for the equations in the AASHTO code - No changes to Tp</i> | <i>Equation steeper than AASHTO violated</i> |

Figure 2
Wave force calculator interface data input

Superstructure Information

- Span – Enter the span number.
- Span Length – Enter the length of the span in feet.
- Span Width – Enter the span width (W) in feet.
- Girder Height – This field is employed for all forces except the horizontal force associated with the maximum vertical force. Enter the girder height in feet.
- Girder Type – This field is only employed in the horizontal force associated with the maximum vertical force equation. Select a girder type from the drop down menu only. Selecting a slab structure changes the inputs. For slab structures, girder height, number of girders, overhang, and % trapped air are not used, so they are removed from the input table.
- Number of Girders – Enter the number of girders.
- Deck Thickness – Enter the deck thickness (D) in feet. For slab structures, enter the combined slab and deck thickness.
- Railing Height – Enter the height of the railing in feet.
- % Trapped Air – Enter the percentage of air trapped between the girders - in most cases, this is 100%. The % trapped air is less than 100% when the diaphragm does not extend to the bottom of the girder. For that case, determine the % trapped air by dividing the height of the diaphragm by the height of the girder and multiply by 100.
- Clearance – Enter the distance between the low chord (bottom of the girders or slab) and the storm water level. If the storm water level is below the low chord elevation the value is positive, if the storm water level is above the low chord elevation the value is negative.
- Overhang – the distance from the edge of the deck to the edge of the girder.



| Superstructure Information | |
|----------------------------|-----------------|
| Span | 1 |
| Span Length (ft) | 50 |
| Span Width (ft) | 30 |
| Girder Height (ft) | 3.75 |
| Girder Type | AASHTO Type III |
| Number of Girders | 5 |
| Deck Thickness (ft) | 0.7 |
| Railing Height (ft) | 3 |
| Overhang (ft) | 2.5 |
| % Trapped Air | 100 |
| Clearance Zc (ft) | -1 |

| Superstructure Information | |
|----------------------------|---------------------|
| Span | 1 |
| Span Length (ft) | 50 |
| Span Width (ft) | 30 |
| Girder Type | 3.75 |
| Girder Type | 21-inch Voided Slab |
| Number of Girders | 5 |
| Slab Thickness (ft) | 0.7 |
| Railing Height (ft) | 3 |
| Overhang (ft) | 2.5 |
| % Trapped Air | 100 |
| Clearance Zc (ft) | -1 |

Note - for slab bridges, the following fields are not used: Number of Girders, Overhang, % Trapped Air.

Figure 3
Wave force calculator superstructure information table

| Sea State | |
|---------------------------------|----|
| Significant Wave Height Hs (ft) | 5 |
| Wave Peak Period Tp (sec) | 6 |
| Water Depth ds (ft) | 15 |

- Significant Wave Height – Enter the significant wave height (ft) from the “Wave and Surge Atlas” (or from another reliable source).
- Wave Peak Period – Enter the significant peak wave period (sec) from the “Wave and Surge Atlas” (or from another reliable source).
- Water Depth – Enter the water depth in feet. The water depth is the distance from the Storm Water Level to the bed.

Figure 4
Wave force calculator sea state table



- Run Wave Force Calculator – Once all the input tables are populated, clicking the “Run Wave Force Calculator” button calculates the wave forces and writes the results to the results table.
- Create Reports – Clicking this button creates reports for each span with a “1” in the first column of the results table. Note that the span identifier (number) must be unique for each span checked.
- Reset Wave Force Calculator – Clicking this button resets the Calculator sheet, clearing the table and resetting the input to the default values.

Figure 5
Wave force calculator control buttons

| | | | |
|-----------------|----------------------|----------------|----------------------|
| Project Name: | <input type="text"/> | | |
| Project Number: | <input type="text"/> | Recall Num: | <input type="text"/> |
| Parish: | <input type="text"/> | Structure Num: | <input type="text"/> |
| Route: | <input type="text"/> | Date: | <input type="text"/> |
| Designer: | <input type="text"/> | Reviewer: | <input type="text"/> |
| Notes: | <input type="text"/> | | |

- Enter the project information into the fields in this section.
- These fields are the same for all spans and are retained in the span reports created for the project.
- Fields contained in this section include:
 - Project Name
 - Project Number
 - Recall Number
 - Parish
 - Structure Number
 - Route
 - Date
 - Designer
 - Reviewer
- The section also contains a field for notes (a maximum of 6 lines). The notes will be reproduced the non-printable area within each span report.

Figure 6
Wave force calculator results table project information

| Results | | | | | | | | | | | |
|---------|------|-------------------------------|-----------------------------------|--|--------------------------------|----------------------------------|--|--|---|--|--|
| Report | Span | Maximum Vertical Force (kips) | Associated Horizontal Force (kip) | Moment Associated with the Maximum Vertical Force (kip-ft) | Maximum Horizontal Force (kip) | Associated Vertical Force (kips) | Moment Associated with the Maximum Horizontal Force (kip-ft) | Equation Note 1 | Equation Note 2 | Steepness Limit Notes | Depth Limit Notes |
| 1 | 1 | 593 | 63 | 8949 | 148 | 494 | 8180 | Tp was within the limits for the equations in the AASHTO code - No changes to Tp | Equation steepness limit was not violated | Physical limitations to the wave steepness were not violated | Physical limitations to the wave height imposed by water depth were not violated |
| 1 | 2 | 593 | 63 | 8949 | 148 | 494 | 8180 | Tp was within the limits for the equations in the AASHTO code - No changes to Tp | Equation steepness limit was not violated | Physical limitations to the wave steepness were not violated | Physical limitations to the wave height imposed by water depth were not violated |
| 1 | 3 | 593 | 63 | 8949 | 148 | 494 | 8180 | Tp was within the limits for the equations in the AASHTO code - No changes to Tp | Equation steepness limit was not violated | Physical limitations to the wave steepness were not violated | Physical limitations to the wave height imposed by water depth were not violated |
| | 4 | 593 | 63 | 8949 | 148 | 494 | 8180 | Tp was within the limits for the equations in the AASHTO code - No changes to Tp | Equation steepness limit was not violated | Physical limitations to the wave steepness were not violated | Physical limitations to the wave height imposed by water depth were not violated |
| | 4 | 593 | 63 | 8949 | 148 | 494 | 8180 | Tp was within the limits for the equations in the AASHTO code - No changes to Tp | Equation steepness limit was not violated | Physical limitations to the wave steepness were not violated | Physical limitations to the wave height imposed by water depth were not violated |
| | 4 | 593 | 63 | 8949 | 148 | 494 | 8180 | Tp was within the limits for the equations in the AASHTO code - No changes to Tp | Equation steepness limit was not violated | Physical limitations to the wave steepness were not violated | Physical limitations to the wave height imposed by water depth were not violated |

- Report – This column tells the program to generate a report for the span. When the “Create Reports” button is pressed, the program will create a new Excel book with a separate report for each span that has a “1” in this column. Note that span numbers can not be the same for selected spans.
- Span – The number to identify the span. When generating reports, each report selected must have a unique number.
- Maximum Vertical Force – The maximum vertical force produced on the span by the conditions using equation 6.1.2.2.1 from the AASHTO specifications.
- Associated Horizontal Force – The horizontal force associated with the maximum vertical force using equation 6.1.2.2.3 from the AASHTO specifications.
- Moment Associated with the Maximum Vertical Force – The moment associated with the maximum vertical force using equation 6.1.2.2.4 from the AASHTO specifications.
- Maximum Horizontal Force – The maximum horizontal force produced on the span by the conditions using equation 6.1.2.3.1 from the AASHTO specifications.
- Associated Vertical Force – The vertical force associated with the maximum horizontal force using equation 6.1.2.2.2 from the AASHTO specifications.
- Moment Associated with the Maximum Horizontal Force – The moment associated with the maximum horizontal force using equation 6.1.2.3.4 from the AASHTO specifications.
- Equation Note 1 – Notes whether the peak period (Tp) is within the limits for the equations in the AASHTO code and if the program adjusted Tp.
- Equation Note 2 – Notes whether the wave steepness is within the limits for the equations in the AASHTO code and if the program adjusted either the wave length or the maximum wave height (Hmax).
- Steepness Limit Note – Notes whether the physical limitations to the wave steepness were violated and if the program adjusted the maximum wave height (Hmax).
- Depth Limit Note – Notes whether the physical limitations to the wave height imposed by water depth were violated and if the program adjusted the maximum wave height (Hmax).

Figure 7
Wave force calculator results table data input

| Input Data | | | | | | | | | | | | | |
|------------------|-----------------|--------------------|--------------|-------------------|---------------------|---------------------|---------------|---------------|-------------------|---------------------------------|----------------------|---------------------|--|
| Span Length (ft) | Span Width (ft) | Girder Height (ft) | Girder Type | Number of Girders | Deck Thickness (ft) | Railing Height (ft) | Overhang (ft) | % Trapped Air | Clearance Zc (ft) | Significant Wave Height Hs (ft) | Peak Period Tp (sec) | Water Depth ds (ft) | |
| 50 | 30 | 3.75 | FL Bulb-T 78 | 5 | 0.7 | 3 | 2.5 | 100 | -1 | 5 | 6 | 15 | |
| 50 | 30 | 3.75 | FL Bulb-T 78 | 5 | 0.7 | 3 | 2.5 | 100 | -1 | 5 | 6 | 15 | |
| 50 | 30 | 3.75 | FL Bulb-T 78 | 5 | 0.7 | 3 | 2.5 | 100 | -1 | 5 | 6 | 15 | |
| 50 | 30 | 3.75 | FL Bulb-T 78 | 5 | 0.7 | 3 | 2.5 | 100 | -1 | 5 | 6 | 15 | |

- This portion of the table re-tabulates the user input data from the "Superstructure Input" and "Sea State Data" tables for each calculation.

Figure 8
Wave force calculator results table data input

| Wave Parameters | Intermediate Steps | | | | | | | | | | | | | A.1.2.2.1 Maximum Horizontal Wave Force | | A.1.2.2.2 Associated Quasi-Static Vertical Force | | A.1.2.2.3 Associated Moment about Trailing Edge | | A.1.2.2.4 Associated Quasi-Static Vertical Force | | A.1.2.2.5 Associated Moment about Trailing Edge | |
|-------------------------|---|-----------------|----------------|--------------------------------|----------------|----------------|---|----------------|----------------|---|----------------|----------------|--|---|-----------------|--|-----------------|--|-----------------|--|-----------------|---|-----------------|
| | A.1.2.1.1 Maximum Quasi-Static Vertical Force | | | A.1.2.2.4.0 Trapped Air Factor | | | A.1.2.2.2 Associated Vertical Screening Force | | | A.1.2.2.3 Associated Horizontal Quasi-Static Wave Force | | | A.1.2.2.4 Associated Moment about the Trailing Edge due on the Quasi-Static and Screening Forces | | | A.1.2.2.1 Maximum Horizontal Wave Force | | A.1.2.2.2 Associated Quasi-Static Vertical Force | | A.1.2.2.3 Associated Moment about Trailing Edge | | | |
| Design Wave Length (ft) | Span (ft) | Wavelength (ft) | A ₀ | A ₁ | A ₂ | A ₃ | A ₄ | A ₅ | A ₆ | A ₇ | A ₈ | A ₉ | A ₁₀ | A ₁₁ | A ₁₂ | A ₁₃ | A ₁₄ | A ₁₅ | A ₁₆ | A ₁₇ | A ₁₈ | A ₁₉ | A ₂₀ |
| 50 | 30 | 30 | 0.000 | 0.020 | 0.009 | 0.002 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 |
| 50 | 30 | 30 | 0.000 | 0.020 | 0.009 | 0.002 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 |
| 50 | 30 | 30 | 0.000 | 0.020 | 0.009 | 0.002 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 |
| 50 | 30 | 30 | 0.000 | 0.020 | 0.009 | 0.002 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 |

- This portion of the table provides the intermediate steps in the wave force calculations as a tool to perform independent checks.

Figure 9
Wave force calculator results table intermediate steps

Wave and Surge Atlas

The magnitude of the met/ocean information produced by this study is extremely large; therefore, its presentation does not lend itself to the usual tables and graphs. For this reason, this information, which has applications beyond the computation of wave loads on bridge superstructures, is presented in a GIS database. The database, constructed with ESRI ArcInfo and ArcReader GIS mapping software, provides greater flexibility in graphical representation of large datasets with seamless flow between various types of information.

The Wave and Surge GIS Database presents the user with an interactive map that contains 5-, 10-, 25-, 50-, and 100-year hydraulic design data for Louisiana's coastal waters. Hydraulic data contained within the database include the following: 5-, 10-, 25-, 50-, and 100-year maximum storm surge level (and associated significant wave height and peak wave period) and the 5-, 10-, 25-, 50-, and 100-year maximum significant wave height (and associated peak wave period and storm water level). Thematic groups include roadways, county boundaries, and city and town locations. Base maps in the database include land boundaries, aerial imagery, and topographic maps.

Accessibility of the hydraulic information is by mouse click at the desired point on the map. This information can also be obtained by typing in the x-y coordinates (lat-long, state-plane, etc.) or bridge recall number. The search results are presented in tabular format with the coordinates and hydraulic values displayed in a window on the map.

The information in the Storm Surge and Wave GIS Database will have numerous applications for both existing and future water related projects. Figure 10 displays a screen shot of the Storm Surge and Wave GIS base map. In the figure, the locations of the 65 bridges evaluated in the study are represented by the yellow dots. A detailed description of the database and its application is presented in Appendix A.

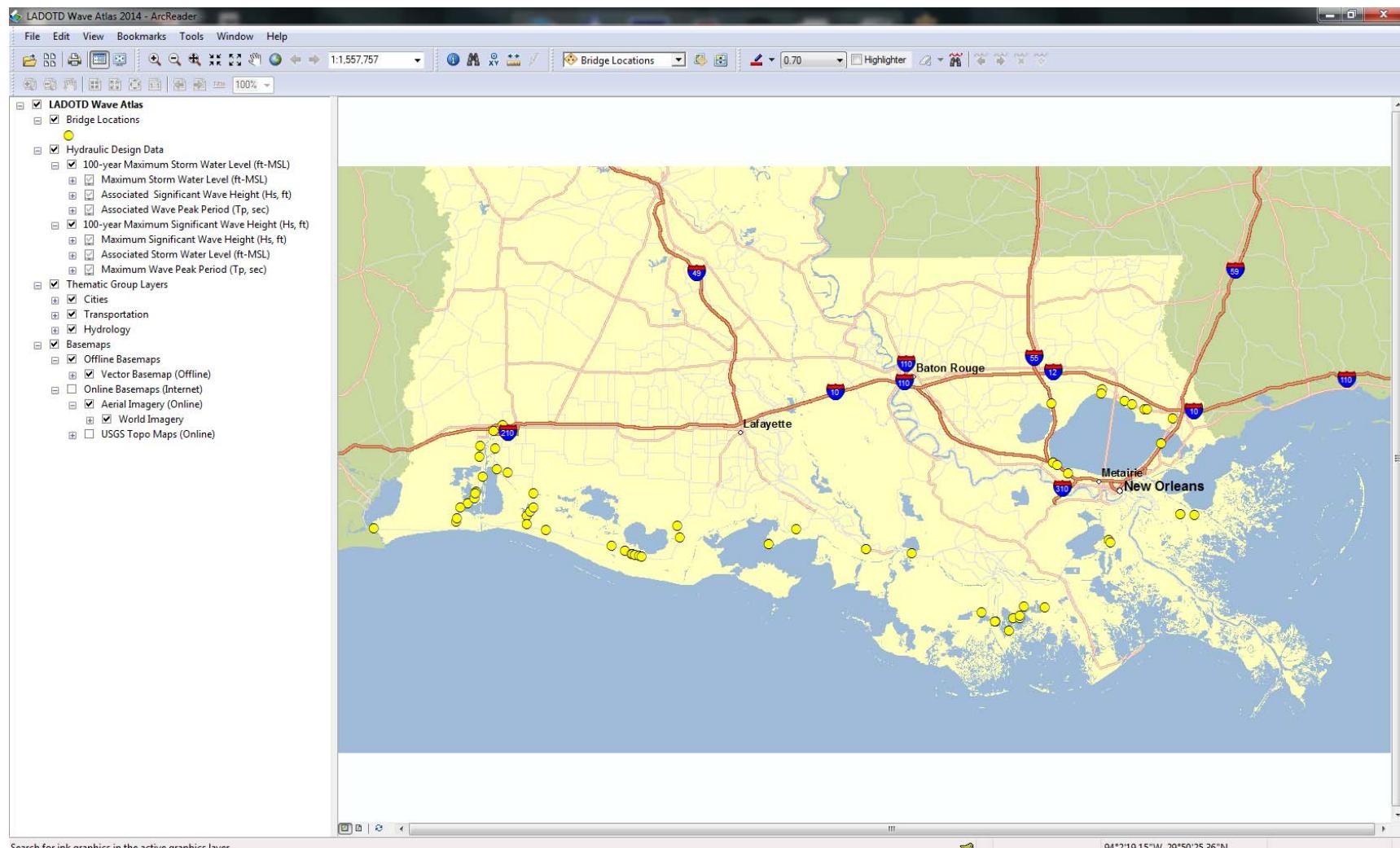


Figure 10
Screenshot of storm wave and surge atlas GIS geodatabase

Detailed Surge/Wave Forces

Phase 1 evaluated 65 bridges for vulnerability to storm surge and wave loading. The analysis took a conservative approach evaluating the most vulnerable span (typically near the begin or end bridge) while applying the largest met/ocean conditions (wave height and surge elevation) along the alignment. By design, this conservative approach leads to false positives. Based on that approach, Phase 1 identified 18 vulnerable bridges in coastal Louisiana. This phase, Phase 2, employs a more detailed approach that evaluates all the spans and the met/ocean conditions at that location. Table 2 lists the bridges identified as vulnerable in Phase 1 that are further evaluated for detailed wave loading in this phase as described in the following sections.

Table 2
Bridges evaluated

| Bridge Recall No. |
|----------------------|
| 002631 |
| 002632 |
| 002650 |
| 002892 |
| 002894 |
| 003432 |
| 003440 |
| 003450 |
| 003480 |
| 003510 |
| 003520 |
| 003690 |
| 009030 |
| 031755 |
| 033698 |
| 033700 |
| 059482 |
| 060360 |

Design Conditions for the Detailed Wave Loading

Since these forces and moments depend on the combination of water elevation and wave parameters (height and length), two sets of conditions require examination at each bridge; (1) the maximum 100-year water elevation and associated wave height and (2) the maximum wave height and associated water elevation. This is necessary since the maximum wave

height does not necessarily occur at the same time as the maximum water elevation during the storm. The extreme value analyses applied to the hindcasted storms (in Phase 1) produced these two sets of data for 100-year return interval met/ocean conditions.

For bridges crossing large bodies of water, the 100-year storm water level and wave heights can vary along the bridge. For these cases, the extreme value analysis was performed at multiple locations along the bridge alignment and the results from the location provided the met/ocean input for the adjacent spans. Appendix B lists the two 100-year met/ocean cases evaluated.

Method of Analysis

The procedure for identifying the vulnerability of the selected bridges began by obtaining the dimensions for each span on each bridge. The structural parameters (span type, dimensions, low chord elevation, superstructure dead weight, etc.) for that span were provided by DOTD. Next, the design forces and moments were computed along with a conservative estimate (i.e., a lower estimate) of the resistive forces and moments. As discussed in detail below, the design forces include a load factor, the magnitude of which depends on the criticality of the bridge. The resistive forces are conservative in that only the dead weight of the superstructure is considered. If there are tie-downs or other means of increasing the resistance, then the actual resistive forces and moments will be greater. The vulnerability index is the ratio of the design force or moment divided by the maximum resistive force or moment. If the vulnerability index is greater than or equal to 1 for either force or moment, the bridge span is considered to be potentially vulnerable. The computer model used to compute the forces and moments is discussed below.

Physics Based Model. Until recently, the methods for predicting wave forces on horizontal structures such as bridge spans were not well developed. Kaplan and Kaplan et al. published an analytical approach for computing forces on the decks of offshore platforms using an approach similar to that used in the development of Morison's Equation for horizontal wave forces on vertical piles [2], [3], [4]. There are, however, differences between offshore platform decks and bridge spans as well as differences in the range of wave frequencies (and thus wave lengths) encountered by most coastal bridges. Starting with Kaplan's Equations, Dr. Sheppard and his graduate students at the University of Florida developed predictive equations for wave-induced horizontal and vertical forces and the resulting moments on bridge superstructures [5]. INTERA developed a proprietary computer program (Physics Based Model or PBM) that evaluates these equations for a wide variety of bridge superstructure designs and met/ocean conditions. The PBM generated the data that formed the basis of the parametric force and moment equations in the AASHTO's *Guide*

Specifications for Bridges Vulnerable to Coastal Storms. This involved calculating wave forces and moments using the PBM for a large number of met/ocean conditions for many of the more common beam types. Curve fitting that data provided the equations in the AASHTO code. These equations envelope the majority of the data, which, in general, results in conservative predictions. Additionally, the equations in the code are limited by the conditions used in their development. Specifically, waves were limited to wave periods between 3 and 10 seconds with steepness limited to values between 0.035 and 0.15 and heights were limited to no greater than 65 percent of the water depth. The PBM computed the surge/wave loads in this study.

Surge/Wave Forces and Moments. The forces and moments were computed for two sets of conditions, the 100-year storm water level and the associated wave height and the 100-year wave height and the associated storm water level and a range of wave periods. For bridges crossing large water bodies, where the 100-year storm water level and wave heights vary along the bridge, the combination of wave heights and storm water levels closest to the span of interest were used in the calculations. Once calculated, the larger of the forces and moments provided the inputs to compute the bridge vulnerability. Appendix B details the results of the wave force calculations.

Bridge Vulnerability. In this study, bridge spans where the design surge/wave forces and/or moments (with the proper load factors) exceed the maximum resistive forces and/or moments are classified as vulnerable. The AASHTO code recommends a strength limit state wave force load factor of 1.75 for bridges classified by the owner as “critical/essential.” For bridges designated as “typical,” the extreme event limit state wave force load factor is specified as 1.00. DOTD provided the criticality classification for the bridges examined in this study. Bridges classified with a criticality of 3 or greater are considered “critical/essential” and are evaluated with a load factor of 1.75. Conversely, bridges classified with a criticality of 2 or less are considered “typical” and are evaluated with a load factor of 1.00.

The resistive forces consist of superstructure dead weight, tie-downs or other constraints (if present), and frictional forces between the super- and substructure. Due to the effort required to obtain information on the existence and condition of tie-downs and estimating frictional forces, only superstructure dead weight is considered in this analysis. Bridges found to be vulnerable, from this conservative approach, should be further examined to discern accurate tie-down information prior to making decisions regarding corrective action. The vulnerability index for both vertical force and moment were computed. These indices, along with the resistive forces and moments, are presented in Appendix B. Table 3 summarizes the results

of the detailed wave force analysis. From the table, this analysis identified 15 of the 18 bridges as vulnerable to the 100-year surge/wave loading. Notably, the bridges that were reclassified as “Not Vulnerable” are longer bridges. For those cases, the conservative approach in Phase 1 resulted in false positive results. This results from selecting the most vulnerable span — the span with the lowest low chord (i.e. near the banks of the body of water) — while applying the largest met/ocean conditions (near the center of the body of water), which, as this analysis demonstrates, can result in conservative results for long crossings.

Table 3
100-year surge/wave forces and moments calculated using the PBM

| Bridge Recall No. | Vulnerability |
|-------------------|----------------|
| 2631 | Vulnerable |
| 2632 | Vulnerable |
| 2650 | Vulnerable |
| 2892 | Not vulnerable |
| 2894 | Vulnerable |
| 3432 | Vulnerable |
| 3440 | Vulnerable |
| 3450 | Vulnerable |
| 3480 | Vulnerable |
| 3510 | Not vulnerable |
| 3520 | Vulnerable |
| 3690 | Vulnerable |
| 9030 | Vulnerable |
| 31755 | Vulnerable |
| 33698 | Vulnerable |
| 33700 | Not vulnerable |
| 59482 | Vulnerable |
| 60360 | Vulnerable |

DISCUSSION OF RESULTS

The objectives of this study included (1) creating a Wave and Surge Atlas for the 5-, 10-, 25-, and 50 year return period events (20, 10, 4, and 2 percent chance), the maximum met/ocean data occurring during all of the hindcasted storms, and the maximum met/ocean data occurring during all of the hindcasted storms and shifted paths; (2) developing a wave and surge calculator that solves the equations in the AASHTO code; (3) applying the 100-year met/ocean data to compute the storm surge and wave loads on the spans of the DOTD coastal bridges identified as vulnerable in Phase 1; and (4), assessing the analyzed bridges' vulnerability based on these computed loads. The Wave and Surge Atlas includes the 5-, 10-, 25-, and 50-year (20, 10, 4, and 2 percent chance) maximum water elevations with associated wave heights, and maximum wave heights with associated water elevations throughout the modeled area. The results are presented in GIS databases with a public domain GIS reader. The met/ocean information has many potential uses beyond that for computing surge/wave loading on bridge superstructures.

Eighteen coastal bridges were examined in this study. The 100-year surge/wave forces and moments were computed for each span on these bridges. The resistive forces and moments (based on the superstructure dead weight) were also computed. The vulnerability index, which is the calculated forces/moments with the appropriate load factors divided by the resistive forces/moments, provides the means for determining a bridge's vulnerability. Bridges with vulnerability indices equal to or greater than one were classified as vulnerable. Of the 18 bridges analyzed, 15 had at least one span that was determined vulnerable to these types of loads.

CONCLUSIONS

While the 100-year met/ocean conditions are appropriate for most designs, there are many issues encountered by DOTD engineers where other frequency met/ocean information (e.g., 5-, 10-, 25-, 50-year return interval values) are more appropriate. For instance, engineers may design a temporary facility (a detour bridge) based on a 5-year return interval (20 percent chance of occurrence each year). Bridges, where the service life is approaching the design life, may undergo retrofitting based on a return interval different from the 100-year return interval. The information needed to produce these values exists in the Level III analysis solution files developed in the recently completed Phase 1 of the project. Results of this study provide the DOTD with higher frequency (e.g., 5-, 10-, 25-, 50-year return interval values) design surge/wave data throughout southern Louisiana.

These data were developed using the best bathymetry/topography available at the time of the study. Future conditions may differ depending on the location. For example, along barrier islands, island breaching during hurricanes may expose the bridge to elevated wave conditions. Another example could include areas where significant subsidence is an issue. For cases where historical evidence shows unstable bathymetry/topography, the designer is encouraged to use a project-specific modeling effort to account for future conditions. Additionally, design of more valuable bridge assets may warrant a detailed modeling effort commensurate with the risk. When applying the Wave and Surge Atlas results for design, good engineering practice dictates performing a sensitivity analysis to evaluate the effects of variations in storm surge level and wave heights.

The detailed vulnerability analysis identified 15 bridges as vulnerable. If any of the vulnerable bridges have constraints (tie-downs, etc.), then a more accurate assessment of the resistive forces and moments contributed by those constraints should be evaluated. For some bridges, the amount the design surge/wave load exceeds the resistive forces (span dead weight) is minimal. In those cases, the additional dead weight resistance contributed by the railings, barriers, and parapets could offset the amount of resistance exceeded by the surge/wave load. For those bridges that remain vulnerable, possible retrofit options include adding constraints or providing venting to reduce the volume of trapped air between girders. In many cases, particularly for older bridges, a more appropriate plan of action may be monitoring and eventual replacement. Implementation of countermeasures or retrofit options are at the discretion of DOTD and beyond the scope of this study.

For future bridges, the AASHTO *Guide Specifications for Bridges Vulnerable to Coastal Storms* recommends raising the low chord 1 ft. above the design (1 percent) wave crest

elevation. The wave crest elevation is the sum of the storm water level and 70 percent of the maximum wave height ($1.8 * \text{significant wave height}$), which are readily accessible using the Wave and Surge Atlas. For cases where raising the bridge above the design wave crest elevation is impractical, AASHTO recommends that bridges designed for the strength limit state include a load factor of 1.75. For bridges designed for the extreme event limit state, AASHTO recommends the wave load factor of unity. The wave force calculator provides DOTD and their consultants a tool to develop wave loads via the equations and procedures detailed in the AASHTO's *Guide Specifications for Bridges Vulnerable to Coastal Storms*.

ACRONYMS, ABBREVIATIONS, AND SYMBOLS

| | |
|-----------|--|
| AASHTO | American Association of State Highway and Transportation Officials |
| ADCIRC | Advanced Circulation Model for Coastal Ocean Hydrodynamics |
| DOTD | Department of Transportation and Development |
| ft. | foot (feet) |
| ft/sec | feet per second |
| GIS | Geographic Information System |
| LTRC | Louisiana Transportation Research Center |
| Min. | minute(s) |
| Met/Ocean | Metrological and oceanographic |
| MSL | mean sea level |
| INTERA | INTERA Incorporated |
| PBM | Physics Based Model |
| RMS | Root Mean Square |
| sec. | second |
| US | United States |

REFERENCES

1. American Association of State Highway and Transportation Officials. *Guide Specifications for Bridges Vulnerable to Coastal Storms*. 2008.
2. Kaplan, P. "Wave Impact Forces on Offshore Structures: Re-Examination and New Interpretations." Proceedings of the 24th Annual Conference on Offshore Technology, OTC 6814, Houston, TX, 1992, pp. 79-86.
3. Kaplan, P., Murray, J.J., and Yu, W.C. 1995. "Theoretical Analysis of Wave Impact Forces on Platform Deck Structures." Proceedings of the 14th International Conference on Offshore Mechanics and Arctic Engineering, Copenhagen. American Society of Mechanical Engineers, NY, 1995, pp. 189-198.
4. Morison, J.R., O'Brien, M.P., Johnson, J.W., and Schaaf, S.A. "The Forces Exerted by Surface Waves on Piles." Petroleum Trans., AIME, Vol. 189, 1950, pp. 149-157.
5. Marin, J. and Sheppard, D.M. "Storm Surge and Wave Loading on Bridge Superstructures." Don't Mess with Structural Engineers: Expanding Our Role, Proceedings of the 2009 ASCE Structures Congress, April 30–May 2, 2009, Austin, TX, pp. 1-10.

APPENDICES

Appendix A: STORM SURGE AND WAVE GIS DATABASE

Appendix B: DETAILED WAVE FORCE CALCULATIONS

APPENDIX A

Storm Surge and Wave GIS Database

A public domain ArcReader 10.1 GIS Reader is provided with the Storm Surge and Wave Database. The ArcReader10 program requires installation on the computer(s) that will access the database. To open the database, simply click on the desired Wave Atlas PMF file (5-, 10-, 25-, 50-, and 100-year return periods) after the ArcReader 10.1 application has been installed.

Workspace Layout

One of the key features in using a geo-referenced GIS map is the ease of navigation. At startup the image shown in Figure A. 1 is displayed. Notably, the screen is divided into three regions or frames, a Data Toolbar Frame, Table of Contents Frame, and a Map Display Frame as displayed in Figure A. 2.

The Data Toolbar Frame contains several common features plus several that are unique to this application. The Table of Contents Frame lists all of the graphical data that can be displayed in the Map Display Frame. Selecting or deselecting (by turning on or off the check box) a dataset or dataset group in the Table of Contents will display or turn off the set or group in the Map Display. The Map Display Frame displays the map of the coverage area at various scales and the graphical information turned on within the Table of Contents. Also, items selected using the specific tools in Data Toolbar will be displayed in tabular form in a pop-up window in or around the Data Map.

The ArcReader 10.1 software also displays information in two different views, Data View and Layout View. The Data View is the initial display mode shown when the software loads the GIS database. This view allows the user to navigate and display information in digital and tabular form. The Layout View within the ArcReader 10.1 software can be accessed by going to Edit-Layout View within the top toolbar. Layout View allows the user to see a print preview of the map with a North Arrow and Scale. Layout View does not show tabular data that is identified from the map. An example of Layout View is shown in Figure A. 3.

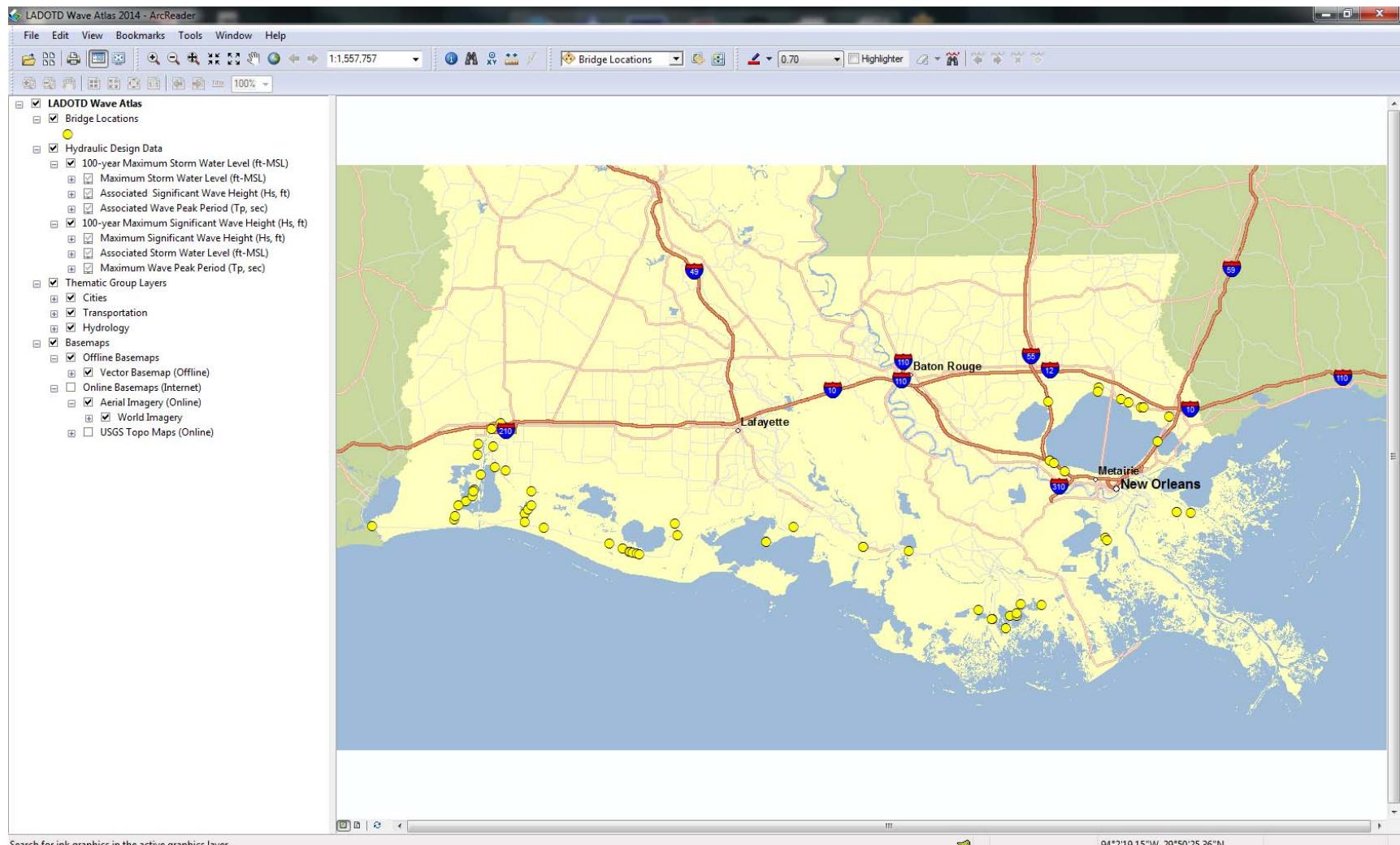


Figure A. 1
ArcGIS default opening screen

Table of Contents

Data Toolbar

Map Display

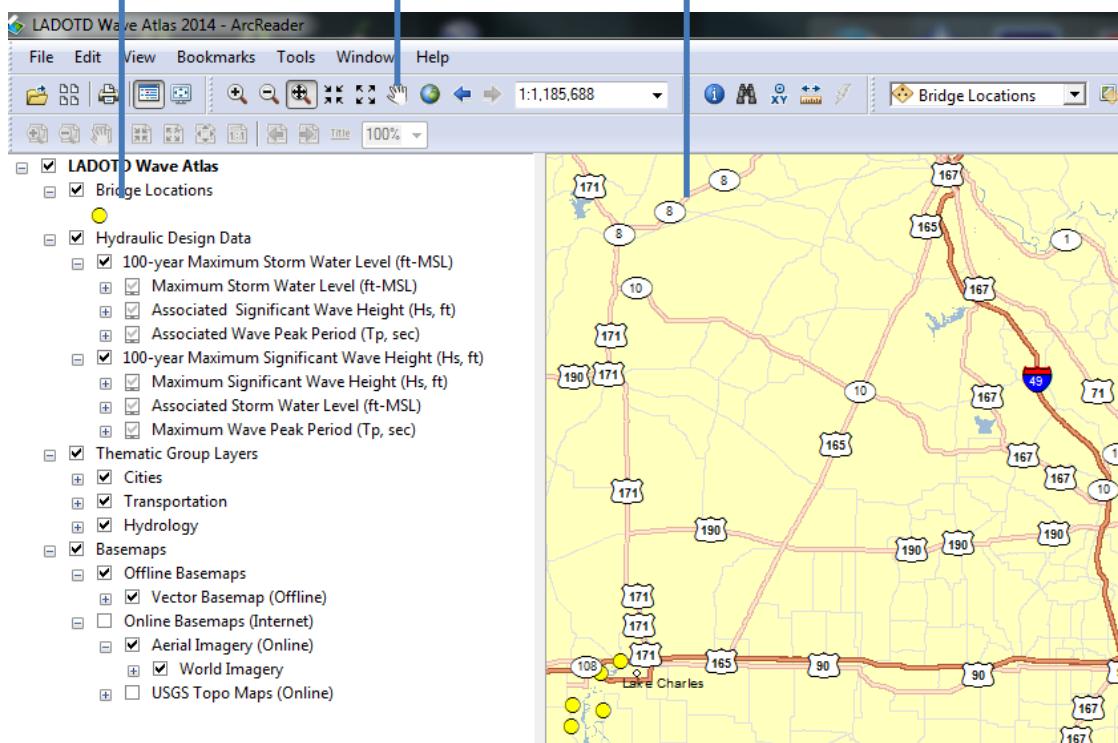


Figure A. 2
ArcGIS navigation frame layout

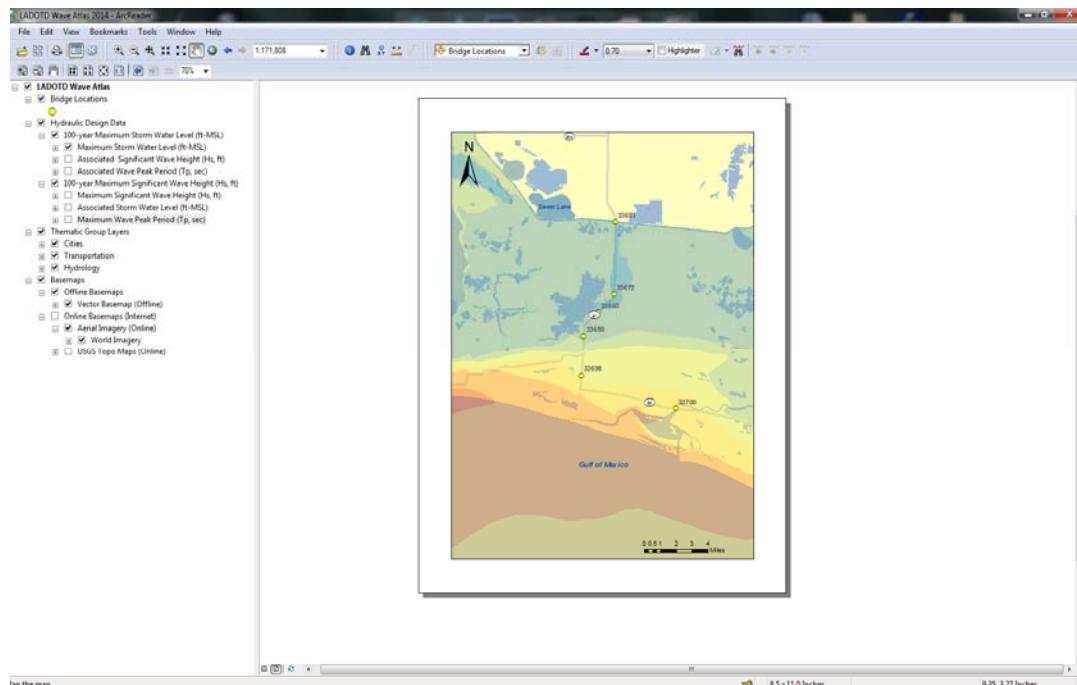


Figure A. 3
ArcGIS layout view

Data Toolbar Frame

Figure A. 4 displays the ArcReader 10.1 Toolbar. Features of this toolbar include printing, zooming, and panning the map as well as selecting various display scales and performing data searches. The data search option includes four tools for accessing and finding data within the map.

The Identify Tool, a blue circle with “i” in the center, identifies database information. The Search Tool, binoculars symbol, searches on bridge names (for bridges that have been analyzed), parishes, roadways, or features. The Hyperlinks Tool provides access to hyperlinks within the Atlas. The Coordinate Search Tool, XY symbol, searches a specific geographic position via various coordinate systems (Lat. - Long., state plane, etc.) and orients the user to bridge locations that were not part of this study. The Distance Measure Tool, ruler symbol, measures distance between any two points on the map.

The search tools in the toolbar display the desired information at a particular location on the map in a window in tabular form in the Map Frame.

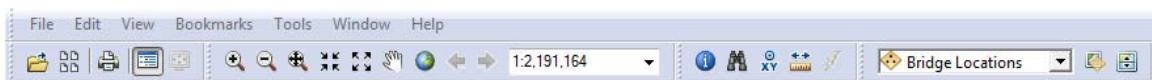


Figure A. 4
ArcGIS Reader 10.1 toolbar

Table of Contents Frame

Figure A. 5 shows the ArcReader 10.1 Navigation Frame. This frame lists the layers of graphical data contained in the Map Frame. Selecting a data set or data group causes the graphical information to be displayed in the Map Display Frame. For example, selecting 100-year Maximum Significant Wave Height displays a color coded contour map of 100-year Maximum Significant Wave Height. The color legend for the contours will be displayed in the Table of Contents Frame. To avoid confusing images, only select one contour item at a time.

Each data set is contained within a specific category or folder. There are four main groups of data within the DOTD Wave Atlas map. The four groups are Bridge Locations and Recall Numbers, Hydraulic Design Data, Thematic Group Layers, and the Basemap group. Each group can either be turned on or off on the Map Display by selecting or deselecting the checkbox to the left of the group. Each main group except for the Bridge Location and Recall Group contain subgroups. For example, the Hydraulic Design Data main group contains two subgroups, the 100-year Maximum Storm Water Level and the 100-year Maximum Significant Wave Height.

The subgroups contain data layers relating to their main group. If the subgroups are not shown, use the plus sign to expand or compress the subgroups. This Group/Subgroup methodology applies to all groups within the Table of Contents such as the Hydraulic Design Data, Thematic Group Layers, and Basemaps.

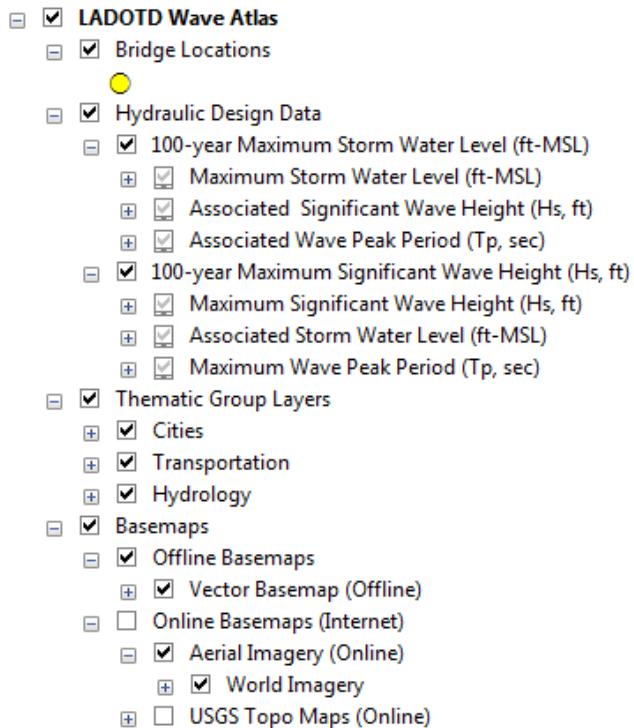


Figure A. 5
ArcGIS Reader table of contents

Map Display Frame

The ArcReader 10.1 Map Display contains, or has links to, all of the data in the database. Graphical data can be displayed in this frame by selecting it in the Table of Contents. Information at a specific location in the map can be displayed in tabular form in a superimposed window in the Map Display by specifying the location with the coordinate (XY) or search (binocular) tools and identifying the desired quantity with the identify tool. The location can also be specified by the cursor and a left click of the mouse.

Figure A. 6 displays the default map for the DOTD Storm Surge and Wave Database. The map can be panned and zoomed with the tools in the Data Toolbar. Zooming to a specific area can also occur by selecting a rectangular area with the mouse. Zooming in and out in this frame changes the information that can be displayed, i.e., some information will only be displayed when the map is zoomed in to a certain point. For example, the bridge numbers can only be

displayed when the map is zoomed in to a 1:500,000 scale. As an example, the zoomed-in image of Lake Pontchartrain shown in Figure A. 7 displays the I-10 Bridge numbers. The bridge recall numbers were not displayed prior to the magnification.

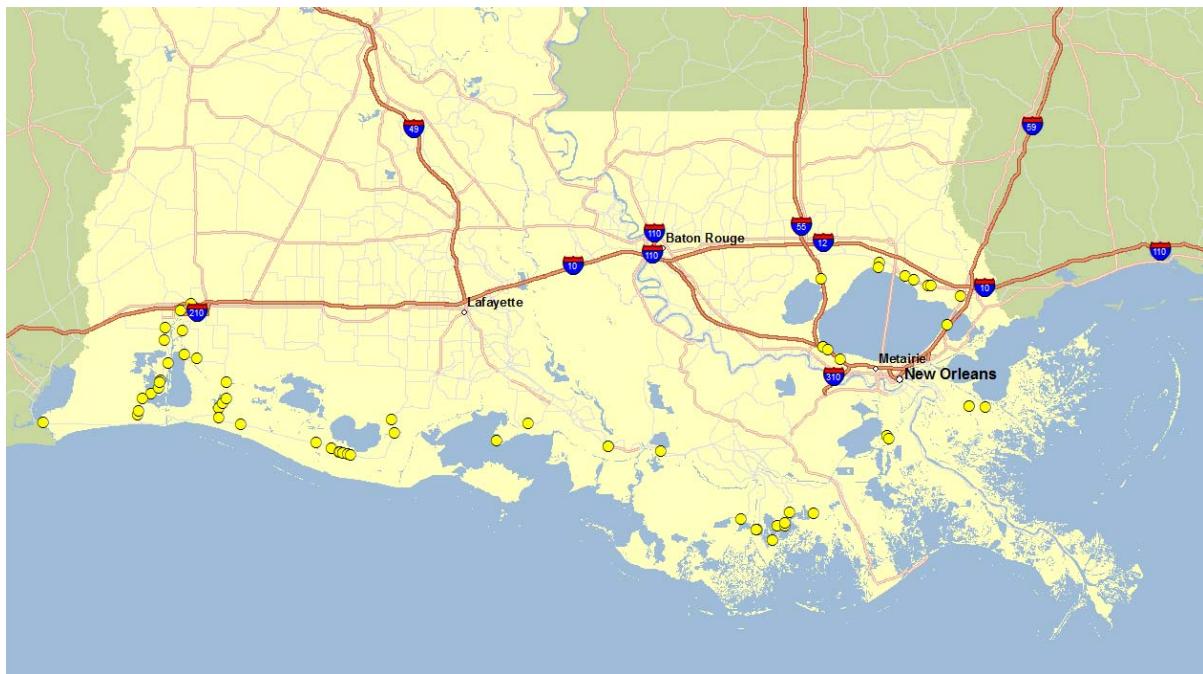


Figure A. 6
ArcGIS Reader 10.1 default map frame map

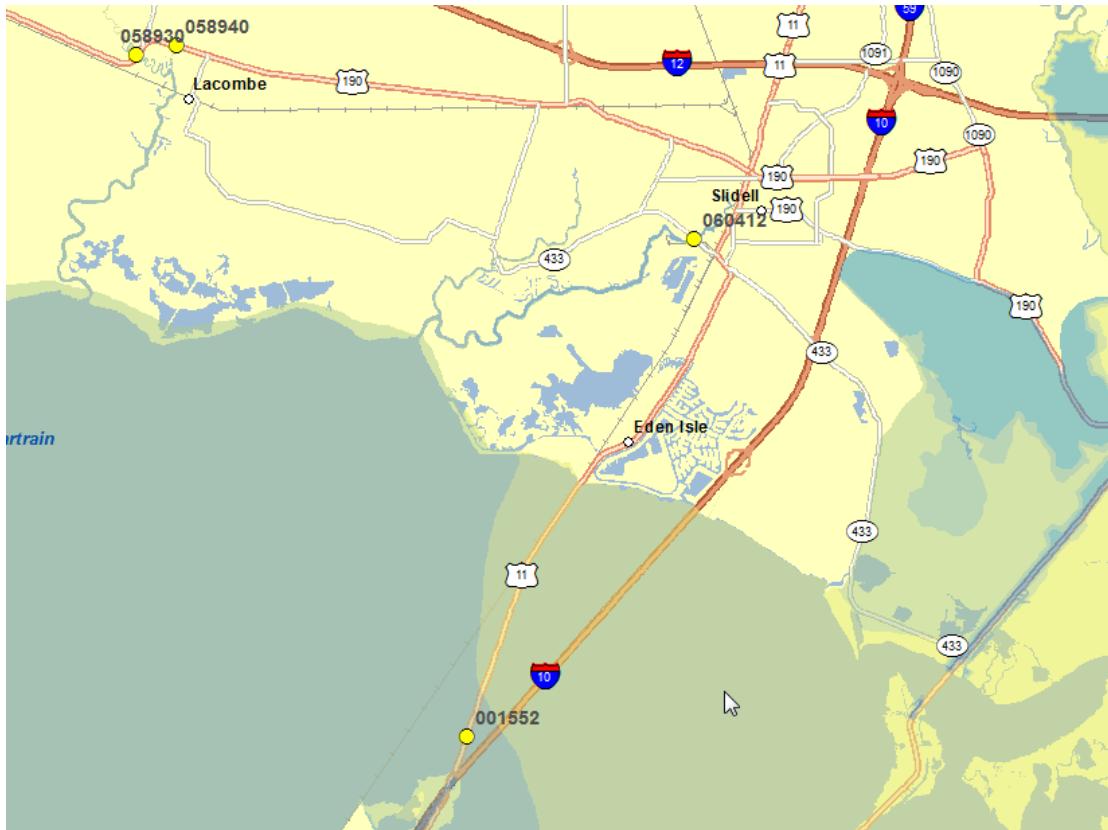


Figure A. 7
Magnified ArcGIS reader map of Lake Pontchartrain

Navigation Example – Bridge Recall Number 003440

The example below demonstrates how to obtain Hydraulic Values from the DOTD Wave Atlas GIS Database from start to finish. This example will be used further in the following section to demonstrate the Wave Force Calculator. This step-by-step example investigates Bridge Recall Number 003440.

STEP 1: OPENING THE DOTD WAVE ATLAS GIS Database

The ArcReader 10.1 software was designed only for Microsoft Windows operating systems. Initially opening the software requires navigating to the Start Menu Shortcut as displayed in FIGURE A.8. The software will be loaded but will not display any data. Accessing the data occurs by opening the software as shown in Figure A. 8 or by double clicking the DOTD Wave Atlas 100 year.pmf file within the windows operating system. ArcReader can only open published map files (.pmf) that have been created and saved within the ArcMap software. The map will now be displayed as shown in Figure A. 9.

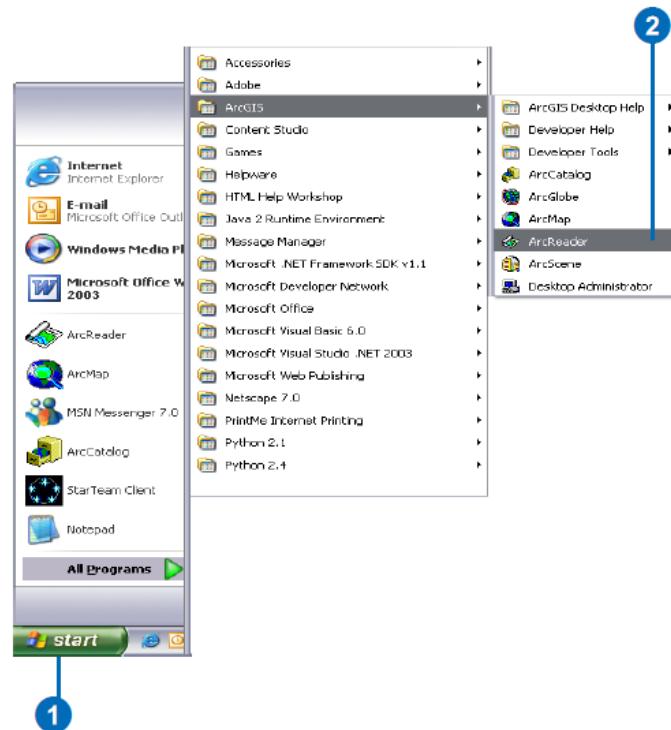


Figure A. 8
Using the windows start menu to open the ArcReader 10.1 software

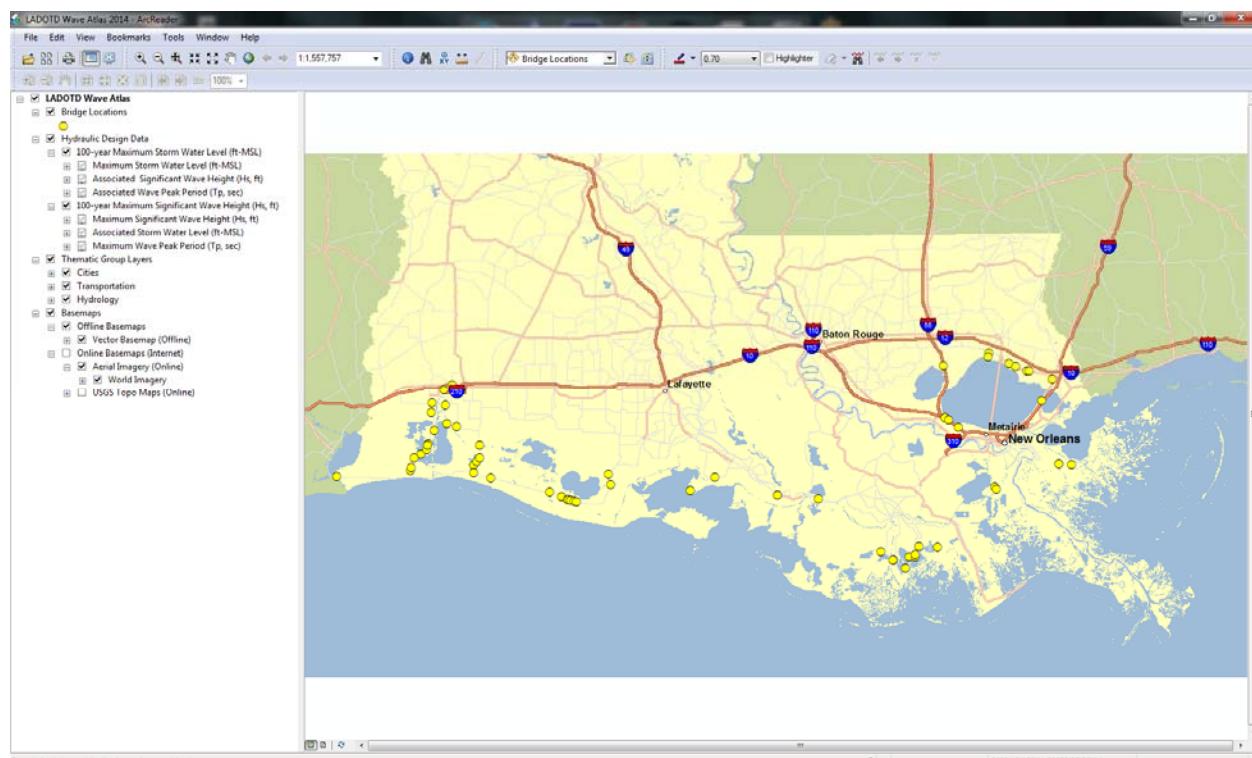


Figure A. 9

DOTD wave atlas GIS database

STEP 2: SELECTING DISPLAYED DATA

Selecting layers to display on the Display Map Frame is performed within the Table of Contents Frame. Turning on and off layers within the Table of Contents Frame occurs by checking or unchecking the selection box to the left of the dataset. This example will identify the hydraulic data values associated with the 100-year Maximum Storm Water Level. Therefore, the second subgroup under the main Hydraulic Design Data Group, the 100-year Maximum Significant Wave Height, is turned off. Turning this subgroup off occurs in one of two ways. The first way is to turn off the entire subgroup in the Table of Contents. This is performed by unchecking the box next to the subgroup heading as displayed in Figure A. 10. The second method is to turn off the subgroups data sets individually as displayed in Figure A. 11. Note, the Hydraulic Design Data Group only displays on the map at a scale of 1:500,000. If the map is at a smaller scale, there will not be any change in the Map Display Frame. Regardless, the data sets will remain turned off if the check box next to the group, subgroup or data set layer is turned off.

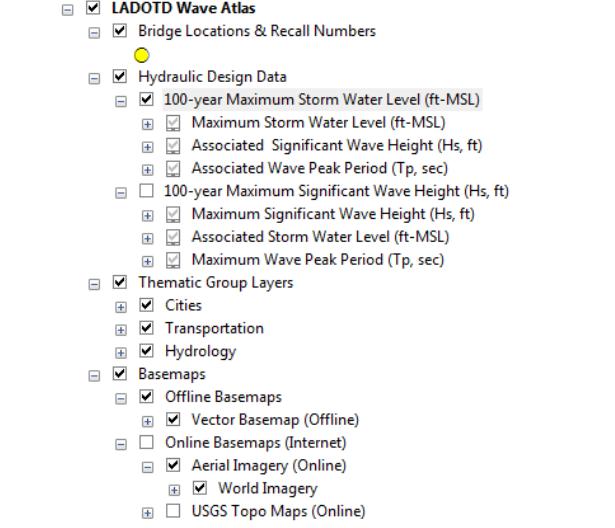
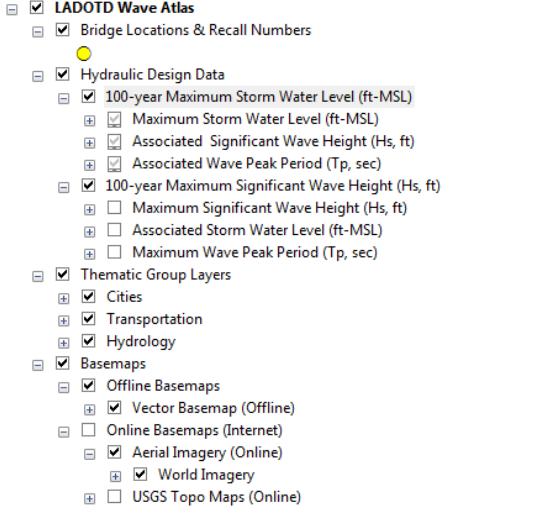
| | |
|--|--|
|  <ul style="list-style-type: none"><input checked="" type="checkbox"/> LADOTD Wave Atlas<ul style="list-style-type: none"><input checked="" type="checkbox"/> Bridge Locations & Recall Numbers<input checked="" type="checkbox"/> Hydraulic Design Data<ul style="list-style-type: none"><input checked="" type="checkbox"/> 100-year Maximum Storm Water Level (ft-MSL)<ul style="list-style-type: none"><input type="checkbox"/> Maximum Storm Water Level (ft-MSL)<input type="checkbox"/> Associated Significant Wave Height (Hs, ft)<input type="checkbox"/> Associated Wave Peak Period (Tp, sec)<input type="checkbox"/> 100-year Maximum Significant Wave Height (Hs, ft)<ul style="list-style-type: none"><input type="checkbox"/> Maximum Significant Wave Height (Hs, ft)<input type="checkbox"/> Associated Storm Water Level (ft-MSL)<input type="checkbox"/> Maximum Wave Peak Period (Tp, sec)<input checked="" type="checkbox"/> Thematic Group Layers<ul style="list-style-type: none"><input checked="" type="checkbox"/> Cities<input checked="" type="checkbox"/> Transportation<input checked="" type="checkbox"/> Hydrology<input checked="" type="checkbox"/> Basemaps<ul style="list-style-type: none"><input checked="" type="checkbox"/> Offline Basemaps<ul style="list-style-type: none"><input checked="" type="checkbox"/> Vector Basemap (Offline)<input type="checkbox"/> Online Basemaps (Internet)<ul style="list-style-type: none"><input checked="" type="checkbox"/> Aerial Imagery (Online)<input checked="" type="checkbox"/> World Imagery<input type="checkbox"/> USGS Topo Maps (Online) |  <ul style="list-style-type: none"><input checked="" type="checkbox"/> LADOTD Wave Atlas<ul style="list-style-type: none"><input checked="" type="checkbox"/> Bridge Locations & Recall Numbers<input checked="" type="checkbox"/> Hydraulic Design Data<ul style="list-style-type: none"><input checked="" type="checkbox"/> 100-year Maximum Storm Water Level (ft-MSL)<ul style="list-style-type: none"><input type="checkbox"/> Maximum Storm Water Level (ft-MSL)<input type="checkbox"/> Associated Significant Wave Height (Hs, ft)<input type="checkbox"/> Associated Wave Peak Period (Tp, sec)<input checked="" type="checkbox"/> 100-year Maximum Significant Wave Height (Hs, ft)<ul style="list-style-type: none"><input type="checkbox"/> Maximum Significant Wave Height (Hs, ft)<input type="checkbox"/> Associated Storm Water Level (ft-MSL)<input type="checkbox"/> Maximum Wave Peak Period (Tp, sec)<input checked="" type="checkbox"/> Thematic Group Layers<ul style="list-style-type: none"><input checked="" type="checkbox"/> Cities<input checked="" type="checkbox"/> Transportation<input checked="" type="checkbox"/> Hydrology<input checked="" type="checkbox"/> Basemaps<ul style="list-style-type: none"><input checked="" type="checkbox"/> Offline Basemaps<ul style="list-style-type: none"><input checked="" type="checkbox"/> Vector Basemap (Offline)<input type="checkbox"/> Online Basemaps (Internet)<ul style="list-style-type: none"><input checked="" type="checkbox"/> Aerial Imagery (Online)<input checked="" type="checkbox"/> World Imagery<input type="checkbox"/> USGS Topo Maps (Online) |
|--|--|

Figure A. 10
Method 1 of turning off subgroup data

Figure A. 11
Method 2 of turning off subgroup data

STEP 3: IDENTIFYING HYDRAULIC DESIGN DATA FOR A LOCATION

A. Locating via the Find Tool

First, select the Find Tool located in the Data Toolbar Frame. This tool looks like a pair of binoculars as displayed in Figure A. 12.

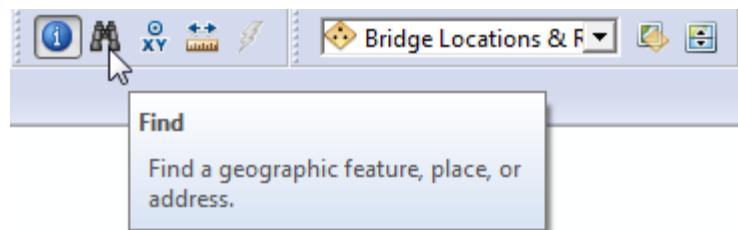


Figure A. 12
Selecting the find tool

Once this tool is selected, an information window will appear on the screen with the heading ‘Find.’ There are two tabs labeled Features and Locations. The Features Tab locates information found within the database. The Locations Tab locates information through street addresses (internet connection required). For this example, use the Features Tab.

For this example, Bridge Recall Number 003440 is located within the Bridge Locations and Recall Numbers layer. To select this layer, use the layers dropdown menu and select the appropriate layer as shown in Figure A. 13.

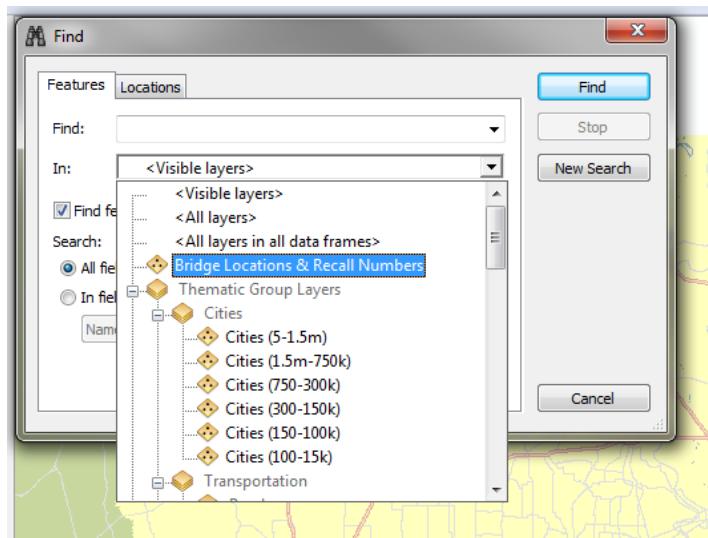


Figure A. 13
Selecting the bridge locations & recall numbers layer

Next, enter the Recall Number in the “Find” box. The box next to “Find Features that are similar to or contain the search string” remains checked. Search the recall number In field: Name as shown in Figure A.14. The Bridge Recall Number will now be displayed in the menu at the bottom after hitting the Find Button as displayed in Figure A. 15.

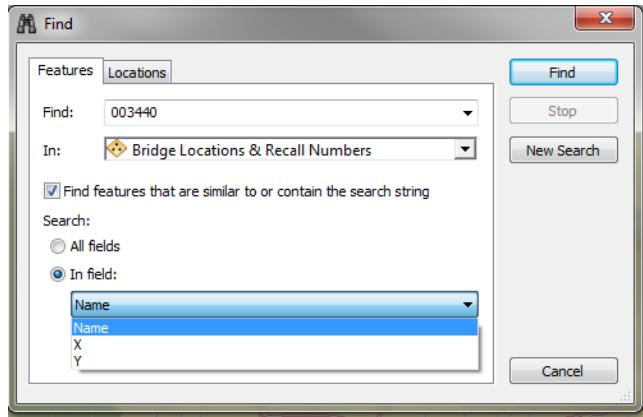


Figure A. 14
Locating bridge recall number 003440 using the find tool

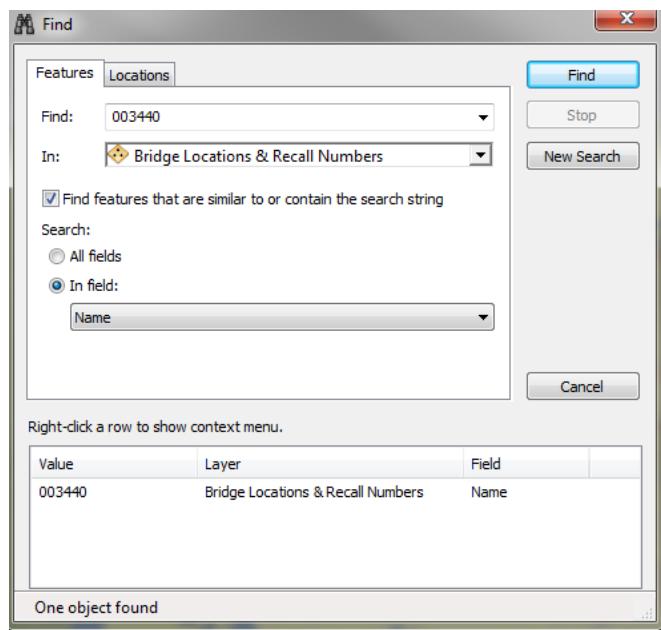


Figure A. 15
Bridge recall number displayed in find tool menu

Right-click the row containing the recall number to display the context menu (Figure A. 16). Within the context menu, “Zoom To” the feature. This will center the location relating to the Recall Number in the center of the screen as displayed in Figure A. 17.

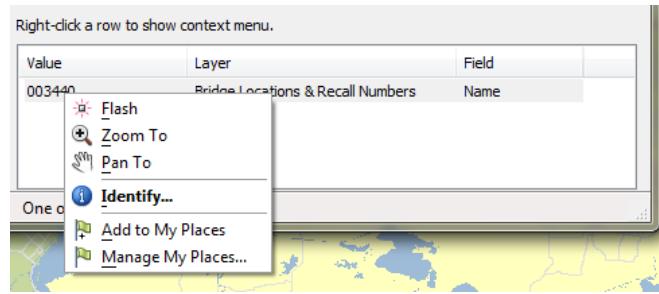


Figure A. 16
Opening the context menu

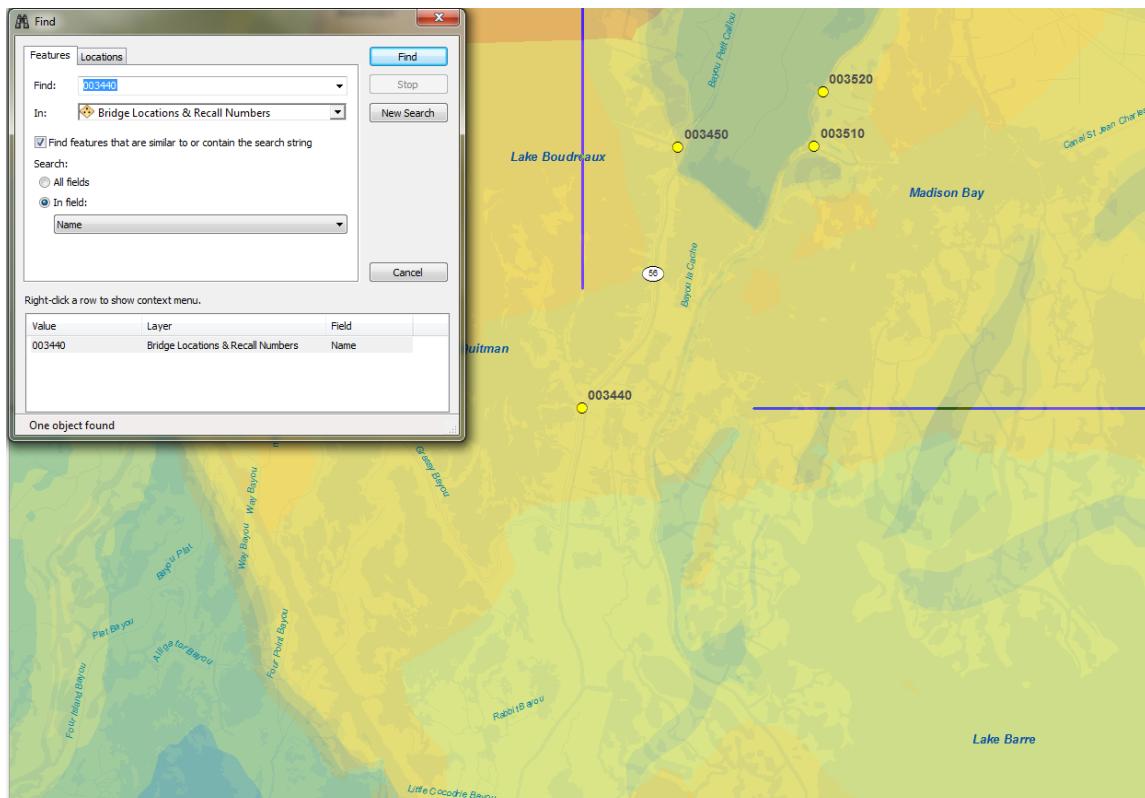


Figure A. 17
“Zooming To” the bridge recall number

Acquiring data from a location requires a scale of 1:10,000 or below. This is accomplished using the Scale Dropdown Menu located in the Data Toolbar. Figure A. 18 presents selecting 1:10,000 scale using the dropdown menu.

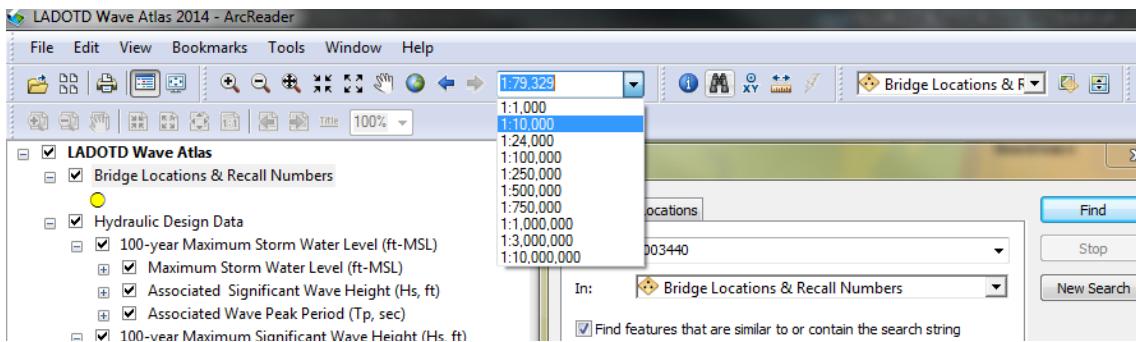


Figure A. 18
Selecting the 1:10,000 scale using the scale dropdown menu

B. Locating using the Go To XY Tool

The second method to locate hydraulic values is to use the “Go To XY” Tool. First, select the Go To XY located in the Data Toolbar Frame. This tool appears as a blue dot with XY underneath as displayed in Figure A. 19.

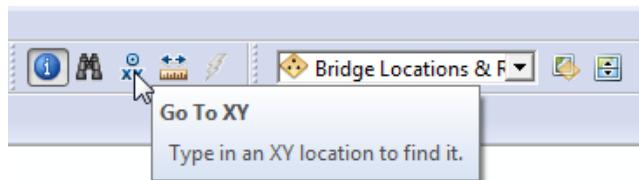


Figure A. 19
Selecting the “Go To XY” tool

The “Go To XY” Tool opens a small input window within the Map Display. The user can choose the search units by using the arrow drop down menu as displayed in Figure A. 20 and Figure A. 21. If the input is in Degrees Minutes Seconds, use a space to differentiate between degrees, minutes and seconds (i.e., 90 45 33.25W, 24 52 46.2N).

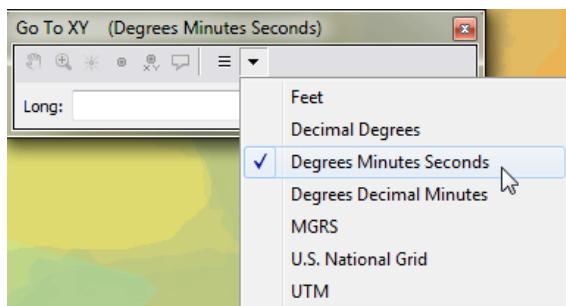


Figure A. 20
Selecting degrees minutes seconds

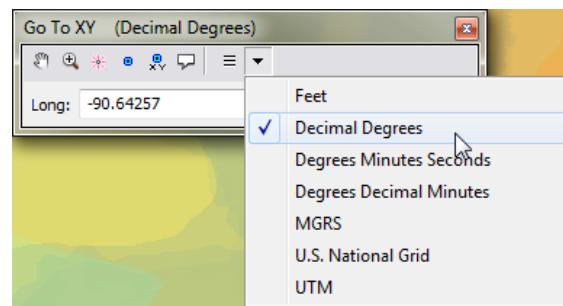


Figure A. 21
Selecting the decimal degrees

STEP 4: IDENTIFYING HYDRAULIC DATA

Identifying data is accomplished via the Identify Tool located in the Data Toolbar. This tool is located next to the Find Tool and has an icon which is a blue circle with a lowercase “i.”

Selecting the tool opens up the Identify Tool window. In this example, the data will be identified from the Hydraulic Design Data group. This group is selected using the layer dropdown menu as displayed in Figure A. 22. The data being identified should be turned on within the table of contents.

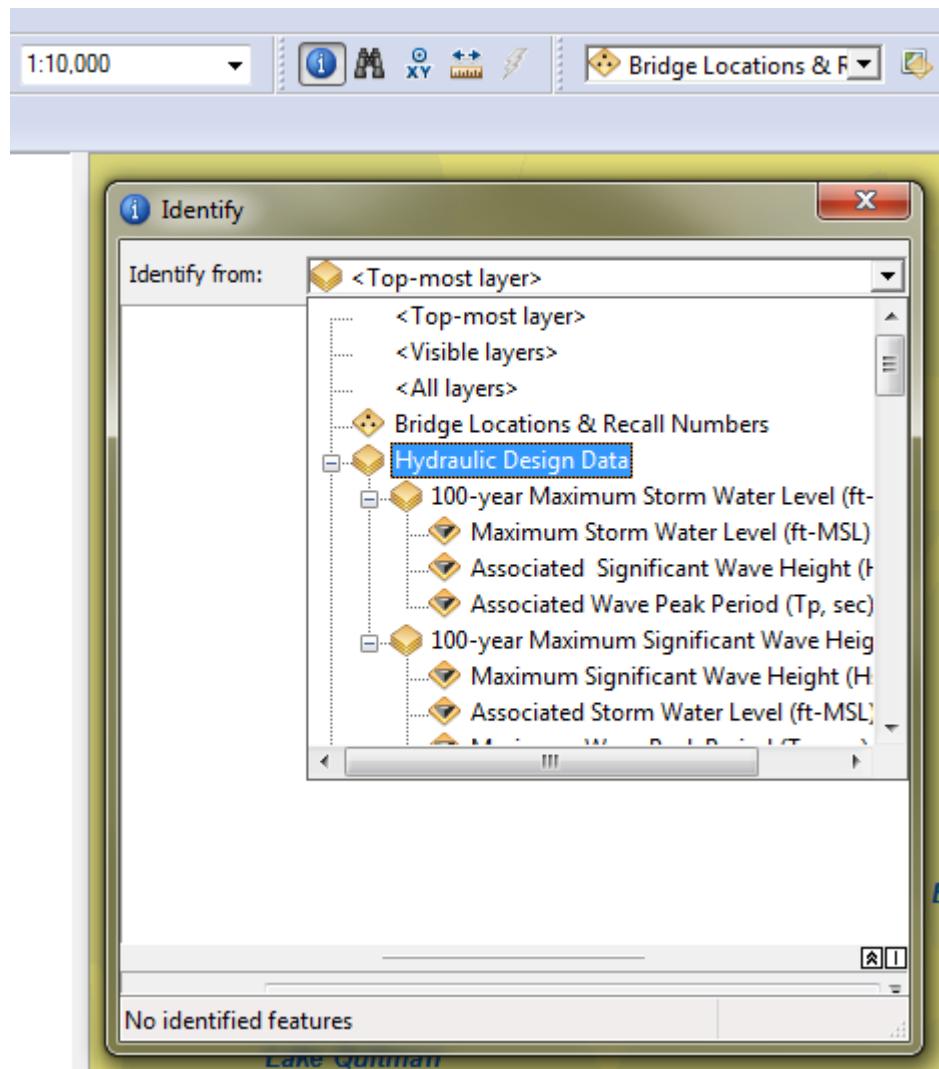


Figure A. 22
Selecting hydraulic design data group

The cursor is used to identify the hydraulic data by clicking the location of the bridge. This will now display the hydraulic design values within the Identify window. Figure A. 23 displays the screen showing the identified hydraulic values, Table on Contents, and the Map Display frame.

The identified hydraulic values associated with the two sea conditions for Bridge Recall Number 003440 are displayed in Table A. 1 and Table A. 2.

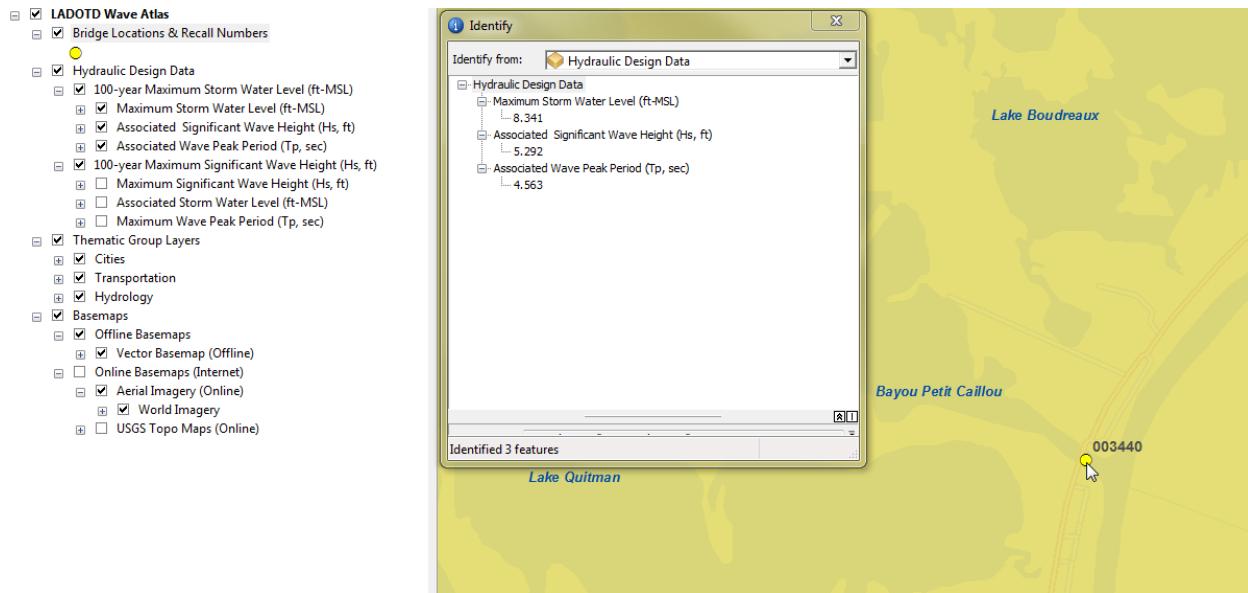


Figure A. 23
Hydraulic design data displayed for bridge recall number 003440

Table A. 1
100-year met/ocean parameters at the time of maximum storm water elevation (from 100-year wave and surge atlas)

| Maximum Storm Water Level (ft.) | Associated Significant Wave Height (ft.) | Associated Peak Wave Period (sec) |
|---------------------------------|--|-----------------------------------|
| 8.3 | 5.3 | 4.6 |

Table A. 2
100-year met/ocean parameters at the time of maximum significant wave height (from 100-year wave and surge atlas)

| Associated Storm Water Level (ft.) | Maximum Significant Wave Height (ft.) | Associated Peak Wave Period (sec) |
|------------------------------------|---------------------------------------|-----------------------------------|
| 8.1 | 5.7 | 4.4 |

Both the 100-year Maximum Significant Wave Height and Maximum Storm Water Level hydraulic design values and associated values can be displayed in the same Identify Window.

Both subgroups must be turned on (checked) in the Table of Contents. Figure A. 24 displays the Table of Contents, Identify Window, and Map Display for this scenario.

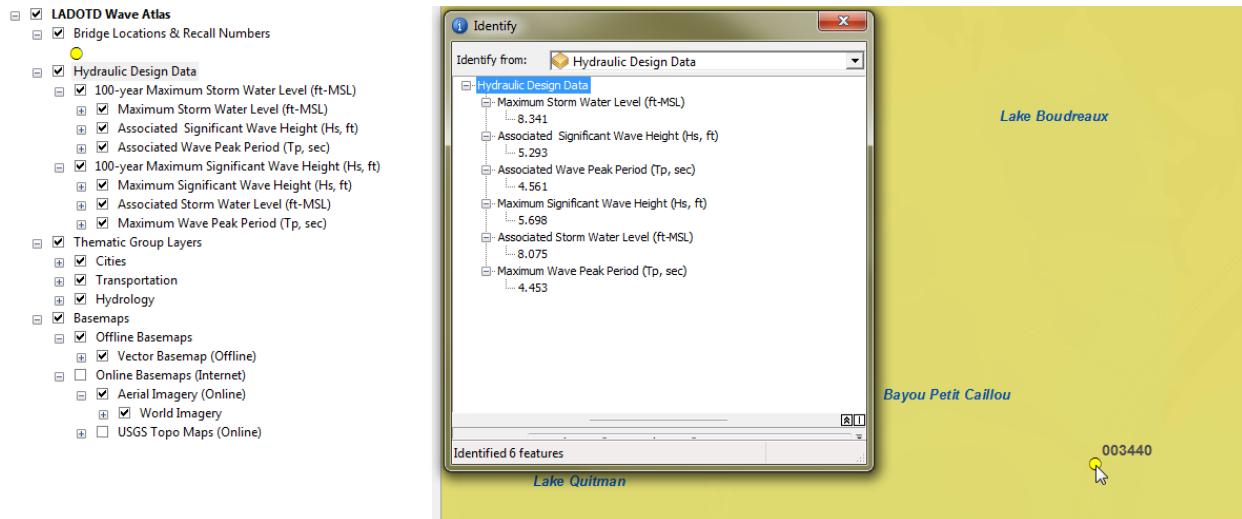


Figure A. 24
All hydraulic design data subgroups selected and identified for bridge recall number 003440

The 100-year Wave and Surge Atlas includes PDF files that detail the wave force calculations for the bridges identified as vulnerable during Phase 1. To access those data, select the Hyperlinks Tool. Once the Hyperlinks Tool cursor is selected, the bridges with hyperlinks turn blue. Selecting the desired bridge will display the PDF containing a table and plot that details the wave force information for the bridge. Figure A. 25 displays the PDF file for Bridge Recall Number 003440. To use this function, your computer must have Adobe Acrobat Reader (<https://acrobat.adobe.com/us/en/products/pdf-reader.html>) installed.

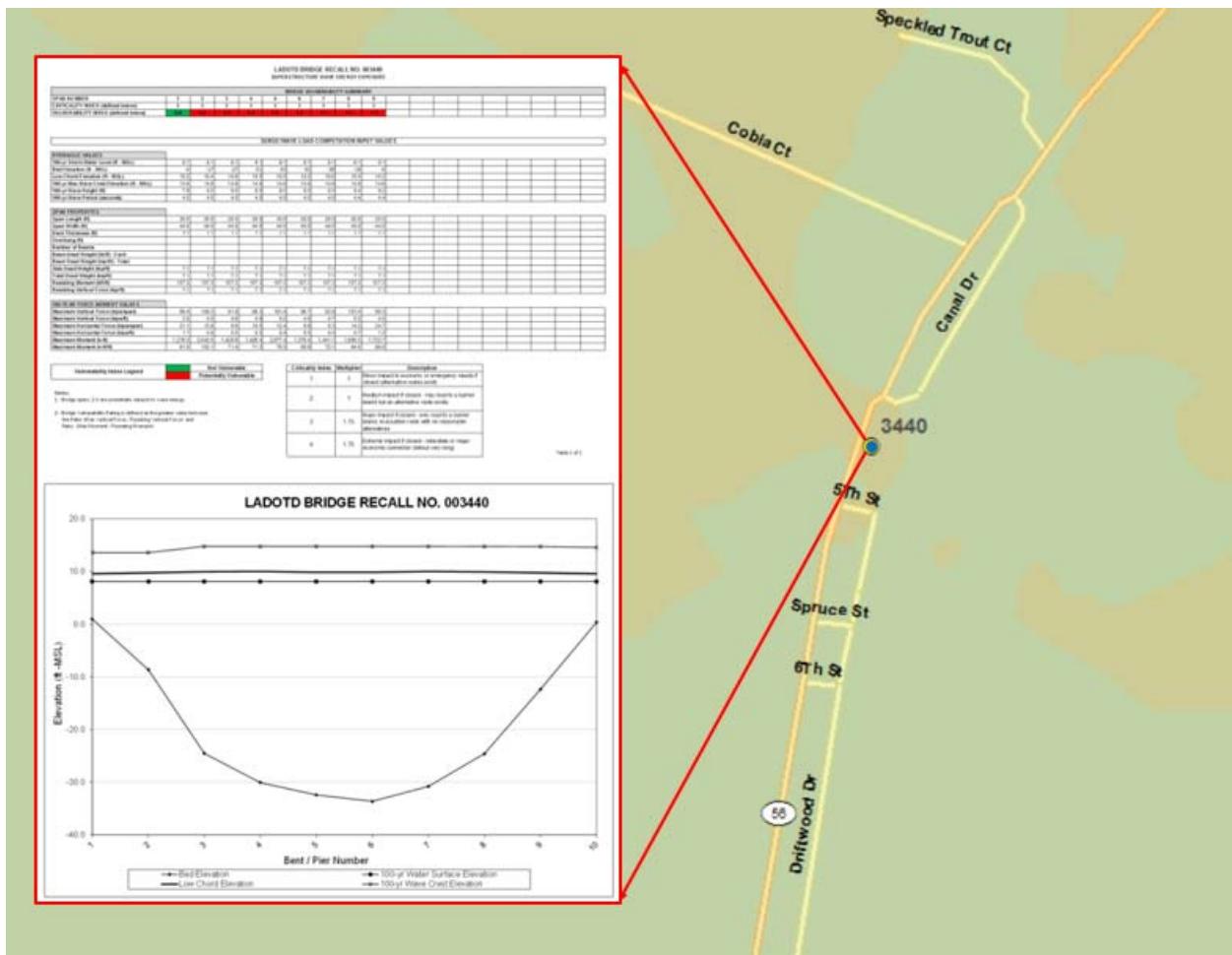


Figure A. 25
Wave force data displayed for bridge recall number 003440

Wave Force Calculator Example – Bridge Recall Number 003440

The example below demonstrates how to use the data obtained from the DOTD Wave Atlas GIS Database to calculate wave forces with the Wave Force Calculator. Each span should be evaluated for two sets of data from the DOTD Wave Atlas GIS Database — the Maximum Storm Water Level and Associated Significant Wave Height and Associated Wave Peak Period and the Maximum Significant Wave Height and Associated Storm Water Level and Associated Wave Peak Period. This step by step example calculates wave forces for Bridge Recall Number 003440.

STEP 1: Obtain the Required Bridge Superstructure Information

In order to calculate the wave forces, specific superstructure information is required. Those data include the span lengths, span widths, deck thickness, beam type, number of beams, overhang, rail height, low chord elevation, and bed elevation at each span. Ideally, these data are obtained from the as-built plans. Most of these data are found on the plan and elevation sheets. Low chord elevations are typically tabulated on a detail sheet. Table A.3 presents the superstructure information, provided by DOTD staff, for this example problem. The remainder of the data are obtained from the DOTD Wave Atlas GIS Database. Those data, presented in Table A.4 and Table A.5, include two calculated values — the clearance and the water depth. The clearance is the distance between the storm water level (Table A.4 and Table A.5) and the low chord elevation (Table A.3). For the case where the storm water level is below the low chord elevation, the clearance is positive. Conversely, for the case where the storm water level is above the low chord elevation, the clearance is negative. For this example, all of the clearance values are positive. Water depth is the distance between the storm water level and the bed elevation.

Table A. 3
Superstructure information (provided by DOTD)

| Span Number | Span Length (ft.) | Span Width (ft.) | Deck Thickness (ft.) | Beam Type | Rail Height (ft.) | Low Chord at the Bent (ft-MSL) | Bed Elevation at the Bent (ft.) |
|-------------|-------------------|------------------|----------------------|-----------|-------------------|--------------------------------|---------------------------------|
| 1 | 20.000 | 44.000 | 1.083 | Slab | 2.7 | 10.2 | -4.0 |
| 2 | 20.000 | 44.000 | 1.083 | Slab | 2.7 | 10.4 | -17.0 |
| 3 | 20.000 | 44.000 | 1.083 | Slab | 2.7 | 10.6 | -27.0 |
| 4 | 20.000 | 44.000 | 1.083 | Slab | 2.7 | 10.5 | -31.0 |
| 5 | 35.000 | 44.000 | 1.083 | Slab | 2.7 | 10.5 | -33.0 |
| 6 | 20.000 | 44.000 | 1.083 | Slab | 2.7 | 10.5 | -32.0 |
| 7 | 20.000 | 44.000 | 1.083 | Slab | 2.7 | 10.6 | -28.0 |
| 8 | 20.000 | 44.000 | 1.083 | Slab | 2.7 | 10.4 | -18.0 |
| 9 | 20.000 | 44.000 | 1.083 | Slab | 2.7 | 10.2 | -6.0 |

Table A. 4
100-year met/ocean parameters at the time of maximum storm water elevation (from 100-year wave and surge atlas)

| Span Number | Case 1 Maximum Storm Water Level and Associated Waves | | | | |
|-------------|---|--|-----------------------------------|-----------------|----------------|
| | Max Storm Water Level (ft-MSL) | Associated Significant Wave Height (ft.) | Associated Wave Peak Period (sec) | Clearance (ft.) | Depth (ft-MSL) |
| 1 | 8.3 | 5.3 | 4.6 | 1.9 | 12.3 |
| 2 | 8.3 | 5.3 | 4.6 | 2.1 | 25.3 |
| 3 | 8.3 | 5.3 | 4.6 | 2.3 | 35.3 |
| 4 | 8.3 | 5.3 | 4.6 | 2.2 | 39.3 |
| 5 | 8.3 | 5.3 | 4.6 | 2.2 | 41.3 |
| 6 | 8.3 | 5.3 | 4.6 | 2.2 | 40.3 |
| 7 | 8.3 | 5.3 | 4.6 | 2.3 | 36.3 |
| 8 | 8.3 | 5.3 | 4.6 | 2.1 | 26.3 |
| 9 | 8.3 | 5.3 | 4.6 | 1.9 | 14.3 |

Table A. 5
100-year met/ocean parameters at the time of maximum wave height (from 100-year Wave and Surge Atlas)

| Span Number | Case 2 Maximum Waves and Associated Storm Water Level | | | | |
|-------------|---|---------------------------------------|--------------------------------|-----------------|----------------|
| | Associated Storm Water Level (ft-MSL) | Maximum Significant Wave Height (ft.) | Maximum Wave Peak Period (sec) | Clearance (ft.) | Depth (ft-MSL) |
| 1 | 8.1 | 5.7 | 4.4 | 2.1 | 12.1 |
| 2 | 8.1 | 5.7 | 4.4 | 2.3 | 25.1 |
| 3 | 8.1 | 5.7 | 4.4 | 2.5 | 35.1 |
| 4 | 8.1 | 5.7 | 4.4 | 2.4 | 39.1 |
| 5 | 8.1 | 5.7 | 4.4 | 2.4 | 41.1 |
| 6 | 8.1 | 5.7 | 4.4 | 2.4 | 40.1 |
| 7 | 8.1 | 5.7 | 4.4 | 2.5 | 36.1 |
| 8 | 8.1 | 5.7 | 4.4 | 2.3 | 26.1 |
| 9 | 8.1 | 5.7 | 4.4 | 2.1 | 14.1 |

STEP 2: Running the Wave Calculator

Once all of the data is collected, the forces are calculated using the Wave Force Calculator. Begin by opening the Wave Force Calculator — Wave_Force_Calculator_Version 1.8 — and agreeing to the terms. The first step is to enter the data into the calculator. As the Methodology Section detailed, running the Wave Calculator requires inputting two sets of data —superstructure information and the sea state. This example evaluates the two cases provided in the DOTD Wave Atlas GIS Database — Case 1; the Maximum Storm Water Level and Associated Significant Wave Height and Associated Wave Peak Period and Case 2; the Maximum Significant Wave Height and Associated Storm Water Level and Associated Wave Peak Period. Comparing the wave forces produced by the two sea state conditions will determine the design conditions.

Most of the superstructure information is provided in Table A.3. Figure A.26 illustrates the superstructure input for span 1. Span length, span width, deck thickness (in this case the slab thickness), and the railing height are the values from Table A.3. The parametric wave force equations in the AASHTO specifications evaluate the vertical and horizontal forces using different inputs for the girder/slab. The equations for the vertical force employ the girder height, while the equations for the horizontal force are limited to five types of girders and

two types of slabs (see Figure A.27). For this case, a 1.083-ft. (13-in.) slab supported structure, the closest available girder type is the 21-in. Voided Slab. Since the modeled slab is 8 in. thicker than the actual slab, the horizontal force will be conservative. As Figure A.28 demonstrates, selecting one of the slab superstructures blanks the “Girder Height,” “Number of Girders,” “Overhang,” and “% Trapped Air” in the “Superstructure Information” table and inserts a note below the table that provides guidance for filling the table. For slab supported superstructures, the program sets the “Girder Height,” “Number of Girders,” and “Overhang” to zero and uses the deck thickness as the thickness of the slab in the vertical force calculations. Additionally, unlike a girder supported superstructure, a slab supported superstructure does not trap air, so the program does not use “% Trapped Air.” Finally, for span 1 of this case, the Clearance (Z_c) is obtained from Table A.4. Notably, for this example, the only value in the Superstructure Information table that changes for the different spans and the two cases, is the clearance, which is dependent on the storm water level and the low chord of that span.

| Wave Calculator | | | | | | | | | | | |
|-----------------|----------------------------|---------------------|----------------|------|-------------|------------|------------|-------------|----------------|------------|-------------|
| | Home | | Insert | | Page Layout | | Formulas | | Data | | Review |
| | Cut | Copy | Format Painter | Font | Font Size | Font Style | Font Color | Font Weight | Font Size | Font Color | Font Weight |
| | K7 | X | ✓ | fx | 16.3 | | | | | | |
| 1 | E | F | G | H | I | J | K | L | M | N | |
| 2 | | | | | | | | | | | |
| 3 | | | | | | | | | | | |
| 4 | Superstructure Information | | | | Sea State | | | | Project Name | | |
| 5 | Span | 1 | | | | | | | Project Number | | |
| 6 | Span Length (ft) | 20 | | | | | | | Parish | | |
| 7 | Span Width (ft) | 44 | | | | | | | Route | | |
| 8 | Girder Height (ft) | 0 | | | | | | | Designer | | |
| 9 | Girder Type | 21-inch Voided Slab | | | | | | | Notes: | | |
| 10 | Number of Girders | 0 | | | | | | | | | |
| 11 | Deck Thickness (ft) | 1.083 | | | | | | | | | |
| 12 | Railing Height (ft) | 2.7 | | | | | | | | | |
| 13 | Overhang (ft) | 0 | | | | | | | | | |
| 14 | % Trapped Air | 100 | | | | | | | | | |
| 15 | Clearance Zc (ft) | 2.1 | | | | | | | | | |
| 16 | | | | | | | | | | | |
| 17 | | | | | | | | | | | |
| 18 | | | | | | | | | | | |
| 19 | | | | | | | | | | | |
| 20 | | | | | | | | | | | |

Figure A. 26
Entering superstructure data

| Superstructure Information | |
|----------------------------|---------------------|
| Span | 1 |
| Span Length (ft) | 20 |
| Span Width (ft) | 44 |
| Girder Height (ft) | 0 |
| Girder Type | 21-inch Voided Slab |
| Number of Girders | AASHTO Type II |
| Deck Thickness (ft) | AASHTO Type IV |
| Railing Height (ft) | AASHTO Type VI |
| Overhang (ft) | FL Bulb-T 72 |
| % Trapped Air | FL Bulb-T 78 |
| Clearance Zc (ft) | 2-inch Voided Slab |
| | 36-inch Box Girder |
| | 2.1 |

| Sea State | |
|---------------------------------|--|
| Significant Wave Height Hs (ft) | |
| Wave Peak Period Tp (sec) | |
| Water Depth ds (ft) | |

Figure A. 27

Wave calculator drop down box for the five girder types and two slab type structures in the AASHTO horizontal force equations

| Superstructure Information | |
|----------------------------|---------------------|
| Span | 1 |
| Span Length (ft) | 20 |
| Span Width (ft) | 44 |
| | 0 |
| Girder Type | 21-inch Voided Slab |
| | 0 |
| Slab Thickness (ft) | 1.083 |
| Railing Height (ft) | 2.7 |
| | 0 |
| | 0 |
| Clearance Zc (ft) | 2.1 |

| Sea State | |
|---------------------------------|----|
| Significant Wave Height Hs (ft) | 5 |
| Wave Peak Period Tp (sec) | 6 |
| Water Depth ds (ft) | 15 |

Note - for slab bridges, the following fields are not used: Number of Girders, Overhang, % Trapped Air.

Figure A. 28

Wave calculator drop down box with the 21-inch voided slab type structure selected

The sea state data include the significant wave height, wave peak period, and water depth. As noted above, for this example those data are derived from the DOTD Wave Atlas GIS Database and summarized in Table A.4 and Table A.5 at each span for the two cases. The program converts the wave height to the appropriate design wave based on the AASHTO Guide Specifications [1]. The program provides notes in the results table documenting any changes to the wave height and period. Figure A.29 shows the completed Sea State table filled with the conditions for span 1 case 1.

| Superstructure Information | |
|----------------------------|---------------------|
| Span | 1 |
| Span Length (ft) | 20 |
| Span Width (ft) | 44 |
| Girder Height (ft) | 0 |
| Girder Type | 21-inch Voided Slab |
| Number of Girders | 0 |
| Deck Thickness (ft) | 1.083 |
| Railing Height (ft) | 2.7 |
| Overhang (ft) | 0 |
| % Trapped Air | 100 |
| Clearance Zc (ft) | 2.1 |

| Sea State | |
|---------------------------------|------|
| Significant Wave Height Hs (ft) | 5.3 |
| Wave Peak Period Tp (sec) | 4.6 |
| Water Depth ds (ft) | 16.3 |

Figure A. 29

Completed sea state table for span 1 case 1 of the example

The program provides the user with the ability to generate a report for the calculations of each span evaluated. The report can include details on the bridge including the location, project name and number, designer, and the date. Figure A.30 presents the available detailed information included in the report.

| | | | |
|-----------------|--|----------------|-----------|
| Project Name: | Example Calculations | | |
| Project Number: | LTRC Project No. 15-1ST | Recall Num: | 003440 |
| Parish: | Terrebonne | Structure Num: | Bridge |
| Route: | SR 56 | Date: | 4/27/2016 |
| Designer: | LADOTD | | |
| Notes: | Wave Force Calculator example problem. | | |

Figure A. 30
Wave force report information

The next step is to run the program and generate reports. As Figure A.31 illustrates, there are three control buttons. The “Run Wave Force Calculator” button runs the portion of the program that calculates the forces and populates Results/Input Data/Intermediate Steps table. The “Create Reports” button generates a report for each span with the numeral 1 in the first column of the Results/Input Data/Intermediate Steps Table — note when printing reports for multiple results, each span must have a unique span number. The “Reset Wave Force Calculator” resets the input data in the Superstructure Information table and the Sea State table to the default values and the clears the Results/Input Data/Intermediate Steps table.

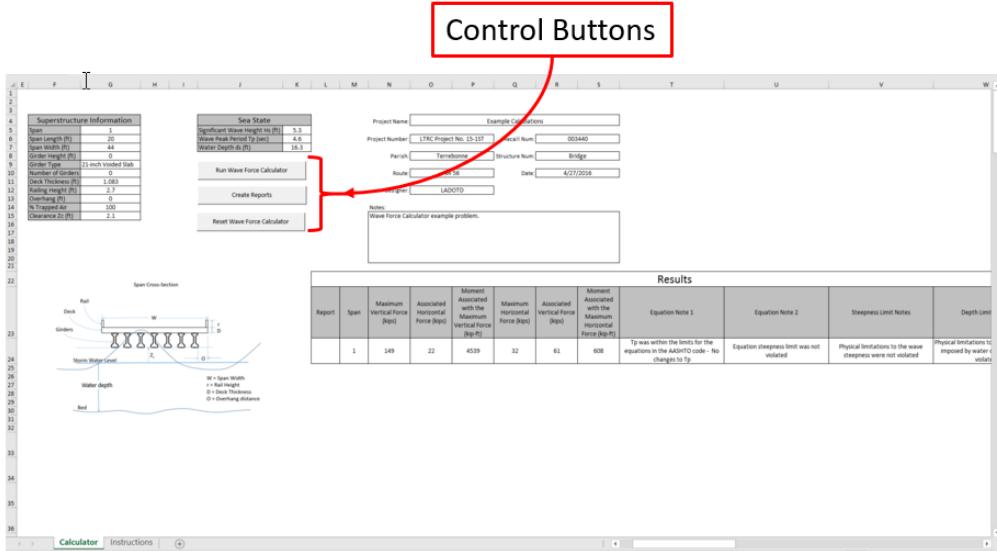
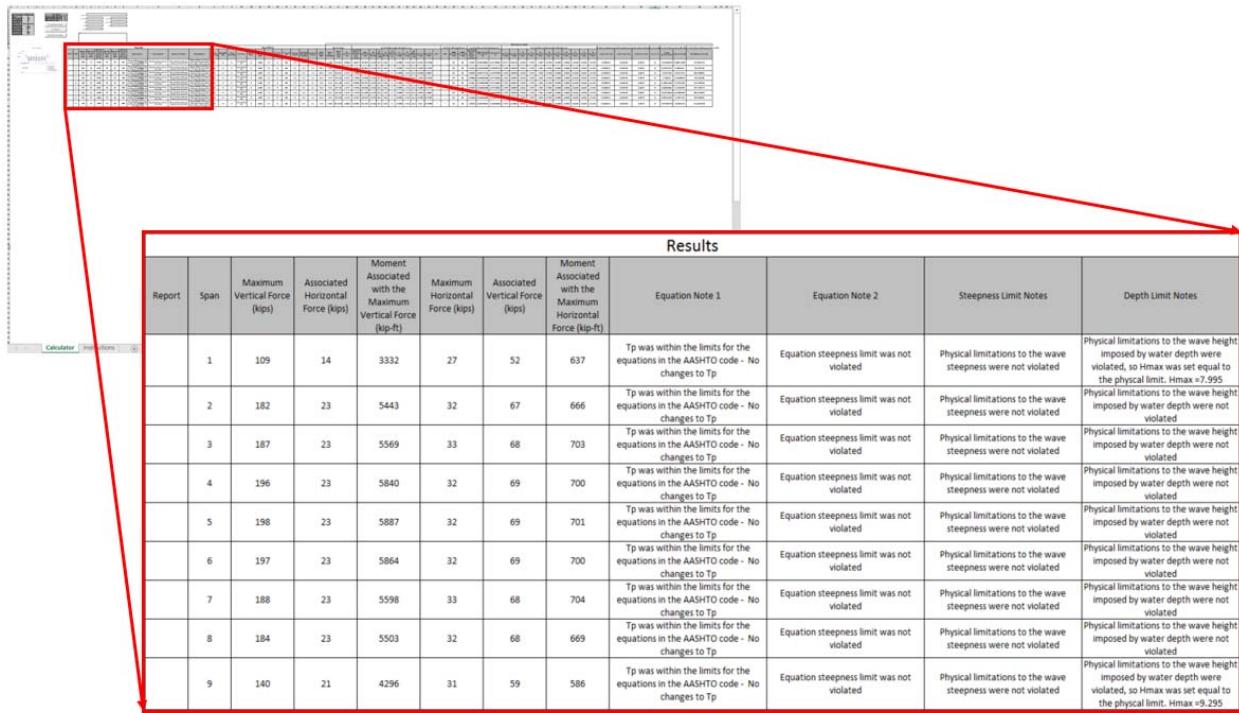


Figure A. 31
Wave force calculator control buttons

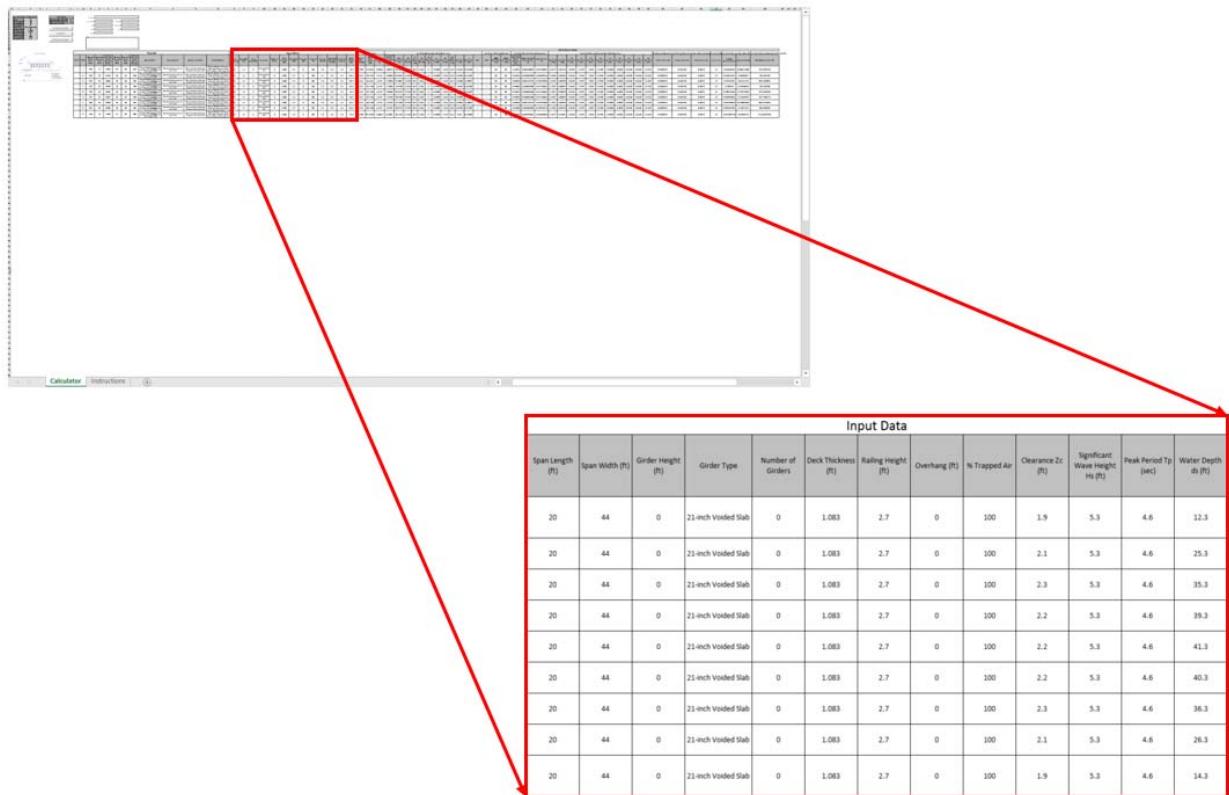
Figure A.32 through Figure A.34 presents the Results/Input Data/Intermediate Steps table from the Wave Force Calculator for all nine spans under Case 1 sea state conditions. Figure A.32 presents the results section of the combined table from the Wave Force Calculator, which provide the forces and moments and notes regarding any changes to the sea state to avoid violating the equation limits and physical limits. Figure A.33 presents the input data section of the combined table, which provides a quality control to check that the input is the data intended for the input. Figure A.34 presents the intermediate steps section of the combined table, which provides all the final wave parameters (AASHTO Guide Specifications, and the intermediate steps in the wave force calculations [1]. This provides a method to check hand calculations.

To create reports from the Wave Force Calculator, insert a “1” in the Report column of the Results/Input Data/Intermediate Steps next to each of the desired spans as demonstrated in Figure A.35. Running the Create Reports program (by clicking the Create Reports button), creates a new spreadsheet with separate tabs containing reports for each span with a “1” in the Report column.



| Results | | | | | | | | | | | |
|---------|------|-------------------------------|------------------------------------|--|---------------------------------|----------------------------------|--|---|--|---|-------------------|
| Report | Span | Maximum Vertical Force (kips) | Associated Horizontal Force (kips) | Moment Associated with the Maximum Vertical Force (kip-ft) | Maximum Horizontal Force (kips) | Associated Vertical Force (kips) | Moment Associated with the Maximum Horizontal Force (kip-ft) | Equation Note 1 | Equation Note 2 | Steepness Limit Notes | Depth Limit Notes |
| 1 | 109 | 14 | 3332 | 27 | 52 | 637 | Tp was within the limits for the equations in the AASHTO code - No changes to Tp | Equation steepness limit was not violated | Physical limitations to the wave steepness were not violated | Physical limitations to the wave height imposed by water depth were violated, so Hmax was set equal to the physical limit. Hmax = 7.995 | |
| 2 | 182 | 23 | 5443 | 32 | 67 | 666 | Tp was within the limits for the equations in the AASHTO code - No changes to Tp | Equation steepness limit was not violated | Physical limitations to the wave steepness were not violated | Physical limitations to the wave height imposed by water depth were not violated | |
| 3 | 187 | 23 | 5569 | 33 | 68 | 703 | Tp was within the limits for the equations in the AASHTO code - No changes to Tp | Equation steepness limit was not violated | Physical limitations to the wave steepness were not violated | Physical limitations to the wave height imposed by water depth were not violated | |
| 4 | 196 | 23 | 5840 | 32 | 69 | 700 | Tp was within the limits for the equations in the AASHTO code - No changes to Tp | Equation steepness limit was not violated | Physical limitations to the wave steepness were not violated | Physical limitations to the wave height imposed by water depth were not violated | |
| 5 | 198 | 23 | 5887 | 32 | 69 | 701 | Tp was within the limits for the equations in the AASHTO code - No changes to Tp | Equation steepness limit was not violated | Physical limitations to the wave steepness were not violated | Physical limitations to the wave height imposed by water depth were not violated | |
| 6 | 197 | 23 | 5864 | 32 | 69 | 700 | Tp was within the limits for the equations in the AASHTO code - No changes to Tp | Equation steepness limit was not violated | Physical limitations to the wave steepness were not violated | Physical limitations to the wave height imposed by water depth were not violated | |
| 7 | 188 | 23 | 5598 | 33 | 68 | 704 | Tp was within the limits for the equations in the AASHTO code - No changes to Tp | Equation steepness limit was not violated | Physical limitations to the wave steepness were not violated | Physical limitations to the wave height imposed by water depth were not violated | |
| 8 | 184 | 23 | 5503 | 32 | 68 | 669 | Tp was within the limits for the equations in the AASHTO code - No changes to Tp | Equation steepness limit was not violated | Physical limitations to the wave steepness were not violated | Physical limitations to the wave height imposed by water depth were not violated | |
| 9 | 140 | 21 | 4296 | 31 | 59 | 586 | Tp was within the limits for the equations in the AASHTO code - No changes to Tp | Equation steepness limit was not violated | Physical limitations to the wave steepness were not violated | Physical limitations to the wave height imposed by water depth were violated, so Hmax was set equal to the physical limit. Hmax = 9.295 | |

Figure A. 32
Case 1 results from the wave force calculator



| Input Data | | | | | | | | | | | | |
|------------------|-----------------|--------------------|---------------------|-------------------|---------------------|---------------------|---------------|---------------|-------------------|---------------------------------|----------------------|--------------------|
| Span Length (ft) | Span Width (ft) | Girder Height (ft) | Girder Type | Number of Girders | Deck Thickness (ft) | Railing Height (ft) | Overhang (ft) | % Trapped Air | Clearance Zc (ft) | Significant Wave Height Hs (ft) | Peak Period Tp (sec) | Water Depth d (ft) |
| 20 | 44 | 0 | 21-inch Veeded Slab | 0 | 1.083 | 2.7 | 0 | 100 | 1.9 | 5.3 | 4.6 | 12.3 |
| 20 | 44 | 0 | 21-inch Veeded Slab | 0 | 1.083 | 2.7 | 0 | 100 | 2.1 | 5.3 | 4.6 | 25.3 |
| 20 | 44 | 0 | 21-inch Veeded Slab | 0 | 1.083 | 2.7 | 0 | 100 | 2.3 | 5.3 | 4.6 | 35.3 |
| 20 | 44 | 0 | 21-inch Veeded Slab | 0 | 1.083 | 2.7 | 0 | 100 | 2.2 | 5.3 | 4.6 | 39.3 |
| 20 | 44 | 0 | 21-inch Veeded Slab | 0 | 1.083 | 2.7 | 0 | 100 | 2.2 | 5.3 | 4.6 | 41.3 |
| 20 | 44 | 0 | 21-inch Veeded Slab | 0 | 1.083 | 2.7 | 0 | 100 | 2.2 | 5.3 | 4.6 | 40.3 |
| 20 | 44 | 0 | 21-inch Veeded Slab | 0 | 1.083 | 2.7 | 0 | 100 | 2.3 | 5.3 | 4.6 | 36.3 |
| 20 | 44 | 0 | 21-inch Veeded Slab | 0 | 1.083 | 2.7 | 0 | 100 | 2.1 | 5.3 | 4.6 | 26.3 |
| 20 | 44 | 0 | 21-inch Veeded Slab | 0 | 1.083 | 2.7 | 0 | 100 | 1.9 | 5.3 | 4.6 | 14.3 |

Figure A. 33
Case 1 input data from the wave force calculator



Figure A. 34
Case 1 intermediate steps from the wave force calculator

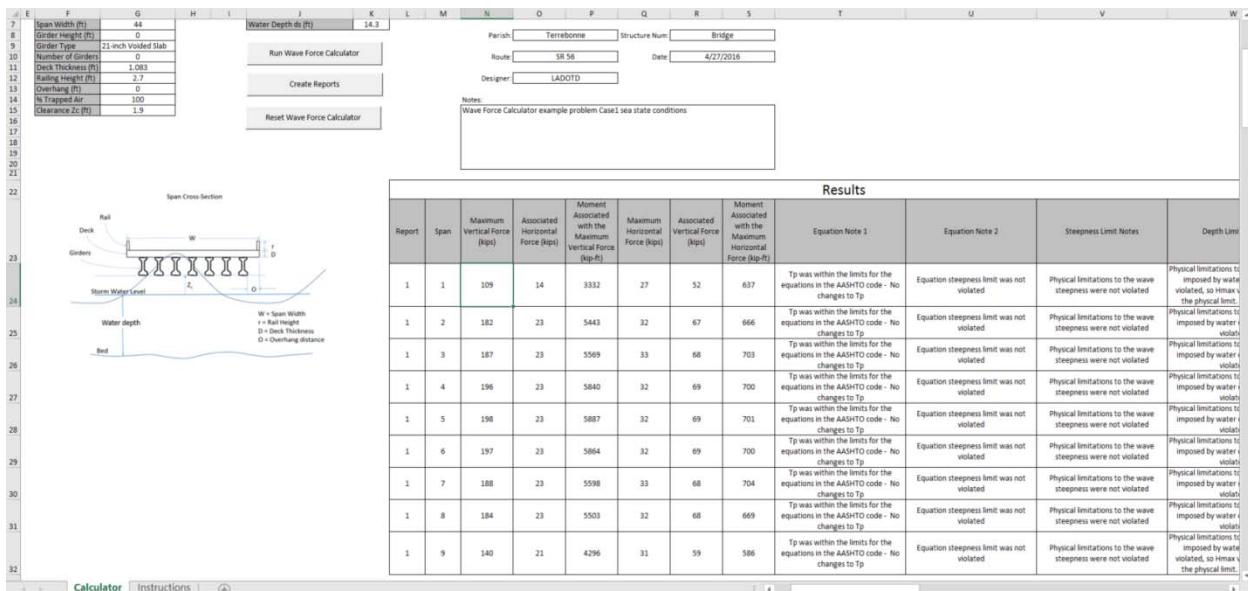


Figure A. 35
Creating reports with the wave force calculator

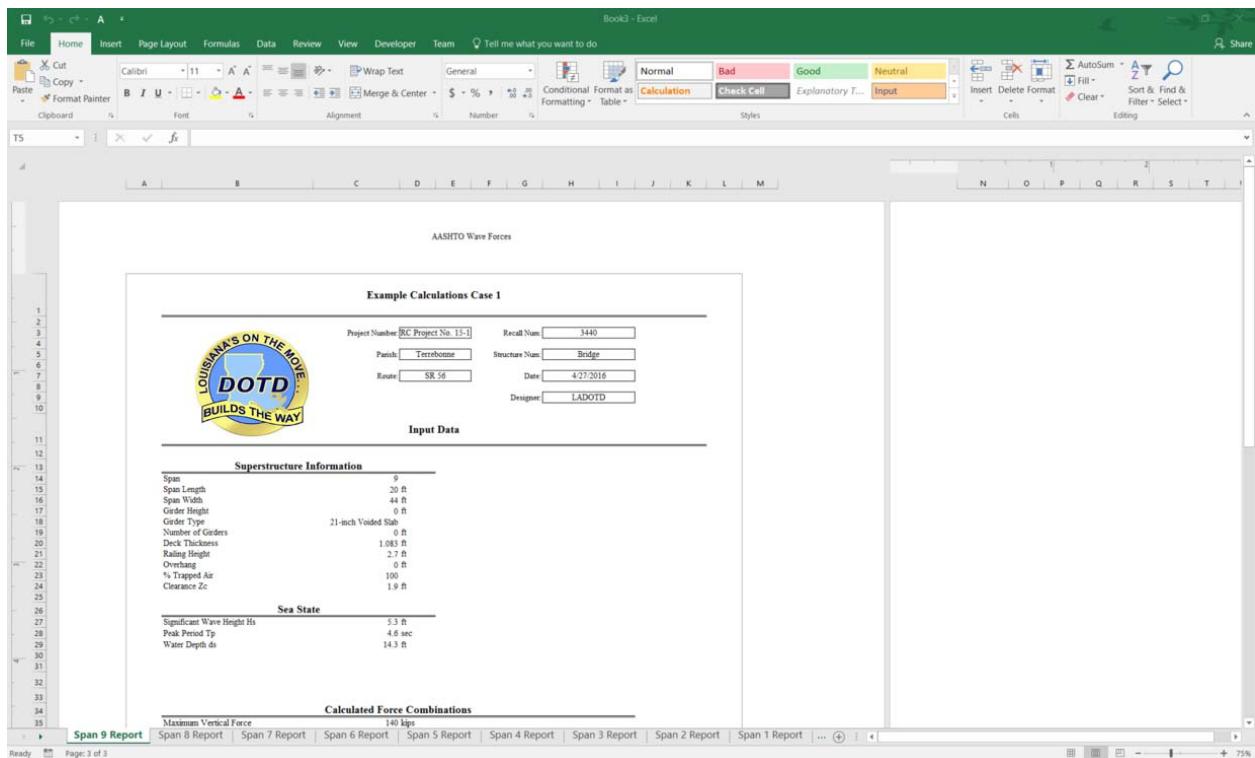


Figure A. 36
Reports from the wave force calculator

The next step involves calculating the wave forces using Case 2 sea state conditions. Figure A.37 presents the wave forces for the nine spans under Case 2 sea state conditions. The design condition is based on the case that produces the largest wave forces for each span. Figure A.38 presents a comparison of the wave forces for the two cases. As the figure demonstrates, neither case controls the forces at all the spans. For example, for Span 1 the forces produced by Case 1 sea state exceed those produced by Case 2. However, this is reversed for Span 2, where the forces produced by Case 2 sea state exceed those produced by Case 1.

| Results | | | | | | | | | | | |
|---------|------|-------------------------------|------------------------------------|--|---------------------------------|----------------------------------|--|--|---|--|---|
| Report | Span | Maximum Vertical Force (kips) | Associated Horizontal Force (kips) | Moment Associated with the Maximum Vertical Force (kip-ft) | Maximum Horizontal Force (kips) | Associated Vertical Force (kips) | Moment Associated with the Maximum Horizontal Force (kip-ft) | Equation Note 1 | Equation Note 2 | Steepness Limit Notes | Depth Limit Notes |
| | 1 | 97 | 13 | 2994 | 26 | 47 | 619 | Tp was within the limits for the equations in the AASHTO code - No changes to Tp | Equation steepness limit was not violated | Physical limitations to the wave steepness were not violated | Physical limitations to the wave height imposed by water depth were violated, so Hmax was set equal to the physical limit, Hmax = 7.865 |
| | 2 | 185 | 26 | 5658 | 34 | 63 | 591 | Tp was within the limits for the equations in the AASHTO code - No changes to Tp | Equation steepness limit was not violated | Physical limitations to the wave steepness were not violated | Physical limitations to the wave height imposed by water depth were not violated |
| | 3 | 193 | 27 | 5874 | 34 | 64 | 622 | Tp was within the limits for the equations in the AASHTO code - No changes to Tp | Equation steepness limit was not violated | Physical limitations to the wave steepness were not violated | Physical limitations to the wave height imposed by water depth were not violated |
| | 4 | 203 | 27 | 6162 | 34 | 65 | 617 | Tp was within the limits for the equations in the AASHTO code - No changes to Tp | Equation steepness limit was not violated | Physical limitations to the wave steepness were not violated | Physical limitations to the wave height imposed by water depth were not violated |
| | 5 | 205 | 27 | 6218 | 34 | 65 | 618 | Tp was within the limits for the equations in the AASHTO code - No changes to Tp | Equation steepness limit was not violated | Physical limitations to the wave steepness were not violated | Physical limitations to the wave height imposed by water depth were not violated |
| | 6 | 204 | 27 | 6191 | 34 | 65 | 618 | Tp was within the limits for the equations in the AASHTO code - No changes to Tp | Equation steepness limit was not violated | Physical limitations to the wave steepness were not violated | Physical limitations to the wave height imposed by water depth were not violated |
| | 7 | 194 | 27 | 5908 | 34 | 64 | 623 | Tp was within the limits for the equations in the AASHTO code - No changes to Tp | Equation steepness limit was not violated | Physical limitations to the wave steepness were not violated | Physical limitations to the wave height imposed by water depth were not violated |
| | 8 | 188 | 26 | 5730 | 34 | 63 | 594 | Tp was within the limits for the equations in the AASHTO code - No changes to Tp | Equation steepness limit was not violated | Physical limitations to the wave steepness were not violated | Physical limitations to the wave height imposed by water depth were not violated |
| | 9 | 128 | 20 | 3944 | 31 | 54 | 573 | Tp was within the limits for the equations in the AASHTO code - No changes to Tp | Equation steepness limit was not violated | Physical limitations to the wave steepness were not violated | Physical limitations to the wave height imposed by water depth were violated, so Hmax was set equal to the physical limit, Hmax = 9.165 |

Figure A. 37
Case 2 results from the wave force calculator

| Span | Case 1 | | | | | | Case 2 | | | | | |
|------|-------------------------------|------------------------------------|--|---------------------------------|----------------------------------|--|-------------------------------|------------------------------------|--|---------------------------------|----------------------------------|--|
| | Maximum Vertical Force (kips) | Associated Horizontal Force (kips) | Moment Associated with the Maximum Vertical Force (kip-ft) | Maximum Horizontal Force (kips) | Associated Vertical Force (kips) | Moment Associated with the Maximum Horizontal Force (kip-ft) | Maximum Vertical Force (kips) | Associated Horizontal Force (kips) | Moment Associated with the Maximum Vertical Force (kip-ft) | Maximum Horizontal Force (kips) | Associated Vertical Force (kips) | Moment Associated with the Maximum Horizontal Force (kip-ft) |
| 1 | 109 | 14 | 3332 | 27 | 52 | 637 | 97 | 13 | 2994 | 26 | 47 | 619 |
| 2 | 182 | 23 | 5443 | 32 | 67 | 666 | 185 | 26 | 5658 | 34 | 63 | 591 |
| 3 | 187 | 23 | 5569 | 33 | 68 | 703 | 193 | 27 | 5874 | 34 | 64 | 622 |
| 4 | 196 | 23 | 5840 | 32 | 69 | 700 | 203 | 27 | 6162 | 34 | 65 | 617 |
| 5 | 198 | 23 | 5887 | 32 | 69 | 701 | 205 | 27 | 6218 | 34 | 65 | 618 |
| 6 | 197 | 23 | 5864 | 32 | 69 | 700 | 204 | 27 | 6191 | 34 | 65 | 618 |
| 7 | 188 | 23 | 5598 | 33 | 68 | 704 | 194 | 27 | 5908 | 34 | 64 | 623 |
| 8 | 184 | 23 | 5503 | 32 | 68 | 669 | 188 | 26 | 5730 | 34 | 63 | 594 |
| 9 | 140 | 21 | 4296 | 31 | 59 | 586 | 128 | 20 | 3944 | 31 | 54 | 573 |

Figure A. 38
Case 1 and Case 2 wave force comparison

APPENDIX B

Detailed Wave Force calculations

Met/Ocean Data

Table B. 1
Maximum Storm Water Level and Associated Waves

| Bridge Recall Number | Span | Maximum Storm Water Level (ft-MSL) | Associated Significant Wave Height (ft.) | Associated Peak Period (sec) |
|----------------------|------|------------------------------------|--|------------------------------|
| 002631 | 1 | 8.6 | 2.9 | 4.1 |
| 002631 | 2 | 8.6 | 2.9 | 4.1 |
| 002631 | 3 | 8.6 | 2.9 | 4.1 |
| 002632 | 1 | 8.6 | 2.9 | 4.1 |
| 002632 | 2 | 8.6 | 2.9 | 4.1 |
| 002632 | 3 | 8.6 | 2.9 | 4.1 |
| 002650 | 1 | 8.0 | 2.9 | 4.0 |
| 002650 | 2 | 8.0 | 2.9 | 4.0 |
| 002650 | 3 | 8.0 | 2.9 | 4.0 |
| 002650 | 4 | 8.0 | 2.9 | 4.0 |
| 002650 | 5 | 8.0 | 2.9 | 4.0 |
| 002892 | 5 | 9.1 | 3.8 | 6.1 |
| 002892 | 6 | 9.1 | 3.8 | 6.0 |
| 002892 | 7 | 9.1 | 3.8 | 6.0 |
| 002892 | 8 | 9.1 | 3.8 | 6.0 |
| 002892 | 9 | 9.1 | 3.8 | 6.0 |
| 002892 | 10 | 9.1 | 3.8 | 6.0 |
| 002892 | 11 | 9.1 | 3.9 | 6.0 |
| 002892 | 12 | 9.1 | 3.9 | 6.0 |
| 002892 | 13 | 9.0 | 3.8 | 6.1 |
| 002892 | 14 | 9.1 | 3.9 | 6.0 |
| 002892 | 15 | 9.1 | 3.9 | 6.0 |
| 002892 | 16 | 9.1 | 3.9 | 6.0 |
| 002892 | 17 | 9.1 | 3.9 | 6.0 |
| 002892 | 18 | 9.1 | 4.0 | 6.0 |
| 002892 | 19 | 9.1 | 4.0 | 6.0 |
| 002892 | 20 | 9.1 | 4.0 | 6.0 |
| 002892 | 21 | 9.1 | 4.0 | 6.0 |

| Bridge Recall Number | Span | Maximum Storm Water Level (ft-MSL) | Associated Significant Wave Height (ft.) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|---|-------------------------------------|
| 002892 | 22 | 9.1 | 4.0 | 6.0 |
| 002892 | 23 | 9.2 | 4.0 | 6.0 |
| 002892 | 24 | 9.2 | 4.1 | 6.1 |
| 002892 | 25 | 9.2 | 4.1 | 6.1 |
| 002892 | 26 | 9.2 | 4.1 | 6.1 |
| 002892 | 27 | 9.2 | 4.1 | 6.1 |
| 002892 | 28 | 9.2 | 4.1 | 6.1 |
| 002892 | 29 | 9.1 | 4.0 | 6.0 |
| 002892 | 30 | 9.2 | 4.1 | 6.1 |
| 002892 | 31 | 9.2 | 4.1 | 6.1 |
| 002892 | 32 | 9.2 | 4.1 | 6.1 |
| 002892 | 33 | 9.2 | 4.1 | 6.1 |
| 002892 | 34 | 9.2 | 4.2 | 6.1 |
| 002892 | 35 | 9.2 | 4.1 | 6.1 |
| 002892 | 36 | 9.2 | 4.2 | 6.1 |
| 002892 | 37 | 9.2 | 4.2 | 6.1 |
| 002892 | 38 | 9.2 | 4.2 | 6.1 |
| 002892 | 39 | 9.2 | 4.2 | 6.1 |
| 002892 | 40 | 9.2 | 4.3 | 6.1 |
| 002892 | 41 | 9.2 | 4.3 | 6.1 |
| 002892 | 42 | 9.2 | 4.3 | 6.1 |
| 002892 | 43 | 9.2 | 4.3 | 6.1 |
| 002892 | 44 | 9.2 | 4.3 | 6.1 |
| 002892 | 45 | 9.2 | 4.2 | 6.1 |
| 002892 | 46 | 9.3 | 4.3 | 6.1 |
| 002892 | 47 | 9.3 | 4.3 | 6.1 |
| 002892 | 48 | 9.3 | 4.3 | 6.1 |
| 002892 | 49 | 9.3 | 4.4 | 6.1 |
| 002892 | 50 | 9.3 | 4.4 | 6.1 |
| 002892 | 51 | 9.3 | 4.4 | 6.1 |
| 002892 | 52 | 9.3 | 4.4 | 6.1 |
| 002892 | 53 | 9.3 | 4.4 | 6.1 |
| 002892 | 54 | 9.3 | 4.5 | 6.1 |
| 002892 | 55 | 9.3 | 4.3 | 6.1 |
| 002892 | 56 | 9.3 | 4.5 | 6.1 |

| Bridge Recall Number | Span | Maximum Storm Water Level (ft-MSL) | Associated Significant Wave Height (ft.) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|---|-------------------------------------|
| 002892 | 57 | 9.3 | 4.6 | 6.1 |
| 002892 | 58 | 9.3 | 4.6 | 6.1 |
| 002892 | 59 | 9.3 | 4.6 | 6.1 |
| 002892 | 60 | 9.3 | 4.6 | 6.1 |
| 002892 | 61 | 9.3 | 4.6 | 6.1 |
| 002892 | 62 | 9.3 | 4.7 | 6.1 |
| 002892 | 63 | 9.3 | 4.7 | 6.1 |
| 002892 | 64 | 9.3 | 4.7 | 6.1 |
| 002892 | 65 | 9.3 | 4.5 | 6.1 |
| 002892 | 66 | 9.3 | 4.7 | 6.1 |
| 002892 | 67 | 9.3 | 4.7 | 6.1 |
| 002892 | 68 | 9.4 | 4.7 | 6.1 |
| 002892 | 69 | 9.4 | 4.7 | 6.1 |
| 002892 | 70 | 9.4 | 4.8 | 6.1 |
| 002892 | 71 | 9.4 | 4.8 | 6.1 |
| 002892 | 72 | 9.4 | 4.8 | 6.1 |
| 002892 | 73 | 9.4 | 4.8 | 6.1 |
| 002892 | 74 | 9.4 | 4.8 | 6.1 |
| 002892 | 75 | 9.3 | 4.7 | 6.1 |
| 002892 | 76 | 9.4 | 4.8 | 6.1 |
| 002892 | 77 | 9.4 | 4.8 | 6.1 |
| 002892 | 78 | 9.4 | 4.8 | 6.1 |
| 002892 | 79 | 9.4 | 4.8 | 6.1 |
| 002892 | 80 | 9.4 | 4.8 | 6.1 |
| 002892 | 81 | 9.4 | 4.8 | 6.1 |
| 002892 | 82 | 9.4 | 4.8 | 6.1 |
| 002892 | 83 | 9.4 | 4.8 | 6.1 |
| 002892 | 84 | 9.4 | 4.8 | 6.1 |
| 002892 | 85 | 9.4 | 4.8 | 6.1 |
| 002892 | 86 | 9.4 | 4.8 | 6.1 |
| 002892 | 87 | 9.4 | 4.9 | 6.1 |
| 002892 | 88 | 9.4 | 4.9 | 6.1 |
| 002892 | 89 | 9.4 | 4.9 | 6.1 |
| 002892 | 90 | 9.4 | 4.9 | 6.1 |
| 002892 | 91 | 9.4 | 4.9 | 6.1 |

| Bridge Recall Number | Span | Maximum Storm Water Level (ft-MSL) | Associated Significant Wave Height (ft.) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|---|-------------------------------------|
| 002892 | 92 | 9.4 | 4.9 | 6.1 |
| 002892 | 93 | 9.4 | 4.9 | 6.1 |
| 002892 | 94 | 9.4 | 4.9 | 6.1 |
| 002892 | 95 | 9.4 | 4.9 | 6.1 |
| 002892 | 96 | 9.4 | 4.9 | 6.1 |
| 002892 | 97 | 9.4 | 4.9 | 6.1 |
| 002892 | 98 | 9.4 | 4.9 | 6.1 |
| 002892 | 99 | 9.4 | 4.9 | 6.0 |
| 002892 | 100 | 9.5 | 4.9 | 6.0 |
| 002892 | 101 | 9.5 | 5.0 | 6.0 |
| 002892 | 102 | 9.5 | 5.0 | 6.0 |
| 002892 | 103 | 9.5 | 5.0 | 6.0 |
| 002892 | 104 | 9.5 | 5.0 | 6.0 |
| 002892 | 105 | 9.4 | 4.9 | 6.1 |
| 002892 | 106 | 9.5 | 5.0 | 6.0 |
| 002892 | 107 | 9.5 | 5.0 | 6.1 |
| 002892 | 108 | 9.5 | 5.1 | 6.1 |
| 002892 | 109 | 9.5 | 5.1 | 6.1 |
| 002892 | 110 | 9.5 | 5.1 | 6.1 |
| 002892 | 111 | 9.5 | 5.1 | 6.1 |
| 002892 | 112 | 9.5 | 5.1 | 6.1 |
| 002892 | 113 | 9.5 | 5.2 | 6.1 |
| 002892 | 114 | 9.5 | 5.1 | 6.1 |
| 002892 | 115 | 9.5 | 5.0 | 6.1 |
| 002892 | 116 | 9.5 | 5.1 | 6.1 |
| 002892 | 117 | 9.5 | 5.1 | 6.2 |
| 002892 | 118 | 9.5 | 5.1 | 6.2 |
| 002892 | 119 | 9.5 | 5.1 | 6.2 |
| 002892 | 120 | 9.5 | 5.1 | 6.2 |
| 002892 | 121 | 9.5 | 5.1 | 6.2 |
| 002892 | 122 | 9.5 | 5.1 | 6.2 |
| 002892 | 123 | 9.6 | 5.1 | 6.1 |
| 002892 | 124 | 9.5 | 5.1 | 6.1 |
| 002892 | 125 | 9.6 | 5.1 | 6.1 |
| 002892 | 126 | 9.6 | 5.1 | 6.1 |

| Bridge Recall Number | Span | Maximum Storm Water Level (ft-MSL) | Associated Significant Wave Height (ft.) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|---|-------------------------------------|
| 002892 | 127 | 9.6 | 5.2 | 6.1 |
| 002892 | 128 | 9.6 | 5.2 | 6.1 |
| 002892 | 129 | 9.6 | 5.2 | 6.1 |
| 002892 | 130 | 9.6 | 5.2 | 6.1 |
| 002892 | 131 | 9.6 | 5.2 | 6.1 |
| 002892 | 132 | 9.6 | 5.2 | 6.1 |
| 002892 | 133 | 9.6 | 5.3 | 6.1 |
| 002892 | 134 | 9.6 | 5.1 | 6.1 |
| 002892 | 135 | 9.6 | 5.3 | 6.1 |
| 002892 | 136 | 9.6 | 5.4 | 6.1 |
| 002892 | 137 | 9.6 | 5.4 | 6.0 |
| 002892 | 138 | 9.6 | 5.4 | 6.0 |
| 002892 | 139 | 9.6 | 5.4 | 6.0 |
| 002892 | 140 | 9.6 | 5.4 | 6.0 |
| 002892 | 141 | 9.6 | 5.4 | 6.1 |
| 002892 | 142 | 9.6 | 5.4 | 6.1 |
| 002892 | 143 | 9.6 | 5.4 | 6.1 |
| 002892 | 144 | 9.6 | 5.3 | 6.1 |
| 002892 | 145 | 9.6 | 5.4 | 6.1 |
| 002892 | 146 | 9.6 | 5.5 | 6.1 |
| 002892 | 147 | 9.6 | 5.5 | 6.1 |
| 002892 | 148 | 9.6 | 5.5 | 6.0 |
| 002892 | 149 | 9.6 | 5.5 | 6.0 |
| 002892 | 150 | 9.6 | 5.5 | 6.0 |
| 002892 | 151 | 9.6 | 5.5 | 6.0 |
| 002892 | 152 | 9.6 | 5.5 | 6.0 |
| 002892 | 153 | 9.7 | 5.5 | 6.0 |
| 002892 | 154 | 9.6 | 5.4 | 6.1 |
| 002892 | 155 | 9.7 | 5.5 | 6.0 |
| 002892 | 156 | 9.7 | 5.5 | 6.1 |
| 002892 | 157 | 9.7 | 5.5 | 6.1 |
| 002892 | 158 | 9.7 | 5.5 | 6.1 |
| 002892 | 159 | 9.7 | 5.5 | 6.1 |
| 002892 | 160 | 9.7 | 5.5 | 6.1 |
| 002892 | 161 | 9.7 | 5.5 | 6.1 |

| Bridge Recall Number | Span | Maximum Storm Water Level (ft-MSL) | Associated Significant Wave Height (ft.) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|---|-------------------------------------|
| 002892 | 162 | 9.7 | 5.4 | 6.1 |
| 002892 | 163 | 9.7 | 5.5 | 6.1 |
| 002892 | 164 | 9.7 | 5.5 | 6.1 |
| 002892 | 165 | 9.7 | 5.5 | 6.1 |
| 002892 | 166 | 9.7 | 5.5 | 6.1 |
| 002892 | 167 | 9.7 | 5.5 | 6.0 |
| 002892 | 168 | 9.7 | 5.5 | 6.0 |
| 002892 | 169 | 9.7 | 5.5 | 6.0 |
| 002892 | 170 | 9.7 | 5.5 | 6.0 |
| 002892 | 171 | 9.7 | 5.5 | 6.0 |
| 002892 | 172 | 9.7 | 5.5 | 6.0 |
| 002892 | 173 | 9.7 | 5.5 | 6.0 |
| 002892 | 174 | 9.7 | 5.5 | 6.1 |
| 002892 | 175 | 9.7 | 5.5 | 6.0 |
| 002892 | 176 | 9.7 | 5.5 | 6.0 |
| 002892 | 177 | 9.7 | 5.5 | 6.0 |
| 002892 | 178 | 9.8 | 5.5 | 6.0 |
| 002892 | 179 | 9.8 | 5.5 | 6.0 |
| 002892 | 180 | 9.8 | 5.4 | 6.0 |
| 002892 | 181 | 9.8 | 5.4 | 6.1 |
| 002892 | 182 | 9.8 | 5.5 | 6.0 |
| 002892 | 183 | 9.8 | 5.5 | 6.0 |
| 002892 | 184 | 9.7 | 5.5 | 6.0 |
| 002892 | 185 | 9.8 | 5.5 | 6.0 |
| 002892 | 186 | 9.8 | 5.5 | 6.0 |
| 002892 | 187 | 9.8 | 5.5 | 6.0 |
| 002892 | 188 | 9.8 | 5.5 | 6.0 |
| 002892 | 189 | 9.8 | 5.5 | 6.0 |
| 002892 | 190 | 9.8 | 5.5 | 6.0 |
| 002892 | 191 | 9.8 | 5.5 | 6.0 |
| 002892 | 192 | 9.8 | 5.5 | 6.0 |
| 002892 | 193 | 9.8 | 5.5 | 6.0 |
| 002892 | 194 | 9.8 | 5.5 | 6.0 |
| 002892 | 195 | 9.8 | 5.5 | 6.0 |
| 002892 | 196 | 9.8 | 5.5 | 6.0 |

| Bridge Recall Number | Span | Maximum Storm Water Level (ft-MSL) | Associated Significant Wave Height (ft.) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|---|-------------------------------------|
| 002892 | 197 | 9.8 | 5.4 | 6.0 |
| 002892 | 198 | 9.8 | 5.4 | 6.0 |
| 002892 | 199 | 9.8 | 5.4 | 6.0 |
| 002892 | 200 | 9.8 | 5.4 | 6.0 |
| 002892 | 201 | 9.9 | 5.3 | 6.0 |
| 002892 | 202 | 9.9 | 5.3 | 6.0 |
| 002892 | 203 | 9.9 | 5.3 | 6.0 |
| 002892 | 204 | 9.9 | 5.2 | 6.1 |
| 002892 | 205 | 9.9 | 5.2 | 6.1 |
| 002892 | 206 | 9.9 | 5.2 | 6.1 |
| 002892 | 207 | 9.9 | 5.2 | 6.1 |
| 002892 | 208 | 9.9 | 5.2 | 6.1 |
| 002892 | 209 | 9.9 | 5.2 | 6.1 |
| 002892 | 210 | 9.9 | 5.2 | 6.0 |
| 002892 | 211 | 9.9 | 5.2 | 6.0 |
| 002892 | 212 | 9.9 | 5.2 | 6.0 |
| 002892 | 213 | 9.9 | 5.2 | 6.0 |
| 002892 | 214 | 9.9 | 5.2 | 6.0 |
| 002892 | 215 | 9.9 | 5.2 | 6.0 |
| 002892 | 216 | 9.9 | 5.2 | 6.0 |
| 002892 | 217 | 9.9 | 5.2 | 6.0 |
| 002892 | 218 | 9.8 | 5.4 | 6.0 |
| 002892 | 219 | 9.9 | 5.2 | 6.0 |
| 002892 | 220 | 9.9 | 5.2 | 6.0 |
| 002892 | 221 | 10.0 | 5.1 | 6.0 |
| 002892 | 222 | 10.0 | 5.1 | 6.0 |
| 002892 | 223 | 10.0 | 5.1 | 6.0 |
| 002892 | 224 | 10.0 | 5.0 | 6.0 |
| 002892 | 225 | 10.0 | 5.0 | 6.0 |
| 002892 | 226 | 10.0 | 4.9 | 6.0 |
| 002892 | 227 | 10.0 | 4.9 | 6.0 |
| 002892 | 228 | 10.0 | 4.9 | 6.0 |
| 002892 | 229 | 10.0 | 4.8 | 6.0 |
| 002892 | 230 | 10.0 | 4.8 | 6.1 |
| 002892 | 231 | 10.0 | 4.7 | 6.1 |

| Bridge Recall Number | Span | Maximum Storm Water Level (ft-MSL) | Associated Significant Wave Height (ft.) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|---|-------------------------------------|
| 002892 | 232 | 10.0 | 4.7 | 6.1 |
| 002892 | 233 | 10.0 | 4.6 | 6.1 |
| 002892 | 234 | 10.1 | 4.6 | 6.1 |
| 002892 | 235 | 10.1 | 4.6 | 6.1 |
| 002892 | 236 | 10.1 | 4.6 | 6.1 |
| 002892 | 237 | 10.1 | 4.6 | 6.1 |
| 002892 | 238 | 10.1 | 4.5 | 6.1 |
| 002892 | 239 | 10.1 | 4.5 | 6.1 |
| 002892 | 240 | 10.1 | 4.5 | 6.1 |
| 002892 | 241 | 10.0 | 5.1 | 6.0 |
| 002892 | 242 | 10.1 | 4.5 | 6.1 |
| 002894 | 5 | 9.7 | 5.6 | 6.1 |
| 002894 | 6 | 9.7 | 5.6 | 6.1 |
| 002894 | 7 | 9.0 | 3.8 | 6.1 |
| 002894 | 8 | 9.0 | 3.9 | 6.1 |
| 002894 | 9 | 9.0 | 3.9 | 6.1 |
| 002894 | 10 | 9.1 | 3.9 | 6.1 |
| 002894 | 11 | 9.1 | 3.9 | 6.1 |
| 002894 | 12 | 9.1 | 3.9 | 6.1 |
| 002894 | 13 | 9.1 | 3.9 | 6.1 |
| 002894 | 14 | 9.1 | 3.9 | 6.1 |
| 002894 | 15 | 9.0 | 3.8 | 6.1 |
| 002894 | 16 | 9.1 | 4.0 | 6.1 |
| 002894 | 17 | 9.1 | 4.0 | 6.1 |
| 002894 | 18 | 9.1 | 4.0 | 6.1 |
| 002894 | 19 | 9.1 | 4.0 | 6.1 |
| 002894 | 20 | 9.1 | 4.1 | 6.1 |
| 002894 | 21 | 9.1 | 4.1 | 6.1 |
| 002894 | 22 | 9.1 | 4.1 | 6.1 |
| 002894 | 23 | 9.1 | 4.1 | 6.1 |
| 002894 | 24 | 9.1 | 4.0 | 6.1 |
| 002894 | 25 | 9.1 | 4.1 | 6.1 |
| 002894 | 26 | 9.1 | 4.1 | 6.1 |
| 002894 | 27 | 9.1 | 4.1 | 6.1 |
| 002894 | 28 | 9.1 | 4.1 | 6.1 |

| Bridge Recall Number | Span | Maximum Storm Water Level (ft-MSL) | Associated Significant Wave Height (ft.) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|---|-------------------------------------|
| 002894 | 29 | 9.2 | 4.1 | 6.1 |
| 002894 | 30 | 9.2 | 4.1 | 6.1 |
| 002894 | 31 | 9.2 | 4.2 | 6.1 |
| 002894 | 32 | 9.2 | 4.2 | 6.1 |
| 002894 | 33 | 9.2 | 4.2 | 6.1 |
| 002894 | 34 | 9.2 | 4.2 | 6.1 |
| 002894 | 35 | 9.2 | 4.2 | 6.1 |
| 002894 | 36 | 9.2 | 4.3 | 6.1 |
| 002894 | 37 | 9.2 | 4.2 | 6.1 |
| 002894 | 38 | 9.2 | 4.3 | 6.1 |
| 002894 | 39 | 9.2 | 4.3 | 6.1 |
| 002894 | 40 | 9.2 | 4.4 | 6.1 |
| 002894 | 41 | 9.2 | 4.4 | 6.1 |
| 002894 | 42 | 9.2 | 4.4 | 6.1 |
| 002894 | 43 | 9.2 | 4.4 | 6.1 |
| 002894 | 44 | 9.2 | 4.4 | 6.1 |
| 002894 | 45 | 9.2 | 4.4 | 6.1 |
| 002894 | 46 | 9.2 | 4.5 | 6.1 |
| 002894 | 47 | 9.2 | 4.3 | 6.1 |
| 002894 | 48 | 9.2 | 4.5 | 6.1 |
| 002894 | 49 | 9.2 | 4.6 | 6.1 |
| 002894 | 50 | 9.2 | 4.6 | 6.1 |
| 002894 | 51 | 9.3 | 4.6 | 6.1 |
| 002894 | 52 | 9.3 | 4.6 | 6.1 |
| 002894 | 53 | 9.3 | 4.6 | 6.1 |
| 002894 | 54 | 9.3 | 4.7 | 6.0 |
| 002894 | 55 | 9.3 | 4.7 | 6.1 |
| 002894 | 56 | 9.3 | 4.7 | 6.1 |
| 002894 | 57 | 9.2 | 4.5 | 6.1 |
| 002894 | 58 | 9.3 | 4.7 | 6.1 |
| 002894 | 59 | 9.3 | 4.7 | 6.1 |
| 002894 | 60 | 9.3 | 4.7 | 6.1 |
| 002894 | 61 | 9.3 | 4.8 | 6.1 |
| 002894 | 62 | 9.3 | 4.8 | 6.1 |
| 002894 | 63 | 9.3 | 4.8 | 6.1 |

| Bridge Recall Number | Span | Maximum Storm Water Level (ft-MSL) | Associated Significant Wave Height (ft.) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|---|-------------------------------------|
| 002894 | 64 | 9.3 | 4.8 | 6.1 |
| 002894 | 65 | 9.3 | 4.8 | 6.1 |
| 002894 | 66 | 9.3 | 4.8 | 6.1 |
| 002894 | 67 | 9.3 | 4.7 | 6.1 |
| 002894 | 68 | 9.3 | 4.9 | 6.1 |
| 002894 | 69 | 9.3 | 4.9 | 6.1 |
| 002894 | 70 | 9.3 | 4.9 | 6.1 |
| 002894 | 71 | 9.3 | 4.9 | 6.1 |
| 002894 | 72 | 9.3 | 5.0 | 6.1 |
| 002894 | 73 | 9.3 | 5.0 | 6.1 |
| 002894 | 74 | 9.3 | 5.0 | 6.1 |
| 002894 | 75 | 9.3 | 5.0 | 6.1 |
| 002894 | 76 | 9.3 | 5.0 | 6.1 |
| 002894 | 77 | 9.3 | 4.9 | 6.1 |
| 002894 | 78 | 9.4 | 5.0 | 6.1 |
| 002894 | 79 | 9.4 | 4.9 | 6.1 |
| 002894 | 80 | 9.4 | 4.9 | 6.1 |
| 002894 | 81 | 9.4 | 5.0 | 6.1 |
| 002894 | 82 | 9.4 | 5.0 | 6.1 |
| 002894 | 83 | 9.4 | 5.0 | 6.1 |
| 002894 | 84 | 9.4 | 5.0 | 6.0 |
| 002894 | 85 | 9.4 | 5.0 | 6.1 |
| 002894 | 86 | 9.4 | 5.0 | 6.1 |
| 002894 | 87 | 9.4 | 4.9 | 6.1 |
| 002894 | 88 | 9.4 | 5.0 | 6.1 |
| 002894 | 89 | 9.4 | 5.0 | 6.1 |
| 002894 | 90 | 9.4 | 5.0 | 6.1 |
| 002894 | 91 | 9.4 | 5.0 | 6.1 |
| 002894 | 92 | 9.4 | 5.0 | 6.1 |
| 002894 | 93 | 9.4 | 5.0 | 6.1 |
| 002894 | 94 | 9.4 | 5.0 | 6.1 |
| 002894 | 95 | 9.4 | 5.0 | 6.1 |
| 002894 | 96 | 9.4 | 5.0 | 6.1 |
| 002894 | 97 | 9.4 | 5.0 | 6.1 |
| 002894 | 98 | 9.4 | 5.0 | 6.1 |

| Bridge Recall Number | Span | Maximum Storm Water Level (ft-MSL) | Associated Significant Wave Height (ft.) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|---|-------------------------------------|
| 002894 | 99 | 9.4 | 5.0 | 6.1 |
| 002894 | 100 | 9.4 | 5.0 | 6.1 |
| 002894 | 101 | 9.4 | 5.0 | 6.1 |
| 002894 | 102 | 9.4 | 5.1 | 6.1 |
| 002894 | 103 | 9.4 | 5.1 | 6.1 |
| 002894 | 104 | 9.4 | 5.1 | 6.1 |
| 002894 | 105 | 9.5 | 5.1 | 6.1 |
| 002894 | 106 | 9.5 | 5.1 | 6.1 |
| 002894 | 107 | 9.4 | 5.0 | 6.1 |
| 002894 | 108 | 9.5 | 5.2 | 6.1 |
| 002894 | 109 | 9.5 | 5.2 | 6.1 |
| 002894 | 110 | 9.5 | 5.2 | 6.1 |
| 002894 | 111 | 9.5 | 5.2 | 6.1 |
| 002894 | 112 | 9.5 | 5.2 | 6.1 |
| 002894 | 113 | 9.5 | 5.3 | 6.1 |
| 002894 | 114 | 9.5 | 5.3 | 6.1 |
| 002894 | 115 | 9.5 | 5.3 | 6.1 |
| 002894 | 116 | 9.5 | 5.3 | 6.1 |
| 002894 | 117 | 9.5 | 5.2 | 6.1 |
| 002894 | 118 | 9.5 | 5.3 | 6.2 |
| 002894 | 119 | 9.5 | 5.2 | 6.2 |
| 002894 | 120 | 9.5 | 5.2 | 6.2 |
| 002894 | 121 | 9.5 | 5.2 | 6.2 |
| 002894 | 122 | 9.5 | 5.3 | 6.2 |
| 002894 | 123 | 9.5 | 5.3 | 6.2 |
| 002894 | 124 | 9.5 | 5.3 | 6.2 |
| 002894 | 125 | 9.5 | 5.3 | 6.1 |
| 002894 | 126 | 9.5 | 5.3 | 6.2 |
| 002894 | 127 | 9.5 | 5.3 | 6.1 |
| 002894 | 128 | 9.5 | 5.3 | 6.1 |
| 002894 | 129 | 9.5 | 5.3 | 6.1 |
| 002894 | 130 | 9.5 | 5.4 | 6.1 |
| 002894 | 131 | 9.5 | 5.4 | 6.1 |
| 002894 | 132 | 9.6 | 5.4 | 6.1 |
| 002894 | 133 | 9.6 | 5.4 | 6.1 |

| Bridge Recall Number | Span | Maximum Storm Water Level (ft-MSL) | Associated Significant Wave Height (ft.) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|---|-------------------------------------|
| 002894 | 134 | 9.6 | 5.4 | 6.1 |
| 002894 | 135 | 9.6 | 5.4 | 6.1 |
| 002894 | 136 | 9.5 | 5.3 | 6.1 |
| 002894 | 137 | 9.6 | 5.4 | 6.1 |
| 002894 | 138 | 9.6 | 5.5 | 6.1 |
| 002894 | 139 | 9.6 | 5.5 | 6.1 |
| 002894 | 140 | 9.6 | 5.5 | 6.1 |
| 002894 | 141 | 9.6 | 5.5 | 6.1 |
| 002894 | 142 | 9.6 | 5.5 | 6.1 |
| 002894 | 143 | 9.6 | 5.5 | 6.1 |
| 002894 | 144 | 9.6 | 5.5 | 6.1 |
| 002894 | 145 | 9.6 | 5.5 | 6.1 |
| 002894 | 146 | 9.6 | 5.5 | 6.1 |
| 002894 | 147 | 9.6 | 5.6 | 6.1 |
| 002894 | 148 | 9.6 | 5.6 | 6.1 |
| 002894 | 149 | 9.6 | 5.6 | 6.1 |
| 002894 | 150 | 9.6 | 5.6 | 6.1 |
| 002894 | 151 | 9.6 | 5.6 | 6.1 |
| 002894 | 152 | 9.6 | 5.6 | 6.1 |
| 002894 | 153 | 9.6 | 5.6 | 6.1 |
| 002894 | 154 | 9.6 | 5.6 | 6.1 |
| 002894 | 155 | 9.6 | 5.7 | 6.1 |
| 002894 | 156 | 9.6 | 5.6 | 6.1 |
| 002894 | 157 | 9.6 | 5.7 | 6.1 |
| 002894 | 158 | 9.7 | 5.7 | 6.1 |
| 002894 | 159 | 9.7 | 5.7 | 6.1 |
| 002894 | 160 | 9.7 | 5.7 | 6.1 |
| 002894 | 161 | 9.7 | 5.6 | 6.1 |
| 002894 | 162 | 9.7 | 5.6 | 6.1 |
| 002894 | 163 | 9.7 | 5.6 | 6.1 |
| 002894 | 164 | 9.7 | 5.6 | 6.1 |
| 002894 | 165 | 9.7 | 5.6 | 6.1 |
| 002894 | 166 | 9.6 | 5.7 | 6.1 |
| 002894 | 167 | 9.7 | 5.6 | 6.1 |
| 002894 | 168 | 9.7 | 5.6 | 6.1 |

| Bridge Recall Number | Span | Maximum Storm Water Level (ft-MSL) | Associated Significant Wave Height (ft.) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|---|-------------------------------------|
| 002894 | 169 | 9.7 | 5.6 | 6.1 |
| 002894 | 170 | 9.7 | 5.6 | 6.1 |
| 002894 | 171 | 9.7 | 5.6 | 6.1 |
| 002894 | 172 | 9.7 | 5.6 | 6.1 |
| 002894 | 173 | 9.7 | 5.6 | 6.1 |
| 002894 | 174 | 9.7 | 5.6 | 6.1 |
| 002894 | 175 | 9.7 | 5.6 | 6.1 |
| 002894 | 176 | 9.7 | 5.6 | 6.1 |
| 002894 | 177 | 9.7 | 5.6 | 6.1 |
| 002894 | 178 | 9.7 | 5.6 | 6.1 |
| 002894 | 179 | 9.7 | 5.6 | 6.1 |
| 002894 | 180 | 9.7 | 5.6 | 6.1 |
| 002894 | 181 | 9.7 | 5.6 | 6.1 |
| 002894 | 182 | 9.8 | 5.6 | 6.1 |
| 002894 | 183 | 9.8 | 5.6 | 6.1 |
| 002894 | 184 | 9.8 | 5.6 | 6.1 |
| 002894 | 185 | 9.8 | 5.6 | 6.1 |
| 002894 | 186 | 9.8 | 5.6 | 6.1 |
| 002894 | 187 | 9.8 | 5.6 | 6.1 |
| 002894 | 188 | 9.8 | 5.6 | 6.1 |
| 002894 | 189 | 9.8 | 5.6 | 6.0 |
| 002894 | 190 | 9.8 | 5.6 | 6.0 |
| 002894 | 191 | 9.8 | 5.6 | 6.0 |
| 002894 | 192 | 9.8 | 5.6 | 6.0 |
| 002894 | 193 | 9.8 | 5.6 | 6.0 |
| 002894 | 194 | 9.8 | 5.6 | 6.0 |
| 002894 | 195 | 9.8 | 5.6 | 6.0 |
| 002894 | 196 | 9.8 | 5.5 | 6.0 |
| 002894 | 197 | 9.8 | 5.5 | 6.0 |
| 002894 | 198 | 9.8 | 5.5 | 6.0 |
| 002894 | 199 | 9.8 | 5.5 | 6.0 |
| 002894 | 200 | 9.8 | 5.5 | 6.0 |
| 002894 | 201 | 9.8 | 5.5 | 6.0 |
| 002894 | 202 | 9.8 | 5.4 | 6.0 |
| 002894 | 203 | 9.8 | 5.4 | 6.0 |

| Bridge Recall Number | Span | Maximum Storm Water Level (ft-MSL) | Associated Significant Wave Height (ft.) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|---|-------------------------------------|
| 002894 | 204 | 9.8 | 5.5 | 6.0 |
| 002894 | 205 | 9.9 | 5.4 | 6.0 |
| 002894 | 206 | 9.9 | 5.4 | 6.1 |
| 002894 | 207 | 9.9 | 5.3 | 6.1 |
| 002894 | 208 | 9.9 | 5.3 | 6.1 |
| 002894 | 209 | 9.9 | 5.3 | 6.1 |
| 002894 | 210 | 9.9 | 5.3 | 6.1 |
| 002894 | 211 | 9.9 | 5.3 | 6.1 |
| 002894 | 212 | 9.9 | 5.3 | 6.1 |
| 002894 | 213 | 9.9 | 5.3 | 6.1 |
| 002894 | 214 | 9.9 | 5.3 | 6.1 |
| 002894 | 215 | 9.9 | 5.3 | 6.1 |
| 002894 | 216 | 9.9 | 5.3 | 6.0 |
| 002894 | 217 | 9.9 | 5.3 | 6.1 |
| 002894 | 218 | 9.9 | 5.3 | 6.1 |
| 002894 | 219 | 9.9 | 5.2 | 6.1 |
| 002894 | 220 | 9.9 | 5.2 | 6.1 |
| 002894 | 221 | 9.9 | 5.2 | 6.0 |
| 002894 | 222 | 9.9 | 5.2 | 6.0 |
| 002894 | 223 | 9.9 | 5.2 | 6.0 |
| 002894 | 224 | 10.0 | 5.2 | 6.0 |
| 002894 | 225 | 10.0 | 5.2 | 6.0 |
| 002894 | 226 | 10.0 | 5.2 | 6.0 |
| 002894 | 227 | 10.0 | 5.1 | 6.0 |
| 002894 | 228 | 10.0 | 5.0 | 6.0 |
| 002894 | 229 | 10.0 | 5.0 | 6.0 |
| 002894 | 230 | 10.0 | 4.9 | 6.0 |
| 002894 | 231 | 10.0 | 4.9 | 6.0 |
| 002894 | 232 | 10.0 | 4.8 | 6.1 |
| 002894 | 233 | 10.0 | 4.8 | 6.1 |
| 002894 | 234 | 10.0 | 4.7 | 6.1 |
| 002894 | 235 | 10.0 | 4.7 | 6.1 |
| 002894 | 236 | 10.1 | 4.7 | 6.1 |
| 002894 | 237 | 10.1 | 4.6 | 6.1 |
| 002894 | 238 | 10.1 | 4.6 | 6.1 |

| Bridge Recall Number | Span | Maximum Storm Water Level (ft-MSL) | Associated Significant Wave Height (ft.) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|---|-------------------------------------|
| 002894 | 239 | 10.1 | 4.6 | 6.1 |
| 002894 | 240 | 10.1 | 4.6 | 6.1 |
| 002894 | 241 | 10.1 | 4.6 | 6.1 |
| 002894 | 242 | 10.1 | 4.5 | 6.1 |
| 002894 | 243 | 10.1 | 4.5 | 6.1 |
| 002894 | 244 | 10.1 | 4.5 | 6.1 |
| 002894 | 245 | 10.1 | 4.5 | 6.1 |
| 002894 | 246 | 10.0 | 5.1 | 6.0 |
| 002894 | 247 | 10.1 | 4.4 | 6.1 |
| 003432 | 1 | 9.5 | 6.5 | 4.7 |
| 003432 | 2 | 9.5 | 6.5 | 4.7 |
| 003432 | 3 | 9.5 | 6.5 | 4.7 |
| 003432 | 4 | 9.5 | 6.5 | 4.7 |
| 003432 | 5 | 9.5 | 6.5 | 4.7 |
| 003432 | 6 | 9.5 | 6.6 | 4.7 |
| 003432 | 7 | 9.5 | 6.6 | 4.7 |
| 003432 | 8 | 9.5 | 6.6 | 4.7 |
| 003432 | 9 | 9.5 | 6.6 | 4.7 |
| 003432 | 10 | 9.5 | 6.6 | 4.7 |
| 003432 | 11 | 9.5 | 6.6 | 4.7 |
| 003432 | 12 | 9.5 | 6.6 | 4.7 |
| 003432 | 13 | 9.5 | 6.6 | 4.7 |
| 003432 | 14 | 9.5 | 6.6 | 4.7 |
| 003432 | 15 | 9.5 | 6.6 | 4.7 |
| 003432 | 16 | 9.5 | 6.6 | 4.7 |
| 003432 | 17 | 9.5 | 6.6 | 4.7 |
| 003440 | 1 | 8.1 | 5.3 | 4.5 |
| 003440 | 2 | 8.1 | 5.3 | 4.5 |
| 003440 | 3 | 8.1 | 5.3 | 4.6 |
| 003440 | 4 | 8.1 | 5.3 | 4.6 |
| 003440 | 5 | 8.1 | 5.3 | 4.6 |
| 003440 | 6 | 8.1 | 5.3 | 4.6 |
| 003440 | 7 | 8.1 | 5.3 | 4.6 |
| 003440 | 8 | 8.1 | 5.2 | 4.6 |
| 003440 | 9 | 8.1 | 5.2 | 4.5 |

| Bridge Recall Number | Span | Maximum Storm Water Level (ft-MSL) | Associated Significant Wave Height (ft.) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|---|-------------------------------------|
| 003450 | 1 | 9.3 | 3.8 | 3.7 |
| 003450 | 2 | 9.3 | 3.8 | 3.7 |
| 003450 | 3 | 9.3 | 3.8 | 3.7 |
| 003450 | 4 | 9.3 | 3.8 | 3.7 |
| 003450 | 5 | 9.3 | 3.8 | 3.8 |
| 003450 | 6 | 9.3 | 3.8 | 3.8 |
| 003480 | 1 | 8.1 | 1.3 | 2.0 |
| 003480 | 2 | 8.1 | 1.3 | 2.0 |
| 003480 | 3 | 8.1 | 1.4 | 2.0 |
| 003480 | 4 | 8.1 | 1.4 | 2.0 |
| 003480 | 5 | 8.1 | 1.4 | 2.0 |
| 003510 | 1 | 8.9 | 4.5 | 4.4 |
| 003510 | 2 | 8.9 | 4.6 | 4.4 |
| 003510 | 3 | 8.9 | 4.6 | 4.4 |
| 003510 | 4 | 8.9 | 4.6 | 4.4 |
| 003510 | 5 | 8.9 | 4.6 | 4.4 |
| 003510 | 6 | 8.9 | 4.6 | 4.4 |
| 003510 | 7 | 8.9 | 4.6 | 4.4 |
| 003510 | 8 | 8.9 | 4.7 | 4.4 |
| 003510 | 9 | 8.9 | 4.6 | 4.4 |
| 003520 | 1 | 9.2 | 4.3 | 4.5 |
| 003520 | 2 | 9.2 | 4.3 | 4.5 |
| 003520 | 3 | 9.2 | 4.3 | 4.5 |
| 003690 | 1 | 9.5 | 6.5 | 4.7 |
| 003690 | 2 | 9.5 | 6.5 | 4.7 |
| 003690 | 3 | 9.5 | 6.5 | 4.7 |
| 003690 | 4 | 9.5 | 6.5 | 4.7 |
| 003690 | 5 | 9.5 | 6.5 | 4.7 |
| 009030 | 1 | 6.6 | 1.6 | 4.4 |
| 009030 | 2 | 6.6 | 1.6 | 4.4 |
| 009030 | 3 | 6.6 | 1.6 | 4.4 |
| 009030 | 4 | 6.6 | 1.6 | 4.4 |
| 009030 | 5 | 6.6 | 1.6 | 4.4 |
| 009030 | 6 | 6.6 | 1.6 | 4.4 |
| 009030 | 7 | 6.6 | 1.6 | 4.2 |

| Bridge Recall Number | Span | Maximum Storm Water Level (ft-MSL) | Associated Significant Wave Height (ft.) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|---|-------------------------------------|
| 009030 | 8 | 6.6 | 1.6 | 4.1 |
| 031755 | 1 | 5.8 | 1.4 | 4.1 |
| 031755 | 2 | 5.8 | 1.4 | 4.8 |
| 031755 | 3 | 5.8 | 1.4 | 5.5 |
| 031755 | 4 | 5.8 | 1.5 | 6.4 |
| 031755 | 5 | 5.8 | 1.5 | 7.4 |
| 031755 | 6 | 5.8 | 1.5 | 8.5 |
| 031755 | 7 | 5.8 | 1.5 | 9.2 |
| 031755 | 8 | 5.8 | 1.6 | 9.4 |
| 031755 | 9 | 5.8 | 1.6 | 9.5 |
| 031755 | 10 | 5.8 | 1.7 | 9.4 |
| 031755 | 11 | 5.8 | 1.7 | 9.4 |
| 031755 | 12 | 5.8 | 1.7 | 9.4 |
| 031755 | 13 | 5.8 | 1.8 | 9.4 |
| 031755 | 14 | 5.8 | 1.8 | 9.4 |
| 031755 | 15 | 5.8 | 1.8 | 9.4 |
| 031755 | 16 | 5.8 | 1.8 | 9.4 |
| 031755 | 17 | 5.8 | 1.8 | 9.4 |
| 031755 | 18 | 5.8 | 1.9 | 9.4 |
| 031755 | 19 | 5.8 | 1.9 | 9.4 |
| 031755 | 20 | 5.8 | 1.9 | 9.4 |
| 031755 | 21 | 5.8 | 1.9 | 9.4 |
| 031755 | 22 | 5.8 | 1.9 | 9.4 |
| 031755 | 23 | 5.8 | 1.9 | 9.3 |
| 031755 | 24 | 5.8 | 1.9 | 8.7 |
| 031755 | 25 | 5.8 | 1.8 | 8.4 |
| 031755 | 26 | 5.8 | 1.7 | 8.1 |
| 031755 | 27 | 5.8 | 1.6 | 7.8 |
| 033698 | 1 | 8.3 | 2.2 | 2.7 |
| 033698 | 2 | 8.3 | 2.3 | 2.7 |
| 033700 | 1 | 9.7 | 4.1 | 3.8 |
| 033700 | 2 | 9.7 | 4.1 | 3.8 |
| 033700 | 3 | 9.7 | 4.1 | 3.8 |
| 033700 | 4 | 9.7 | 4.0 | 3.8 |
| 033700 | 5 | 9.7 | 4.0 | 3.8 |

| Bridge Recall Number | Span | Maximum Storm Water Level (ft-MSL) | Associated Significant Wave Height (ft.) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|---|-------------------------------------|
| 033700 | 6 | 9.7 | 4.0 | 3.8 |
| 033700 | 7 | 9.7 | 3.9 | 3.8 |
| 033700 | 8 | 9.6 | 3.9 | 3.8 |
| 033700 | 9 | 9.6 | 3.8 | 3.8 |
| 033700 | 10 | 9.6 | 3.8 | 3.8 |
| 033700 | 11 | 9.6 | 3.7 | 3.8 |
| 033700 | 12 | 9.6 | 3.6 | 3.8 |
| 033700 | 13 | 9.6 | 3.5 | 3.8 |
| 033700 | 14 | 9.6 | 3.5 | 3.7 |
| 033700 | 15 | 9.6 | 3.6 | 3.7 |
| 033700 | 16 | 9.6 | 3.6 | 3.6 |
| 033700 | 17 | 9.6 | 3.6 | 3.6 |
| 033700 | 18 | 9.6 | 3.6 | 3.6 |
| 033700 | 19 | 9.6 | 3.6 | 3.5 |
| 033700 | 20 | 9.6 | 3.6 | 3.5 |
| 033700 | 21 | 9.6 | 3.6 | 3.5 |
| 033700 | 22 | 9.6 | 3.6 | 3.5 |
| 033700 | 23 | 9.6 | 3.6 | 3.5 |
| 033700 | 24 | 9.6 | 3.6 | 3.5 |
| 033700 | 25 | 9.6 | 3.6 | 3.5 |
| 033700 | 26 | 9.6 | 3.6 | 3.5 |
| 033700 | 27 | 9.6 | 3.6 | 3.5 |
| 033700 | 28 | 9.6 | 3.6 | 3.5 |
| 033700 | 29 | 9.6 | 3.6 | 3.5 |
| 033700 | 30 | 9.6 | 3.6 | 3.5 |
| 033700 | 31 | 9.6 | 3.6 | 3.5 |
| 033700 | 32 | 9.6 | 3.6 | 3.5 |
| 033700 | 33 | 9.6 | 3.5 | 3.5 |
| 033700 | 34 | 9.6 | 3.5 | 3.5 |
| 033700 | 35 | 9.6 | 3.5 | 3.5 |
| 059482 | 1 | 6.7 | 1.4 | 2.1 |
| 059482 | 2 | 6.7 | 1.4 | 2.1 |
| 059482 | 3 | 6.7 | 1.4 | 2.2 |
| 059482 | 4 | 6.7 | 1.3 | 2.3 |
| 059482 | 5 | 6.7 | 1.1 | 2.2 |

| Bridge Recall Number | Span | Maximum Storm Water Level (ft-MSL) | Associated Significant Wave Height (ft.) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|---|-------------------------------------|
| 059482 | 6 | 6.7 | 1.0 | 2.1 |
| 060360 | 1 | 6.5 | 2.2 | 4.8 |
| 060360 | 2 | 6.5 | 2.2 | 4.8 |
| 060360 | 3 | 6.5 | 2.2 | 4.8 |
| 060360 | 4 | 6.5 | 2.2 | 4.8 |
| 060360 | 5 | 6.5 | 2.2 | 4.8 |

Table B.2
Maximum Significant Wave Height and Associated Storm Water Level

| Bridge Recall Number | Span | Peak Significant Wave Height (ft.) | Associated Storm Water Level (ft-MSL) | Associated Peak Period (sec) |
|----------------------|------|------------------------------------|---------------------------------------|------------------------------|
| 002631 | 1 | 2.9 | 8.0 | 3.9 |
| 002631 | 2 | 2.9 | 8.0 | 3.9 |
| 002631 | 3 | 2.9 | 8.0 | 3.9 |
| 002632 | 1 | 2.9 | 8.0 | 3.9 |
| 002632 | 2 | 2.9 | 8.0 | 3.9 |
| 002632 | 3 | 2.9 | 8.0 | 3.9 |
| 002650 | 1 | 3.1 | 7.8 | 3.6 |
| 002650 | 2 | 3.1 | 7.8 | 3.6 |
| 002650 | 3 | 3.1 | 7.8 | 3.6 |
| 002650 | 4 | 3.1 | 7.8 | 3.6 |
| 002650 | 5 | 3.1 | 7.8 | 3.6 |
| 002892 | 5 | 3.8 | 9.1 | 6.0 |
| 002892 | 6 | 3.8 | 9.1 | 6.0 |
| 002892 | 7 | 3.8 | 9.1 | 6.0 |
| 002892 | 8 | 3.8 | 9.1 | 6.0 |
| 002892 | 9 | 3.8 | 9.1 | 6.0 |
| 002892 | 10 | 3.9 | 9.1 | 6.0 |
| 002892 | 11 | 3.9 | 9.1 | 6.0 |
| 002892 | 12 | 3.9 | 9.1 | 6.0 |
| 002892 | 13 | 3.8 | 9.0 | 6.1 |
| 002892 | 14 | 3.9 | 9.1 | 6.0 |
| 002892 | 15 | 3.9 | 9.1 | 6.1 |
| 002892 | 16 | 3.9 | 9.1 | 6.1 |
| 002892 | 17 | 4.0 | 9.1 | 6.1 |
| 002892 | 18 | 4.0 | 9.1 | 6.1 |
| 002892 | 19 | 4.0 | 9.1 | 6.0 |
| 002892 | 20 | 4.0 | 9.1 | 6.0 |
| 002892 | 21 | 4.0 | 9.1 | 6.1 |
| 002892 | 22 | 4.0 | 9.1 | 6.0 |
| 002892 | 23 | 4.0 | 9.2 | 6.1 |
| 002892 | 24 | 4.1 | 9.2 | 6.1 |
| 002892 | 25 | 4.1 | 9.2 | 6.1 |

| Bridge Recall Number | Span | Peak Significant Wave Height (ft.) | Associated Storm Water Level (ft-MSL) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|--|-------------------------------------|
| 002892 | 26 | 4.1 | 9.2 | 6.1 |
| 002892 | 27 | 4.1 | 9.2 | 6.1 |
| 002892 | 28 | 4.1 | 9.2 | 6.1 |
| 002892 | 29 | 4.0 | 9.1 | 6.1 |
| 002892 | 30 | 4.1 | 9.2 | 6.1 |
| 002892 | 31 | 4.1 | 9.2 | 6.1 |
| 002892 | 32 | 4.1 | 9.2 | 6.1 |
| 002892 | 33 | 4.1 | 9.2 | 6.1 |
| 002892 | 34 | 4.2 | 9.2 | 6.1 |
| 002892 | 35 | 4.1 | 9.2 | 6.1 |
| 002892 | 36 | 4.2 | 9.2 | 6.1 |
| 002892 | 37 | 4.2 | 9.2 | 6.1 |
| 002892 | 38 | 4.3 | 9.2 | 6.1 |
| 002892 | 39 | 4.3 | 9.2 | 6.1 |
| 002892 | 40 | 4.3 | 9.2 | 6.1 |
| 002892 | 41 | 4.3 | 9.2 | 6.1 |
| 002892 | 42 | 4.3 | 9.2 | 6.2 |
| 002892 | 43 | 4.3 | 9.2 | 6.2 |
| 002892 | 44 | 4.3 | 9.2 | 6.2 |
| 002892 | 45 | 4.2 | 9.2 | 6.1 |
| 002892 | 46 | 4.4 | 9.3 | 6.2 |
| 002892 | 47 | 4.4 | 9.3 | 6.1 |
| 002892 | 48 | 4.4 | 9.3 | 6.1 |
| 002892 | 49 | 4.4 | 9.3 | 6.1 |
| 002892 | 50 | 4.4 | 9.3 | 6.1 |
| 002892 | 51 | 4.4 | 9.3 | 6.1 |
| 002892 | 52 | 4.5 | 9.3 | 6.1 |
| 002892 | 53 | 4.5 | 9.3 | 6.1 |
| 002892 | 54 | 4.5 | 9.3 | 6.1 |
| 002892 | 55 | 4.4 | 9.3 | 6.1 |
| 002892 | 56 | 4.5 | 9.3 | 6.1 |
| 002892 | 57 | 4.6 | 9.3 | 6.1 |
| 002892 | 58 | 4.6 | 9.3 | 6.1 |
| 002892 | 59 | 4.6 | 9.3 | 6.1 |
| 002892 | 60 | 4.6 | 9.3 | 6.1 |

| Bridge Recall Number | Span | Peak Significant Wave Height (ft.) | Associated Storm Water Level (ft-MSL) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|--|-------------------------------------|
| 002892 | 61 | 4.7 | 9.3 | 6.1 |
| 002892 | 62 | 4.7 | 9.3 | 6.1 |
| 002892 | 63 | 4.7 | 9.3 | 6.1 |
| 002892 | 64 | 4.7 | 9.3 | 6.1 |
| 002892 | 65 | 4.6 | 9.3 | 6.1 |
| 002892 | 66 | 4.7 | 9.3 | 6.1 |
| 002892 | 67 | 4.7 | 9.3 | 6.1 |
| 002892 | 68 | 4.8 | 9.4 | 6.1 |
| 002892 | 69 | 4.8 | 9.4 | 6.1 |
| 002892 | 70 | 4.8 | 9.4 | 6.1 |
| 002892 | 71 | 4.8 | 9.4 | 6.1 |
| 002892 | 72 | 4.9 | 9.4 | 6.1 |
| 002892 | 73 | 4.9 | 9.4 | 6.0 |
| 002892 | 74 | 4.9 | 9.4 | 6.0 |
| 002892 | 75 | 4.7 | 9.3 | 6.1 |
| 002892 | 76 | 4.9 | 9.4 | 6.0 |
| 002892 | 77 | 4.9 | 9.4 | 6.0 |
| 002892 | 78 | 4.9 | 9.4 | 6.0 |
| 002892 | 79 | 4.9 | 9.4 | 6.0 |
| 002892 | 80 | 4.9 | 9.4 | 6.0 |
| 002892 | 81 | 4.9 | 9.4 | 6.0 |
| 002892 | 82 | 4.9 | 9.4 | 6.0 |
| 002892 | 83 | 4.9 | 9.4 | 6.0 |
| 002892 | 84 | 4.9 | 9.4 | 6.0 |
| 002892 | 85 | 4.9 | 9.4 | 6.1 |
| 002892 | 86 | 4.9 | 9.4 | 6.0 |
| 002892 | 87 | 4.9 | 9.4 | 6.0 |
| 002892 | 88 | 4.9 | 9.4 | 6.0 |
| 002892 | 89 | 4.9 | 9.4 | 6.0 |
| 002892 | 90 | 4.9 | 9.4 | 6.0 |
| 002892 | 91 | 4.9 | 9.4 | 6.0 |
| 002892 | 92 | 4.9 | 9.4 | 6.0 |
| 002892 | 93 | 4.9 | 9.4 | 6.0 |
| 002892 | 94 | 4.9 | 9.4 | 6.0 |
| 002892 | 95 | 4.9 | 9.4 | 6.0 |

| Bridge Recall Number | Span | Peak Significant Wave Height (ft.) | Associated Storm Water Level (ft-MSL) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|--|-------------------------------------|
| 002892 | 96 | 4.9 | 9.4 | 6.0 |
| 002892 | 97 | 5.0 | 9.4 | 6.0 |
| 002892 | 98 | 5.0 | 9.4 | 6.0 |
| 002892 | 99 | 5.0 | 9.4 | 6.0 |
| 002892 | 100 | 5.0 | 9.5 | 6.0 |
| 002892 | 101 | 5.0 | 9.5 | 6.0 |
| 002892 | 102 | 5.0 | 9.5 | 6.0 |
| 002892 | 103 | 5.1 | 9.5 | 6.0 |
| 002892 | 104 | 5.1 | 9.5 | 6.0 |
| 002892 | 105 | 5.0 | 9.4 | 6.0 |
| 002892 | 106 | 5.1 | 9.5 | 6.0 |
| 002892 | 107 | 5.1 | 9.5 | 6.0 |
| 002892 | 108 | 5.1 | 9.5 | 6.0 |
| 002892 | 109 | 5.1 | 9.5 | 6.0 |
| 002892 | 110 | 5.2 | 9.5 | 6.0 |
| 002892 | 111 | 5.2 | 9.5 | 6.0 |
| 002892 | 112 | 5.2 | 9.5 | 6.0 |
| 002892 | 113 | 5.2 | 9.5 | 6.0 |
| 002892 | 114 | 5.2 | 9.5 | 6.1 |
| 002892 | 115 | 5.1 | 9.5 | 6.0 |
| 002892 | 116 | 5.2 | 9.5 | 6.1 |
| 002892 | 117 | 5.1 | 9.5 | 6.1 |
| 002892 | 118 | 5.1 | 9.5 | 6.1 |
| 002892 | 119 | 5.1 | 9.5 | 6.1 |
| 002892 | 120 | 5.1 | 9.5 | 6.1 |
| 002892 | 121 | 5.1 | 9.5 | 6.1 |
| 002892 | 122 | 5.1 | 9.5 | 6.1 |
| 002892 | 123 | 5.1 | 9.6 | 6.1 |
| 002892 | 124 | 5.1 | 9.5 | 6.1 |
| 002892 | 125 | 5.1 | 9.6 | 6.1 |
| 002892 | 126 | 5.1 | 9.6 | 6.1 |
| 002892 | 127 | 5.2 | 9.6 | 6.1 |
| 002892 | 128 | 5.2 | 9.6 | 6.1 |
| 002892 | 129 | 5.2 | 9.6 | 6.1 |
| 002892 | 130 | 5.2 | 9.6 | 6.1 |

| Bridge Recall Number | Span | Peak Significant Wave Height (ft.) | Associated Storm Water Level (ft-MSL) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|--|-------------------------------------|
| 002892 | 131 | 5.2 | 9.6 | 6.1 |
| 002892 | 132 | 5.2 | 9.6 | 6.1 |
| 002892 | 133 | 5.3 | 9.6 | 6.1 |
| 002892 | 134 | 5.1 | 9.6 | 6.1 |
| 002892 | 135 | 5.3 | 9.6 | 6.0 |
| 002892 | 136 | 5.4 | 9.6 | 6.0 |
| 002892 | 137 | 5.5 | 9.6 | 6.0 |
| 002892 | 138 | 5.5 | 9.6 | 6.0 |
| 002892 | 139 | 5.5 | 9.6 | 6.0 |
| 002892 | 140 | 5.5 | 9.6 | 6.0 |
| 002892 | 141 | 5.5 | 9.6 | 6.0 |
| 002892 | 142 | 5.5 | 9.6 | 6.0 |
| 002892 | 143 | 5.6 | 9.6 | 6.0 |
| 002892 | 144 | 5.4 | 9.6 | 6.0 |
| 002892 | 145 | 5.7 | 9.6 | 6.0 |
| 002892 | 146 | 5.7 | 9.6 | 6.0 |
| 002892 | 147 | 5.7 | 9.6 | 6.0 |
| 002892 | 148 | 5.7 | 9.6 | 6.0 |
| 002892 | 149 | 5.7 | 9.6 | 6.0 |
| 002892 | 150 | 5.7 | 9.6 | 6.0 |
| 002892 | 151 | 5.7 | 9.6 | 6.0 |
| 002892 | 152 | 5.7 | 9.6 | 6.0 |
| 002892 | 153 | 5.7 | 9.7 | 6.0 |
| 002892 | 154 | 5.7 | 9.6 | 6.0 |
| 002892 | 155 | 5.7 | 9.7 | 6.0 |
| 002892 | 156 | 5.7 | 9.7 | 6.0 |
| 002892 | 157 | 5.7 | 9.7 | 6.0 |
| 002892 | 158 | 5.7 | 9.7 | 6.0 |
| 002892 | 159 | 5.6 | 9.7 | 6.1 |
| 002892 | 160 | 5.5 | 9.7 | 6.0 |
| 002892 | 161 | 5.5 | 9.7 | 6.0 |
| 002892 | 162 | 5.5 | 9.7 | 6.0 |
| 002892 | 163 | 5.5 | 9.7 | 6.1 |
| 002892 | 164 | 5.7 | 9.7 | 6.0 |
| 002892 | 165 | 5.5 | 9.7 | 6.0 |

| Bridge Recall Number | Span | Peak Significant Wave Height (ft.) | Associated Storm Water Level (ft-MSL) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|--|-------------------------------------|
| 002892 | 166 | 5.6 | 9.7 | 6.0 |
| 002892 | 167 | 5.6 | 9.7 | 6.0 |
| 002892 | 168 | 5.6 | 9.7 | 6.0 |
| 002892 | 169 | 5.6 | 9.7 | 6.0 |
| 002892 | 170 | 5.7 | 9.7 | 6.0 |
| 002892 | 171 | 5.7 | 9.7 | 6.0 |
| 002892 | 172 | 5.7 | 9.7 | 6.0 |
| 002892 | 173 | 5.7 | 9.7 | 6.0 |
| 002892 | 174 | 5.6 | 9.7 | 6.0 |
| 002892 | 175 | 5.7 | 9.7 | 6.0 |
| 002892 | 176 | 5.7 | 9.7 | 6.0 |
| 002892 | 177 | 5.7 | 9.7 | 6.0 |
| 002892 | 178 | 5.6 | 9.8 | 6.0 |
| 002892 | 179 | 5.6 | 9.8 | 6.0 |
| 002892 | 180 | 5.6 | 9.8 | 6.0 |
| 002892 | 181 | 5.6 | 9.8 | 6.0 |
| 002892 | 182 | 5.6 | 9.8 | 6.0 |
| 002892 | 183 | 5.6 | 9.8 | 6.0 |
| 002892 | 184 | 5.7 | 9.7 | 6.0 |
| 002892 | 185 | 5.6 | 9.8 | 6.0 |
| 002892 | 186 | 5.6 | 9.8 | 6.0 |
| 002892 | 187 | 5.6 | 9.8 | 6.0 |
| 002892 | 188 | 5.6 | 9.8 | 6.0 |
| 002892 | 189 | 5.6 | 9.8 | 6.0 |
| 002892 | 190 | 5.6 | 9.8 | 6.0 |
| 002892 | 191 | 5.6 | 9.8 | 6.0 |
| 002892 | 192 | 5.6 | 9.8 | 5.9 |
| 002892 | 193 | 5.6 | 9.8 | 5.9 |
| 002892 | 194 | 5.6 | 9.8 | 6.0 |
| 002892 | 195 | 5.6 | 9.8 | 5.9 |
| 002892 | 196 | 5.6 | 9.8 | 6.0 |
| 002892 | 197 | 5.5 | 9.8 | 6.0 |
| 002892 | 198 | 5.5 | 9.8 | 6.0 |
| 002892 | 199 | 5.5 | 9.8 | 6.0 |
| 002892 | 200 | 5.5 | 9.8 | 6.0 |

| Bridge Recall Number | Span | Peak Significant Wave Height (ft.) | Associated Storm Water Level (ft-MSL) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|--|-------------------------------------|
| 002892 | 201 | 5.4 | 9.9 | 6.0 |
| 002892 | 202 | 5.4 | 9.9 | 6.0 |
| 002892 | 203 | 5.4 | 9.9 | 6.0 |
| 002892 | 204 | 5.3 | 9.9 | 6.0 |
| 002892 | 205 | 5.3 | 9.9 | 6.0 |
| 002892 | 206 | 5.3 | 9.9 | 6.1 |
| 002892 | 207 | 5.3 | 9.9 | 6.0 |
| 002892 | 208 | 5.3 | 9.9 | 6.0 |
| 002892 | 209 | 5.3 | 9.9 | 6.0 |
| 002892 | 210 | 5.3 | 9.9 | 6.0 |
| 002892 | 211 | 5.3 | 9.9 | 6.0 |
| 002892 | 212 | 5.3 | 9.9 | 6.0 |
| 002892 | 213 | 5.3 | 9.9 | 6.0 |
| 002892 | 214 | 5.3 | 9.9 | 6.0 |
| 002892 | 215 | 5.3 | 9.9 | 6.0 |
| 002892 | 216 | 5.3 | 9.9 | 6.0 |
| 002892 | 217 | 5.3 | 9.9 | 6.0 |
| 002892 | 218 | 5.5 | 9.8 | 6.0 |
| 002892 | 219 | 5.3 | 9.9 | 6.0 |
| 002892 | 220 | 5.3 | 9.9 | 6.0 |
| 002892 | 221 | 5.2 | 10.0 | 6.0 |
| 002892 | 222 | 5.2 | 10.0 | 6.0 |
| 002892 | 223 | 5.2 | 10.0 | 6.0 |
| 002892 | 224 | 5.1 | 10.0 | 6.0 |
| 002892 | 225 | 5.1 | 10.0 | 6.0 |
| 002892 | 226 | 5.1 | 10.0 | 6.0 |
| 002892 | 227 | 5.0 | 10.0 | 6.0 |
| 002892 | 228 | 5.0 | 10.0 | 6.0 |
| 002892 | 229 | 5.0 | 10.0 | 6.0 |
| 002892 | 230 | 4.9 | 10.0 | 6.1 |
| 002892 | 231 | 4.9 | 10.0 | 6.1 |
| 002892 | 232 | 4.8 | 10.0 | 6.1 |
| 002892 | 233 | 4.8 | 10.0 | 6.1 |
| 002892 | 234 | 4.7 | 10.1 | 6.1 |
| 002892 | 235 | 4.7 | 10.1 | 6.1 |

| Bridge Recall Number | Span | Peak Significant Wave Height (ft.) | Associated Storm Water Level (ft-MSL) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|--|-------------------------------------|
| 002892 | 236 | 4.7 | 10.1 | 6.1 |
| 002892 | 237 | 4.7 | 10.1 | 6.1 |
| 002892 | 238 | 4.7 | 10.1 | 6.1 |
| 002892 | 239 | 4.6 | 10.1 | 6.1 |
| 002892 | 240 | 4.6 | 10.1 | 6.1 |
| 002892 | 241 | 5.2 | 10.0 | 6.0 |
| 002892 | 242 | 4.6 | 10.1 | 6.1 |
| 002894 | 5 | 5.8 | 9.7 | 6.0 |
| 002894 | 6 | 5.8 | 9.7 | 6.0 |
| 002894 | 7 | 3.9 | 9.0 | 6.1 |
| 002894 | 8 | 3.9 | 9.0 | 6.1 |
| 002894 | 9 | 3.9 | 9.0 | 6.1 |
| 002894 | 10 | 3.9 | 9.1 | 6.1 |
| 002894 | 11 | 3.9 | 9.1 | 6.1 |
| 002894 | 12 | 3.9 | 9.1 | 6.1 |
| 002894 | 13 | 3.9 | 9.1 | 6.1 |
| 002894 | 14 | 3.9 | 9.1 | 6.1 |
| 002894 | 15 | 3.9 | 9.0 | 6.1 |
| 002894 | 16 | 4.0 | 9.1 | 6.1 |
| 002894 | 17 | 4.0 | 9.1 | 6.1 |
| 002894 | 18 | 4.0 | 9.1 | 6.1 |
| 002894 | 19 | 4.0 | 9.1 | 6.1 |
| 002894 | 20 | 4.1 | 9.1 | 6.1 |
| 002894 | 21 | 4.1 | 9.1 | 6.1 |
| 002894 | 22 | 4.1 | 9.1 | 6.1 |
| 002894 | 23 | 4.1 | 9.1 | 6.1 |
| 002894 | 24 | 4.0 | 9.1 | 6.1 |
| 002894 | 25 | 4.1 | 9.1 | 6.1 |
| 002894 | 26 | 4.1 | 9.1 | 6.1 |
| 002894 | 27 | 4.1 | 9.1 | 6.1 |
| 002894 | 28 | 4.1 | 9.1 | 6.1 |
| 002894 | 29 | 4.1 | 9.2 | 6.1 |
| 002894 | 30 | 4.1 | 9.2 | 6.1 |
| 002894 | 31 | 4.2 | 9.2 | 6.1 |
| 002894 | 32 | 4.2 | 9.2 | 6.1 |

| Bridge Recall Number | Span | Peak Significant Wave Height (ft.) | Associated Storm Water Level (ft-MSL) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|--|-------------------------------------|
| 002894 | 33 | 4.2 | 9.2 | 6.1 |
| 002894 | 34 | 4.2 | 9.2 | 6.1 |
| 002894 | 35 | 4.2 | 9.2 | 6.1 |
| 002894 | 36 | 4.3 | 9.2 | 6.1 |
| 002894 | 37 | 4.2 | 9.2 | 6.1 |
| 002894 | 38 | 4.3 | 9.2 | 6.1 |
| 002894 | 39 | 4.4 | 9.2 | 6.1 |
| 002894 | 40 | 4.4 | 9.2 | 6.1 |
| 002894 | 41 | 4.4 | 9.2 | 6.1 |
| 002894 | 42 | 4.4 | 9.2 | 6.1 |
| 002894 | 43 | 4.4 | 9.2 | 6.1 |
| 002894 | 44 | 4.5 | 9.2 | 6.1 |
| 002894 | 45 | 4.5 | 9.2 | 6.1 |
| 002894 | 46 | 4.5 | 9.2 | 6.1 |
| 002894 | 47 | 4.3 | 9.2 | 6.1 |
| 002894 | 48 | 4.5 | 9.2 | 6.1 |
| 002894 | 49 | 4.6 | 9.2 | 6.1 |
| 002894 | 50 | 4.6 | 9.2 | 6.1 |
| 002894 | 51 | 4.6 | 9.3 | 6.1 |
| 002894 | 52 | 4.7 | 9.3 | 6.1 |
| 002894 | 53 | 4.7 | 9.3 | 6.1 |
| 002894 | 54 | 4.7 | 9.3 | 6.0 |
| 002894 | 55 | 4.7 | 9.3 | 6.1 |
| 002894 | 56 | 4.7 | 9.3 | 6.1 |
| 002894 | 57 | 4.6 | 9.2 | 6.1 |
| 002894 | 58 | 4.7 | 9.3 | 6.1 |
| 002894 | 59 | 4.8 | 9.3 | 6.1 |
| 002894 | 60 | 4.8 | 9.3 | 6.1 |
| 002894 | 61 | 4.8 | 9.3 | 6.1 |
| 002894 | 62 | 4.8 | 9.3 | 6.0 |
| 002894 | 63 | 4.9 | 9.3 | 6.0 |
| 002894 | 64 | 4.9 | 9.3 | 6.0 |
| 002894 | 65 | 4.9 | 9.3 | 6.0 |
| 002894 | 66 | 4.9 | 9.3 | 6.0 |
| 002894 | 67 | 4.8 | 9.3 | 6.1 |

| Bridge Recall Number | Span | Peak Significant Wave Height (ft.) | Associated Storm Water Level (ft-MSL) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|--|-------------------------------------|
| 002894 | 68 | 4.9 | 9.3 | 6.0 |
| 002894 | 69 | 5.0 | 9.3 | 6.0 |
| 002894 | 70 | 5.0 | 9.3 | 6.0 |
| 002894 | 71 | 5.0 | 9.3 | 6.0 |
| 002894 | 72 | 5.0 | 9.3 | 6.0 |
| 002894 | 73 | 5.0 | 9.3 | 6.0 |
| 002894 | 74 | 5.1 | 9.3 | 6.0 |
| 002894 | 75 | 5.1 | 9.3 | 6.0 |
| 002894 | 76 | 5.1 | 9.3 | 6.0 |
| 002894 | 77 | 4.9 | 9.3 | 6.0 |
| 002894 | 78 | 5.1 | 9.4 | 6.0 |
| 002894 | 79 | 5.1 | 9.4 | 6.1 |
| 002894 | 80 | 5.1 | 9.4 | 6.0 |
| 002894 | 81 | 5.1 | 9.4 | 6.0 |
| 002894 | 82 | 5.1 | 9.4 | 6.0 |
| 002894 | 83 | 5.1 | 9.4 | 6.0 |
| 002894 | 84 | 5.1 | 9.4 | 6.0 |
| 002894 | 85 | 5.1 | 9.4 | 6.0 |
| 002894 | 86 | 5.1 | 9.4 | 6.0 |
| 002894 | 87 | 5.1 | 9.4 | 6.0 |
| 002894 | 88 | 5.1 | 9.4 | 6.0 |
| 002894 | 89 | 5.1 | 9.4 | 6.0 |
| 002894 | 90 | 5.1 | 9.4 | 6.0 |
| 002894 | 91 | 5.1 | 9.4 | 6.0 |
| 002894 | 92 | 5.1 | 9.4 | 6.0 |
| 002894 | 93 | 5.1 | 9.4 | 6.0 |
| 002894 | 94 | 5.1 | 9.4 | 6.0 |
| 002894 | 95 | 5.1 | 9.4 | 6.0 |
| 002894 | 96 | 5.1 | 9.4 | 6.0 |
| 002894 | 97 | 5.1 | 9.4 | 6.0 |
| 002894 | 98 | 5.1 | 9.4 | 6.0 |
| 002894 | 99 | 5.1 | 9.4 | 6.0 |
| 002894 | 100 | 5.1 | 9.4 | 6.0 |
| 002894 | 101 | 5.1 | 9.4 | 6.0 |
| 002894 | 102 | 5.1 | 9.4 | 6.0 |

| Bridge Recall Number | Span | Peak Significant Wave Height (ft.) | Associated Storm Water Level (ft-MSL) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|--|-------------------------------------|
| 002894 | 103 | 5.1 | 9.4 | 6.0 |
| 002894 | 104 | 5.1 | 9.4 | 6.0 |
| 002894 | 105 | 5.2 | 9.5 | 6.0 |
| 002894 | 106 | 5.2 | 9.5 | 6.0 |
| 002894 | 107 | 5.1 | 9.4 | 6.0 |
| 002894 | 108 | 5.2 | 9.5 | 6.0 |
| 002894 | 109 | 5.2 | 9.5 | 6.0 |
| 002894 | 110 | 5.3 | 9.5 | 6.0 |
| 002894 | 111 | 5.3 | 9.5 | 6.0 |
| 002894 | 112 | 5.3 | 9.5 | 6.1 |
| 002894 | 113 | 5.3 | 9.5 | 6.1 |
| 002894 | 114 | 5.3 | 9.5 | 6.1 |
| 002894 | 115 | 5.3 | 9.5 | 6.1 |
| 002894 | 116 | 5.3 | 9.5 | 6.1 |
| 002894 | 117 | 5.2 | 9.5 | 6.0 |
| 002894 | 118 | 5.3 | 9.5 | 6.1 |
| 002894 | 119 | 5.2 | 9.5 | 6.1 |
| 002894 | 120 | 5.2 | 9.5 | 6.1 |
| 002894 | 121 | 5.2 | 9.5 | 6.1 |
| 002894 | 122 | 5.3 | 9.5 | 6.1 |
| 002894 | 123 | 5.3 | 9.5 | 6.1 |
| 002894 | 124 | 5.3 | 9.5 | 6.1 |
| 002894 | 125 | 5.3 | 9.5 | 6.1 |
| 002894 | 126 | 5.3 | 9.5 | 6.1 |
| 002894 | 127 | 5.3 | 9.5 | 6.1 |
| 002894 | 128 | 5.3 | 9.5 | 6.1 |
| 002894 | 129 | 5.3 | 9.5 | 6.1 |
| 002894 | 130 | 5.4 | 9.5 | 6.1 |
| 002894 | 131 | 5.4 | 9.5 | 6.1 |
| 002894 | 132 | 5.4 | 9.6 | 6.1 |
| 002894 | 133 | 5.4 | 9.6 | 6.1 |
| 002894 | 134 | 5.4 | 9.6 | 6.1 |
| 002894 | 135 | 5.4 | 9.6 | 6.1 |
| 002894 | 136 | 5.3 | 9.5 | 6.1 |
| 002894 | 137 | 5.4 | 9.6 | 6.1 |

| Bridge Recall Number | Span | Peak Significant Wave Height (ft.) | Associated Storm Water Level (ft-MSL) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|--|-------------------------------------|
| 002894 | 138 | 5.5 | 9.6 | 6.1 |
| 002894 | 139 | 5.6 | 9.6 | 6.0 |
| 002894 | 140 | 5.6 | 9.6 | 6.0 |
| 002894 | 141 | 5.6 | 9.6 | 6.0 |
| 002894 | 142 | 5.6 | 9.6 | 6.0 |
| 002894 | 143 | 5.6 | 9.6 | 6.1 |
| 002894 | 144 | 5.6 | 9.6 | 6.1 |
| 002894 | 145 | 5.6 | 9.6 | 6.1 |
| 002894 | 146 | 5.5 | 9.6 | 6.1 |
| 002894 | 147 | 5.6 | 9.6 | 6.1 |
| 002894 | 148 | 5.8 | 9.6 | 6.1 |
| 002894 | 149 | 5.8 | 9.6 | 6.1 |
| 002894 | 150 | 5.8 | 9.6 | 6.1 |
| 002894 | 151 | 5.8 | 9.6 | 6.1 |
| 002894 | 152 | 5.8 | 9.6 | 6.1 |
| 002894 | 153 | 5.8 | 9.6 | 6.0 |
| 002894 | 154 | 5.8 | 9.6 | 6.0 |
| 002894 | 155 | 5.8 | 9.6 | 6.0 |
| 002894 | 156 | 5.7 | 9.6 | 6.1 |
| 002894 | 157 | 5.8 | 9.6 | 6.0 |
| 002894 | 158 | 5.9 | 9.7 | 6.0 |
| 002894 | 159 | 5.9 | 9.7 | 6.0 |
| 002894 | 160 | 5.8 | 9.7 | 6.0 |
| 002894 | 161 | 5.7 | 9.7 | 6.0 |
| 002894 | 162 | 5.7 | 9.7 | 6.0 |
| 002894 | 163 | 5.7 | 9.7 | 6.0 |
| 002894 | 164 | 5.7 | 9.7 | 6.1 |
| 002894 | 165 | 5.7 | 9.7 | 6.1 |
| 002894 | 166 | 5.8 | 9.6 | 6.0 |
| 002894 | 167 | 5.7 | 9.7 | 6.1 |
| 002894 | 168 | 5.7 | 9.7 | 6.0 |
| 002894 | 169 | 5.8 | 9.7 | 6.0 |
| 002894 | 170 | 5.8 | 9.7 | 6.0 |
| 002894 | 171 | 5.8 | 9.7 | 6.0 |
| 002894 | 172 | 5.8 | 9.7 | 6.0 |

| Bridge Recall Number | Span | Peak Significant Wave Height (ft.) | Associated Storm Water Level (ft-MSL) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|--|-------------------------------------|
| 002894 | 173 | 5.8 | 9.7 | 6.0 |
| 002894 | 174 | 5.8 | 9.7 | 6.0 |
| 002894 | 175 | 5.8 | 9.7 | 6.0 |
| 002894 | 176 | 5.7 | 9.7 | 6.1 |
| 002894 | 177 | 5.8 | 9.7 | 6.0 |
| 002894 | 178 | 5.8 | 9.7 | 6.0 |
| 002894 | 179 | 5.7 | 9.7 | 6.0 |
| 002894 | 180 | 5.7 | 9.7 | 6.0 |
| 002894 | 181 | 5.7 | 9.7 | 6.0 |
| 002894 | 182 | 5.7 | 9.8 | 6.0 |
| 002894 | 183 | 5.8 | 9.8 | 6.0 |
| 002894 | 184 | 5.8 | 9.8 | 6.0 |
| 002894 | 185 | 5.8 | 9.8 | 6.0 |
| 002894 | 186 | 5.8 | 9.8 | 6.0 |
| 002894 | 187 | 5.8 | 9.8 | 6.0 |
| 002894 | 188 | 5.7 | 9.8 | 6.0 |
| 002894 | 189 | 5.7 | 9.8 | 6.0 |
| 002894 | 190 | 5.7 | 9.8 | 6.0 |
| 002894 | 191 | 5.7 | 9.8 | 6.0 |
| 002894 | 192 | 5.7 | 9.8 | 6.0 |
| 002894 | 193 | 5.7 | 9.8 | 6.0 |
| 002894 | 194 | 5.8 | 9.8 | 6.0 |
| 002894 | 195 | 5.7 | 9.8 | 6.0 |
| 002894 | 196 | 5.6 | 9.8 | 6.0 |
| 002894 | 197 | 5.6 | 9.8 | 6.0 |
| 002894 | 198 | 5.6 | 9.8 | 6.0 |
| 002894 | 199 | 5.6 | 9.8 | 6.0 |
| 002894 | 200 | 5.6 | 9.8 | 6.0 |
| 002894 | 201 | 5.6 | 9.8 | 6.0 |
| 002894 | 202 | 5.6 | 9.8 | 6.0 |
| 002894 | 203 | 5.5 | 9.8 | 6.0 |
| 002894 | 204 | 5.6 | 9.8 | 6.0 |
| 002894 | 205 | 5.5 | 9.9 | 6.0 |
| 002894 | 206 | 5.5 | 9.9 | 6.0 |
| 002894 | 207 | 5.5 | 9.9 | 6.1 |

| Bridge Recall Number | Span | Peak Significant Wave Height (ft.) | Associated Storm Water Level (ft-MSL) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|--|-------------------------------------|
| 002894 | 208 | 5.5 | 9.9 | 6.0 |
| 002894 | 209 | 5.4 | 9.9 | 6.0 |
| 002894 | 210 | 5.4 | 9.9 | 6.0 |
| 002894 | 211 | 5.4 | 9.9 | 6.0 |
| 002894 | 212 | 5.4 | 9.9 | 6.0 |
| 002894 | 213 | 5.4 | 9.9 | 6.0 |
| 002894 | 214 | 5.4 | 9.9 | 6.0 |
| 002894 | 215 | 5.4 | 9.9 | 6.0 |
| 002894 | 216 | 5.4 | 9.9 | 6.0 |
| 002894 | 217 | 5.4 | 9.9 | 6.0 |
| 002894 | 218 | 5.4 | 9.9 | 6.1 |
| 002894 | 219 | 5.3 | 9.9 | 6.1 |
| 002894 | 220 | 5.3 | 9.9 | 6.1 |
| 002894 | 221 | 5.3 | 9.9 | 6.0 |
| 002894 | 222 | 5.3 | 9.9 | 6.0 |
| 002894 | 223 | 5.3 | 9.9 | 6.0 |
| 002894 | 224 | 5.3 | 10.0 | 6.0 |
| 002894 | 225 | 5.3 | 10.0 | 6.0 |
| 002894 | 226 | 5.3 | 10.0 | 6.0 |
| 002894 | 227 | 5.2 | 10.0 | 6.0 |
| 002894 | 228 | 5.1 | 10.0 | 6.0 |
| 002894 | 229 | 5.1 | 10.0 | 6.0 |
| 002894 | 230 | 5.1 | 10.0 | 6.0 |
| 002894 | 231 | 5.0 | 10.0 | 6.0 |
| 002894 | 232 | 5.0 | 10.0 | 6.1 |
| 002894 | 233 | 4.9 | 10.0 | 6.1 |
| 002894 | 234 | 4.9 | 10.0 | 6.1 |
| 002894 | 235 | 4.8 | 10.0 | 6.1 |
| 002894 | 236 | 4.8 | 10.1 | 6.1 |
| 002894 | 237 | 4.7 | 10.1 | 6.1 |
| 002894 | 238 | 4.7 | 10.1 | 6.1 |
| 002894 | 239 | 4.7 | 10.1 | 6.1 |
| 002894 | 240 | 4.7 | 10.1 | 6.1 |
| 002894 | 241 | 4.7 | 10.1 | 6.1 |
| 002894 | 242 | 4.6 | 10.1 | 6.1 |

| Bridge Recall Number | Span | Peak Significant Wave Height (ft.) | Associated Storm Water Level (ft-MSL) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|--|-------------------------------------|
| 002894 | 243 | 4.6 | 10.1 | 6.1 |
| 002894 | 244 | 4.6 | 10.1 | 6.1 |
| 002894 | 245 | 4.6 | 10.1 | 6.1 |
| 002894 | 246 | 5.2 | 10.0 | 6.0 |
| 002894 | 247 | 4.6 | 10.1 | 6.1 |
| 003432 | 1 | 7.0 | 9.5 | 4.8 |
| 003432 | 2 | 7.0 | 9.5 | 4.8 |
| 003432 | 3 | 7.0 | 9.5 | 4.8 |
| 003432 | 4 | 7.0 | 9.5 | 4.8 |
| 003432 | 5 | 7.0 | 9.5 | 4.8 |
| 003432 | 6 | 7.0 | 9.5 | 4.8 |
| 003432 | 7 | 7.0 | 9.5 | 4.8 |
| 003432 | 8 | 7.0 | 9.5 | 4.8 |
| 003432 | 9 | 7.0 | 9.5 | 4.8 |
| 003432 | 10 | 7.0 | 9.4 | 4.9 |
| 003432 | 11 | 7.0 | 9.4 | 4.9 |
| 003432 | 12 | 7.0 | 9.4 | 4.9 |
| 003432 | 13 | 7.0 | 9.4 | 4.9 |
| 003432 | 14 | 7.0 | 9.5 | 4.9 |
| 003432 | 15 | 7.0 | 9.5 | 4.9 |
| 003432 | 16 | 7.0 | 9.5 | 4.9 |
| 003432 | 17 | 7.0 | 9.5 | 4.9 |
| 003440 | 1 | 5.9 | 8.1 | 4.5 |
| 003440 | 2 | 5.9 | 8.1 | 4.5 |
| 003440 | 3 | 5.9 | 8.1 | 4.5 |
| 003440 | 4 | 5.9 | 8.1 | 4.5 |
| 003440 | 5 | 5.9 | 8.1 | 4.5 |
| 003440 | 6 | 5.9 | 8.1 | 4.5 |
| 003440 | 7 | 5.9 | 8.1 | 4.5 |
| 003440 | 8 | 5.8 | 8.1 | 4.4 |
| 003440 | 9 | 5.8 | 8.1 | 4.4 |
| 003450 | 1 | 4.8 | 8.0 | 4.8 |
| 003450 | 2 | 4.8 | 7.9 | 4.8 |
| 003450 | 3 | 4.8 | 7.8 | 4.8 |
| 003450 | 4 | 4.8 | 7.8 | 4.8 |

| Bridge Recall Number | Span | Peak Significant Wave Height (ft.) | Associated Storm Water Level (ft-MSL) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|--|-------------------------------------|
| 003450 | 5 | 4.8 | 7.8 | 4.8 |
| 003450 | 6 | 4.8 | 7.8 | 4.8 |
| 003480 | 1 | 1.8 | 2.4 | 2.2 |
| 003480 | 2 | 1.8 | 2.3 | 2.2 |
| 003480 | 3 | 1.9 | 2.4 | 2.2 |
| 003480 | 4 | 1.9 | 2.3 | 2.2 |
| 003480 | 5 | 1.9 | 2.6 | 2.3 |
| 003510 | 1 | 4.7 | 8.9 | 4.4 |
| 003510 | 2 | 4.7 | 8.9 | 4.4 |
| 003510 | 3 | 4.7 | 8.9 | 4.4 |
| 003510 | 4 | 4.7 | 8.9 | 4.4 |
| 003510 | 5 | 4.7 | 8.9 | 4.4 |
| 003510 | 6 | 4.8 | 8.9 | 4.4 |
| 003510 | 7 | 4.8 | 8.9 | 4.4 |
| 003510 | 8 | 4.8 | 8.9 | 4.4 |
| 003510 | 9 | 4.8 | 8.9 | 4.4 |
| 003520 | 1 | 4.4 | 9.2 | 4.3 |
| 003520 | 2 | 4.4 | 9.2 | 4.3 |
| 003520 | 3 | 4.4 | 9.2 | 4.3 |
| 003690 | 1 | 6.9 | 9.3 | 4.9 |
| 003690 | 2 | 6.9 | 9.3 | 4.9 |
| 003690 | 3 | 6.9 | 9.3 | 4.9 |
| 003690 | 4 | 6.9 | 9.3 | 4.9 |
| 003690 | 5 | 6.9 | 9.3 | 4.9 |
| 009030 | 1 | 2.1 | 3.9 | 4.3 |
| 009030 | 2 | 2.3 | 3.4 | 4.5 |
| 009030 | 3 | 2.4 | 2.5 | 4.1 |
| 009030 | 4 | 2.4 | 2.3 | 4.1 |
| 009030 | 5 | 2.4 | 2.3 | 4.1 |
| 009030 | 6 | 2.4 | 2.3 | 4.1 |
| 009030 | 7 | 2.3 | 3.2 | 4.2 |
| 009030 | 8 | 2.1 | 3.9 | 4.2 |
| 031755 | 1 | 1.4 | 5.6 | 3.7 |
| 031755 | 2 | 1.4 | 5.5 | 3.3 |
| 031755 | 3 | 1.4 | 5.4 | 3.6 |

| Bridge Recall Number | Span | Peak Significant Wave Height (ft.) | Associated Storm Water Level (ft-MSL) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|--|-------------------------------------|
| 031755 | 4 | 1.5 | 5.2 | 4.2 |
| 031755 | 5 | 1.6 | 5.0 | 4.5 |
| 031755 | 6 | 1.7 | 4.7 | 4.7 |
| 031755 | 7 | 1.7 | 4.5 | 4.6 |
| 031755 | 8 | 1.8 | 4.5 | 4.6 |
| 031755 | 9 | 1.8 | 4.1 | 5.2 |
| 031755 | 10 | 1.9 | 3.8 | 2.4 |
| 031755 | 11 | 2.0 | 2.8 | 2.3 |
| 031755 | 12 | 2.0 | 2.9 | 2.3 |
| 031755 | 13 | 2.0 | 3.0 | 2.2 |
| 031755 | 14 | 2.1 | 3.2 | 2.2 |
| 031755 | 15 | 2.1 | 3.4 | 2.3 |
| 031755 | 16 | 2.1 | 3.6 | 2.3 |
| 031755 | 17 | 2.1 | 4.2 | 2.4 |
| 031755 | 18 | 2.2 | 4.3 | 3.2 |
| 031755 | 19 | 2.2 | 4.7 | 3.3 |
| 031755 | 20 | 2.1 | 5.1 | 3.9 |
| 031755 | 21 | 2.1 | 5.2 | 4.3 |
| 031755 | 22 | 2.1 | 5.3 | 4.8 |
| 031755 | 23 | 2.1 | 5.3 | 5.1 |
| 031755 | 24 | 1.9 | 5.5 | 5.0 |
| 031755 | 25 | 1.8 | 5.4 | 4.8 |
| 031755 | 26 | 1.7 | 5.4 | 4.7 |
| 031755 | 27 | 1.6 | 5.4 | 4.7 |
| 033698 | 1 | 2.6 | 6.8 | 2.6 |
| 033698 | 2 | 2.8 | 7.6 | 2.7 |
| 033700 | 1 | 4.1 | 9.2 | 3.8 |
| 033700 | 2 | 4.1 | 9.4 | 3.7 |
| 033700 | 3 | 4.1 | 9.4 | 3.7 |
| 033700 | 4 | 4.0 | 9.5 | 3.7 |
| 033700 | 5 | 4.0 | 9.5 | 3.7 |
| 033700 | 6 | 4.0 | 9.5 | 3.7 |
| 033700 | 7 | 3.9 | 9.4 | 3.7 |
| 033700 | 8 | 3.9 | 9.4 | 3.7 |
| 033700 | 9 | 3.8 | 9.4 | 3.7 |

| Bridge Recall Number | Span | Peak Significant Wave Height (ft.) | Associated Storm Water Level (ft-MSL) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|--|-------------------------------------|
| 033700 | 10 | 3.8 | 9.3 | 3.7 |
| 033700 | 11 | 3.7 | 9.3 | 3.7 |
| 033700 | 12 | 3.6 | 9.0 | 3.8 |
| 033700 | 13 | 3.6 | 9.2 | 3.8 |
| 033700 | 14 | 3.5 | 9.1 | 3.7 |
| 033700 | 15 | 3.6 | 9.1 | 3.7 |
| 033700 | 16 | 3.6 | 9.1 | 3.6 |
| 033700 | 17 | 3.7 | 9.1 | 3.6 |
| 033700 | 18 | 3.7 | 9.1 | 3.6 |
| 033700 | 19 | 3.7 | 9.1 | 3.6 |
| 033700 | 20 | 3.7 | 9.1 | 3.6 |
| 033700 | 21 | 3.7 | 9.1 | 3.6 |
| 033700 | 22 | 3.7 | 9.1 | 3.6 |
| 033700 | 23 | 3.7 | 9.2 | 3.6 |
| 033700 | 24 | 3.7 | 9.1 | 3.6 |
| 033700 | 25 | 3.7 | 9.1 | 3.6 |
| 033700 | 26 | 3.7 | 9.1 | 3.6 |
| 033700 | 27 | 3.7 | 9.1 | 3.5 |
| 033700 | 28 | 3.6 | 9.1 | 3.5 |
| 033700 | 29 | 3.6 | 9.1 | 3.5 |
| 033700 | 30 | 3.6 | 9.0 | 3.5 |
| 033700 | 31 | 3.6 | 9.0 | 3.5 |
| 033700 | 32 | 3.6 | 9.0 | 3.5 |
| 033700 | 33 | 3.6 | 9.0 | 3.5 |
| 033700 | 34 | 3.6 | 9.0 | 3.5 |
| 033700 | 35 | 3.6 | 9.0 | 3.5 |
| 059482 | 1 | 2.0 | 2.8 | 2.3 |
| 059482 | 2 | 2.0 | 2.8 | 2.3 |
| 059482 | 3 | 1.9 | 3.0 | 2.3 |
| 059482 | 4 | 1.7 | 3.9 | 2.4 |
| 059482 | 5 | 1.7 | 3.1 | 2.3 |
| 059482 | 6 | 1.5 | 2.3 | 2.2 |
| 060360 | 1 | 2.4 | 6.1 | 5.1 |
| 060360 | 2 | 2.4 | 6.1 | 5.1 |
| 060360 | 3 | 2.4 | 6.1 | 5.2 |

| Bridge Recall Number | Span | Peak Significant Wave Height (ft.) | Associated Storm Water Level (ft-MSL) | Associated Peak Period (sec) |
|-----------------------------|-------------|---|--|-------------------------------------|
| 060360 | 4 | 2.4 | 6.1 | 5.2 |
| 060360 | 5 | 2.4 | 6.1 | 5.2 |

Detailed Wave Force Results

LADOTD BRIDGE RECALL NO. 002631
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| SPAN NUMBER | 1 | 2 | 3 |
|-------------------------------------|-----|-----|-----|
| Criticality Index (defined below) | 3 | 3 | 3 |
| Vulnerability Index (defined below) | 1.4 | 1.3 | 1.2 |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | |
|--|------|
| [100-yr Water Surface Elevation (ft - MSL)] | 8.6 |
| Bad Elevation (ft - MSL) | 1 |
| Low Chord Elevation (ft - MSL) | 6.1 |
| [100-yr Max Wave Crest Elevation (ft - MSL)] | 12.1 |
| [100-yr Wave Height (ft)] | 5.0 |
| [100-yr Wave Period (seconds)] | 3.9 |

SPAN PROPERTIES

| Span Length (ft) | 20.0 | 20.0 | 20.0 |
|-----------------------------------|-------|-------|-------|
| Span Width (ft) | 42.8 | 42.8 | 42.8 |
| Deck Thickness (ft) | 1.1 | 1.1 | 1.1 |
| Overhang (ft) | | | |
| Number of Beams | | | |
| Beam Dead Weight (lb/ft) - Each | | | |
| Beam Dead Weight (kip/ft) - Total | 7.0 | 7.0 | 7.0 |
| Slab Dead Weight (kip/ft) | 7.0 | 7.0 | 7.0 |
| Total Dead Weight (kip/ft) | 149.0 | 149.0 | 149.0 |
| Resisting Moment (kip/ft) | 7.0 | 7.0 | 7.0 |
| Resisting Vertical Force (kip/ft) | | | |

100-YEAR FORCE-MOMENT VALUES

| Maximum Vertical Force (kips/span) | 72.3 | 72.8 | 70.1 |
|--------------------------------------|--------|--------|--------|
| Maximum Vertical Force (kips/ft) | 3.6 | 3.6 | 3.5 |
| Maximum Horizontal Force (kips/span) | 9.3 | 12.5 | 10.2 |
| Maximum Horizontal Force (kips/ft) | 0.5 | 0.6 | 0.5 |
| Maximum Moment (kip-ft) | 2426.7 | 2294.9 | 2099.6 |
| Maximum Moment (k-ft/ft) | 121.3 | 114.7 | 105.0 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable |
|----------------------------|----------------|------------------------|
|----------------------------|----------------|------------------------|

Notes:

- 1 - Bridge spans 1-3 are potentially subject to wave energy.
- 2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

| Criticality Index | Multipplier | Description |
|-------------------|-------------|--|
| 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

Table 1 of 1

LADOTD BRIDGE RECALL NO. 002632
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| SPAN NUMBER | 1 | 2 | 3 |
|-------------------------------------|-----|-----|-----|
| Criticality Index (defined below) | 3 | 3 | 3 |
| Vulnerability Index (defined below) | 1.2 | 1.4 | 1.2 |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | |
|--|------|
| [100-yr Water Surface Elevation (ft - MSL)] | 8.6 |
| Bad Elevation (ft - MSL) | 1 |
| Low Chord Elevation (ft - MSL) | 6.1 |
| [100-yr Max Wave Crest Elevation (ft - MSL)] | 12.1 |
| [100-yr Wave Height (ft)] | 5.0 |
| [100-yr Wave Period (seconds)] | 3.9 |

| SPAN PROPERTIES | |
|-----------------------------------|-------|
| Span Length (ft) | 20.0 |
| Span Width (ft) | 42.8 |
| Deck Thickness (ft) | 1.1 |
| Overhang (ft) | 1.1 |
| Number of Beams | |
| Beam Dead Weight (lb/ft) - Each | |
| Beam Dead Weight (kip/ft) - Total | |
| Slab Dead Weight (kip/ft) | 7.0 |
| Total Dead Weight (kip/ft) | 7.0 |
| Resisting Moment (kip/ft) | 149.0 |
| Resisting Vertical Force (kip/ft) | 7.0 |

| 100-YEAR FORCE-MOMENT VALUES | |
|--------------------------------------|--------|
| Maximum Vertical Force (kips/span) | 69.1 |
| Maximum Vertical Force (kips/ft) | 3.5 |
| Maximum Horizontal Force (kips/span) | 10.7 |
| Maximum Horizontal Force (kips/ft) | 0.5 |
| Maximum Moment (kip-ft) | 2051.8 |
| Maximum Moment (kip-ft/ft) | 102.6 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable | Criticality Index | Multiplier | Description |
|----------------------------|----------------|------------------------|-------------------|------------|--|
| | | | 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| | | | 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| | | | 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| | | | 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

Notes:
 1 - Bridge spans 1-3 are potentially subject to wave energy.
 2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

LADOTD BRIDGE RECALL NO. 002650
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| SPAN NUMBER | 1 | 2 | 3 | 4 | 5 |
|-------------------------------------|-----|-----|-----|-----|-----|
| Criticality Index (defined below) | 3 | 3 | 3 | 3 | 3 |
| Vulnerability Index (defined below) | 0.9 | 1.5 | 1.6 | 1.6 | 0.9 |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | |
|--|------|
| [100-yr] Water Surface Elevation (ft - MSL) | 8.0 |
| Bed Elevation (ft - MSL) | 3 |
| Low Chord Elevation (ft - MSL) | 7.2 |
| [100-yr] Max Wave Crest Elevation (ft - MSL) | 11.1 |
| [100-yr] Wave Height (ft) | 4.4 |
| [100-yr] Wave Period (seconds) | 3.6 |

SPAN PROPERTIES

| | | | | | |
|-----------------------------------|------|------|------|------|------|
| Span Length (ft) | 17.0 | 17.0 | 55.0 | 17.0 | 17.0 |
| Span Width (ft) | 28.9 | 28.9 | 28.9 | 28.9 | 28.9 |
| Deck Thickness (ft) | 1.0 | 1.0 | 0.6 | 1.0 | 1.0 |
| Overhang (ft) | | | 4.4 | | |
| Number of Beams | | | 4 | | |
| Beam Dead Weight (lb/ft) - Each | | | 129 | | |
| Beam Dead Weight (kip/ft) - Total | | | 0.5 | | |
| Slab Dead Weight (kip/ft) | 4.3 | 4.3 | 8.9 | 4.3 | 4.3 |
| Total Dead Weight (kip/ft) | 4.3 | 4.3 | 9.4 | 4.3 | 4.3 |
| Resisting Moment (kip/ft) | 62.6 | 62.6 | 94.1 | 62.6 | 62.6 |
| Resisting Vertical Force (kip/ft) | 4.3 | 4.3 | 9.4 | 4.3 | 4.3 |

100-YEAR FORCE-MOMENT VALUES

| | | | | | |
|--------------------------------------|-------|-------|---------|-------|-------|
| Maximum Vertical Force (kips/span) | 30.4 | 42.3 | 220.5 | 42.3 | 30.4 |
| Maximum Vertical Force (kips/ft) | 1.8 | 2.5 | 4.0 | 2.5 | 1.8 |
| Maximum Horizontal Force (kips/span) | 11.8 | 14.7 | 67.8 | 27.4 | 11.8 |
| Maximum Horizontal Force (kips/ft) | 0.7 | 0.9 | 1.2 | 1.6 | 0.7 |
| Maximum Moment (kip-ft) | 577.7 | 925.2 | 4,664.8 | 943.7 | 577.7 |
| Maximum Moment (k-ft/ft) | 34.0 | 54.4 | 84.8 | 55.5 | 33.9 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable |
|----------------------------|----------------|------------------------|
|----------------------------|----------------|------------------------|

Notes:

- 1 - Bridge spans 2-4 are potentially subject to wave energy.
- 2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

| Criticality Index | Multiplier | Description |
|-------------------|------------|--|
| 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

LADOTD BRIDGE RECALL NO. 002892
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| BRIDGE VULNERABILITY SUMMARY | | | | | | | | | |
|--|------|------|------|------|------|------|------|------|------|
| SPAN NUMBER | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 |
| Criticality Index (defined below) | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 |
| Vulnerability Index (defined below) | 0.4 | 0.3 | 0.3 | 0.1 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 100-yr Water Surface Elevation (ft - MSL) | 9.1 | 9.1 | 9.1 | 9.1 | 9.1 | 9.0 | 9.1 | 9.1 | 9.1 |
| Bad Elevation (ft - MSL) | -0.1 | -0.2 | -0.2 | -0.2 | -0.2 | -0.3 | -0.1 | -0.3 | -0.4 |
| Low Chord Elevation (ft - MSL) | 12.0 | 12.0 | 12.2 | 12.9 | 13.4 | 14.2 | 15.3 | 16.6 | 18.1 |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 13.2 | 13.2 | 13.3 | 13.3 | 13.3 | 13.3 | 13.3 | 13.2 | 13.4 |
| 100-yr Wave Height (ft) | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.1 | 6.1 | 6.0 | 6.1 |
| 100-yr Wave Period (seconds) | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.1 | 6.1 |

| SURGE/WAVE LOAD COMPUTATION INPUT VALUES | | | | | | | | | |
|--|--|--|--|--|--|--|--|--|--|
|--|--|--|--|--|--|--|--|--|--|

| HYDRAULIC VALUES | | | | | | | | | |
|--|------|------|------|------|------|------|------|------|------|
| 100-yr Water Surface Elevation (ft - MSL) | 9.1 | 9.1 | 9.1 | 9.1 | 9.1 | 9.0 | 9.1 | 9.1 | 9.1 |
| Bad Elevation (ft - MSL) | -0.1 | -0.2 | -0.2 | -0.2 | -0.2 | -0.3 | -0.1 | -0.3 | -0.4 |
| Low Chord Elevation (ft - MSL) | 12.0 | 12.0 | 12.2 | 12.9 | 13.4 | 14.2 | 15.3 | 16.6 | 18.1 |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 13.2 | 13.2 | 13.3 | 13.3 | 13.3 | 13.3 | 13.3 | 13.2 | 13.4 |
| 100-yr Wave Height (ft) | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.1 | 6.1 | 6.0 | 6.1 |
| 100-yr Wave Period (seconds) | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.1 | 6.1 |

| SPAN PROPERTIES | | | | | | | | | |
|-----------------------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Span Length (ft) | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 |
| Span Width (ft) | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 |
| Deck Thickness (ft) | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 |
| Overhang (ft) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Number of Beams | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 |
| Beam Dead Weight (lb/ft) - Each | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 |
| Beam Dead Weight (kip/ft) - Total | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 |
| Slab Dead Weight (kip/ft) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Total Dead Weight (kip/ft) | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Resisting Moment (kip/ft) | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 |
| Resisting Vertical Force (kip/ft) | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |

| 100-YEAR FORCE-MOMENT VALUES | | | | | | | | | |
|--------------------------------------|--------|--------|-------|-------|------|-----|-----|-----|-----|
| Maximum Vertical Force (kips/span) | 96.2 | 95.4 | 74.4 | 40.6 | 12.6 | 5.3 | 1.5 | 0.3 | 0.0 |
| Maximum Vertical Force (kips/ft) | 1.5 | 1.5 | 1.1 | 0.6 | 0.2 | 0.1 | 0.0 | 0.0 | 0.0 |
| Maximum Horizontal Force (kips/span) | 56.2 | 59.5 | 49.1 | 27.1 | 12.2 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Horizontal Force (kips/ft) | 0.9 | 0.9 | 0.8 | 0.4 | 0.2 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Moment (kip-ft) | 2032.2 | 1694.6 | 836.7 | 505.3 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Moment (k-ft) | 31.3 | 26.1 | 12.9 | 7.8 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |

| Criticality Index | Multiplier | Description |
|-------------------|------------|--|
| 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

Notes:

- 1 - Bridge spans are not potentially subject to wave energy.
- 2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

LADOTD BRIDGE RECALL NO. 002892
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| SPAN NUMBER | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | |
|-------------------------------------|------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| | 23 | 24 | 25 | 26 | 27 | 28 | 29 | 30 | 31 | 32 |
| Criticality Index (defined below) | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 |
| Vulnerability Index (defined below) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | |
|--|------|
| 100-yr Water Surface Elevation (ft - MSL) | 9.2 |
| Bed Elevation (ft - MSL) | -0.6 |
| Low Chord Elevation (ft - MSL) | 29.0 |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 13.6 |
| 100-yr Wave Height (ft) | 6.3 |
| 100-yr Wave Period (seconds) | 6.1 |

| SPAN PROPERTIES | |
|-----------------------------------|-------|
| Span Length (ft) | 65.0 |
| Span Width (ft) | 45.8 |
| Deck Thickness (ft) | 0.6 |
| Overhang (ft) | 3.0 |
| Number of Beams | 6 |
| Beam Dead Weight (lb/ft) - Each | 583 |
| Beam Dead Weight (lb/ft) - Total | 3.5 |
| Slab Dead Weight (kip/ft) | 4.0 |
| Total Dead Weight (kip/ft) | 7.5 |
| Resisting Moment (kip-ft) | 148.9 |
| Resisting Vertical Force (kip/ft) | 7.5 |

| 100-YEAR FORCE-MOMENT VALUES | |
|--------------------------------------|-----|
| Maximum Vertical Force (kips/span) | 0.0 |
| Maximum Vertical Force (kip/ft) | 0.0 |
| Maximum Horizontal Force (kips/span) | 0.0 |
| Maximum Horizontal Force (kip/ft) | 0.0 |
| Maximum Moment (kip-ft) | 0.0 |
| Maximum Moment (kip-ft) | 0.0 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable |
|----------------------------|----------------|------------------------|
|----------------------------|----------------|------------------------|

| Criticality Index | Multiplier | Description |
|-------------------|------------|--|
| 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

Notes:

1 - Bridge spans are not potentially subject to wave energy.

2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

LADOTD BRIDGE RECALL NO. 002892
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| BRIDGE VULNERABILITY SUMMARY | | | | | | | | | |
|-------------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| SPAN NUMBER | 41 | 42 | 43 | 44 | 45 | 46 | 47 | 48 | 49 |
| Criticality Index (defined below) | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 |
| Vulnerability Index (defined below) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| | | | | | | | | | |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | |
|--|------|
| 100-yr Water Surface Elevation (ft - MSL) | 9.2 |
| Bed Elevation (ft - MSL) | -1.0 |
| Low Chord Elevation (ft - MSL) | 21.7 |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 13.9 |
| 100-yr Wave Height (ft) | 6.7 |
| 100-yr Wave Period (seconds) | 6.1 |
| | |

| SPAN PROPERTIES | |
|-----------------------------------|-------|
| Span Length (ft) | 65.0 |
| Span Width (ft) | 45.8 |
| Deck Thickness (ft) | 0.6 |
| Overhang (ft) | 3.0 |
| Number of Beams | 6 |
| Beam Dead Weight (lb/ft) - Each | 583 |
| Beam Dead Weight (kip/ft) - Total | 3.5 |
| Slab Dead Weight (kip/ft) | 4.0 |
| Total Dead Weight (kip/ft) | 7.5 |
| Resisting Moment (kip-ft) | 148.9 |
| Resisting Vertical Force (kip/ft) | 7.5 |
| | |

| 100-YEAR FORCE-MOMENT VALUES | |
|--------------------------------------|-----|
| Maximum Vertical Force (kips/span) | 0.0 |
| Maximum Vertical Force (kip/ft) | 0.0 |
| Maximum Horizontal Force (kips/span) | 0.0 |
| Maximum Horizontal Force (kip/ft) | 0.0 |
| Maximum Moment (kip-ft) | 0.0 |
| Maximum Moment (kip-ft) | 0.0 |
| | |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable | Criticality Index | Multiplier | Description |
|----------------------------|----------------|------------------------|-------------------|------------|--|
| | | | 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| | | | 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| | | | 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| | | | 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

- Notes:
- 1 - Bridge spans are not potentially subject to wave energy.
 - 2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

Table 3 of 14

LADOTD BRIDGE RECALL NO. 002892
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | | | | | | | | | |
|-------------------------------------|------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| SPAN NUMBER | 59 | 60 | 61 | 62 | 63 | 64 | 65 | 66 | 67 | 68 | 69 | 70 | 71 | 72 | 73 | 74 | 75 | 76 |
| Criticality Index (defined below) | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 |
| Vulnerability Index (defined below) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| | | | | | | | | | | | | | | | | | | |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | | |
|--|------------|--|
| 100-yr Water Surface Elevation (ft - MSL) | 9.3 | |
| Bed Elevation (ft - MSL) | -1.4 | |
| Low Chord Elevation (ft - MSL) | 15.3 | |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 14.2 | |
| 100-yr Wave Height (ft) | 7.0 | |
| 100-yr Wave Period (seconds) | 6.1 | |
| | | |
| SPAN PROPERTIES | | |
| Span Length (ft) | 65.0 | |
| Span Width (ft) | 45.8 | |
| Deck Thickness (ft) | 0.6 | |
| Overhang (ft) | 3.0 | |
| Number of Beams | 6 | |
| Beam Dead Weight (lb/ft) - Each | 583 | |
| Beam Dead Weight (lb/ft) - Total | 3.5 | |
| Slab Dead Weight (kip/ft) | 4.0 | |
| Total Dead Weight (kip/ft) | 7.5 | |
| Resisting Moment (kip-ft) | 148.9 | |
| Resisting Vertical Force (kip/ft) | 7.5 | |
| | | |
| 100-YEAR FORCE-MOMENT VALUES | | |
| Maximum Vertical Force (kips/span) | 6.1 | |
| Maximum Vertical Force (kip/ft) | 0.1 | |
| Maximum Horizontal Force (kips/span) | 0.0 | |
| Maximum Horizontal Force (kip/ft) | 0.0 | |
| Maximum Moment (kip-ft) | 0.0 | |
| Maximum Moment (kip-ft) | 0.0 | |
| | | |
| Vulnerability Index Legend | | |
| Not Vulnerable | Green | |
| Potentially Vulnerable | Red | |
| | | |
| Criticality Index | Multiplier | Description |
| 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

- Notes:
- 1 - Bridge spans are not potentially subject to wave energy.
 - 2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

LADOTD BRIDGE RECALL NO. 002892
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| SPAN NUMBER | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | |
|-------------------------------------|------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| | 77 | 78 | 79 | 80 | 81 | 82 | 83 | 84 | 85 | 86 |
| Criticality Index (defined below) | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 |
| Vulnerability Index (defined below) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | |
|--|------|
| 100-yr Water Surface Elevation (ft - MSL) | 9.4 |
| Bed Elevation (ft - MSL) | -1.7 |
| Low Chord Elevation (ft - MSL) | 15.3 |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 14.4 |
| 100-yr Wave Height (ft) | 7.2 |
| 100-yr Wave Period (seconds) | 6.0 |

| SPAN PROPERTIES | |
|-----------------------------------|-------|
| Span Length (ft) | 65.0 |
| Span Width (ft) | 45.8 |
| Deck Thickness (ft) | 0.6 |
| Overhang (ft) | 3.0 |
| Number of Beams | 6 |
| Beam Dead Weight (lb/ft) - Each | 583 |
| Beam Dead Weight (lb/ft) - Total | 3.5 |
| Slab Dead Weight (kip/ft) | 4.0 |
| Total Dead Weight (kip/ft) | 7.5 |
| Resisting Moment (kip-ft) | 148.9 |
| Resisting Vertical Force (kip/ft) | 7.5 |

| 100-YEAR FORCE-MOMENT VALUES | | | | | | | | | |
|--------------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| Maximum Vertical Force (kips/span) | 8.2 | 8.3 | 8.3 | 8.4 | 8.6 | 8.6 | 8.7 | 8.7 | 8.7 |
| Maximum Vertical Force (kip/ft) | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 |
| Maximum Horizontal Force (kips/span) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Horizontal Force (kip/ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Moment (kip-ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Moment (kip-ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable |
|----------------------------|----------------|------------------------|
|----------------------------|----------------|------------------------|

| Criticality Index | Multiplier | Description |
|-------------------|------------|--|
| 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

Notes:
 1 - Bridge spans are not potentially subject to wave energy.
 2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

LADOTD BRIDGE RECALL NO. 002892
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| BRIDGE VULNERABILITY SUMMARY | | | | | | | | | |
|-------------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| SPAN NUMBER | 95 | 96 | 97 | 98 | 99 | 100 | 101 | 102 | 103 |
| Criticality Index (defined below) | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 |
| Vulnerability Index (defined below) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| | | | | | | | | | |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | | | | | | | | | |
|--|-------|----------------|-------|------------------------|-------|-------------|-------|--|-------|
| 100-yr Water Surface Elevation (ft - MSL) | 9.4 | 9.4 | 9.4 | 9.4 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 |
| Bed Elevation (ft - MSL) | -1.7 | -1.7 | -1.7 | -1.7 | -1.7 | -1.8 | -1.8 | -1.7 | -1.9 |
| Bed Elevation (ft - MSL) | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | -2.0 |
| Low Chord Elevation (ft - MSL) | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 14.5 | 14.5 | 14.5 | 14.5 | 14.5 | 14.6 | 14.6 | 14.6 | 14.7 |
| 100-yr Wave Height (ft) | 7.2 | 7.2 | 7.3 | 7.3 | 7.3 | 7.3 | 7.3 | 7.3 | 7.4 |
| 100-yr Wave Period (seconds) | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 |
| SPAN PROPERTIES | | | | | | | | | |
| Span Length (ft) | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 |
| Span Width (ft) | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 |
| Deck Thickness (ft) | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 |
| Overhang (ft) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Number of Beams | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 |
| Beam Dead Weight (lb/ft) - Each | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 |
| Beam Dead Weight (lb/ft) - Total | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 |
| Slab Dead Weight (kip/ft) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Total Dead Weight (kip/ft) | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Resisting Moment (kip-ft) | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 |
| Resisting Vertical Force (kip/ft) | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| 100-YEAR FORCE-MOMENT VALUES | | | | | | | | | |
| Maximum Vertical Force (kips/span) | 8.7 | 9.1 | 9.2 | 9.3 | 9.4 | 9.5 | 9.7 | 9.8 | 10.1 |
| Maximum Vertical Force (kips/ft) | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.2 | 0.2 |
| Maximum Horizontal Force (kips/span) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Horizontal Force (kips/ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Moment (k-ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Moment (kip-ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Vulnerability Index Legend | | Not Vulnerable | | Potentially Vulnerable | | Description | | | |
| | | | | | | 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) | |
| | | | | | | 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists | |
| | | | | | | 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives | |
| | | | | | | 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) | |

- Notes:
- 1 - Bridge spans are not potentially subject to wave energy.
 - 2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

LADOTD BRIDGE RECALL NO. 002892
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| | | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | | | | | | | | | |
|-------------------------------------|-----------------------------------|------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| SPAN NUMBER | Criticality Index (defined below) | 113 | 114 | 115 | 116 | 117 | 118 | 119 | 120 | 121 | 122 | 123 | 124 | 125 | 126 | 127 | 128 | 129 | 130 |
| Vulnerability Index (defined below) | | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.1 | 0.1 |
| | | | | | | | | | | | | | | | | | | | |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| | | HYDRAULIC VALUES | | | | | | | | | | | | | | | | | |
|--|-------|------------------------------|-------|------------------------|-------|--|-------|-------|-------|-------|-------|-------|-------|-------|-------------|-------|-------|-------|-------|
| | | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | | | | | | | | | |
| | | SPAN PROPERTIES | | | | | | | | | | | | | | | | | |
| 100-yr Water Surface Elevation (ft - MSL) | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | |
| Bed Elevation (ft - MSL) | -2.0 | -1.9 | -2.0 | -2.0 | -2.0 | -2.0 | -2.1 | -2.1 | -2.1 | -2.1 | -2.1 | -2.1 | -2.1 | -2.1 | -2.1 | -2.1 | -2.1 | -2.2 | |
| Low Chord Elevation (ft - MSL) | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 14.7 | 14.6 | 14.8 | 14.8 | 14.8 | 14.8 | 14.8 | 14.8 | 14.8 | 14.8 | 14.8 | 14.8 | 14.8 | 14.8 | 14.8 | 14.8 | 14.8 | 14.9 | 14.9 |
| 100-yr Wave Height (ft) | 7.5 | 7.5 | 7.4 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.6 | 7.6 | 7.6 | 7.6 | 7.6 | 7.6 | 7.6 | 7.6 | 7.6 | 7.6 |
| 100-yr Wave Period (seconds) | 6.0 | 6.1 | 6.0 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 |
| Span Length (ft) | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 |
| Span Width (ft) | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 |
| Deck Thickness (ft) | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 |
| Overhang (ft) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Number of Beams | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 |
| Beam Dead Weight (lb/ft) - Each | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 |
| Beam Dead Weight (lb/ft) - Total | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 |
| Slab Dead Weight (kip/ft) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Total Dead Weight (kip/ft) | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Resisting Moment (kip-ft) | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 |
| Resisting Vertical Force (kip/ft) | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| | | 100-YEAR FORCE-MOMENT VALUES | | | | | | | | | | | | | | | | | |
| Maximum Vertical Force (kips/span) | 11.8 | 12.0 | 10.6 | 12.2 | 12.6 | 12.9 | 13.1 | 13.3 | 13.2 | 13.3 | 13.4 | 12.5 | 13.4 | 12.5 | 13.6 | 13.7 | 13.9 | 14.0 | 14.2 |
| Maximum Vertical Force (kip/ft) | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 |
| Maximum Horizontal Force (kips/span) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Horizontal Force (kip/ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Moment (kip-ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Moment (kip-ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| | | Vulnerability Index Legend | | | | | | | | | | | | | | | | | |
| | | Not Vulnerable | | Potentially Vulnerable | | | | | | | | | | | Description | | | | |
| | | 1 | 1 | 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) | | | | | | | | | | | | | |
| | | 2 | 2 | 2 | 2 | Medium impact if closed - may lead to a barrier island but an alternative route exists | | | | | | | | | | | | | |
| | | 3 | 3 | 3 | 3 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives | | | | | | | | | | | | | |
| | | 4 | 4 | 4 | 4 | Extreme impact if closed - Interstate or major economic connector (detour very long) | | | | | | | | | | | | | |

Notes:

1 - Bridge spans are not potentially subject to wave energy.

2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

LADOTD BRIDGE RECALL NO. 002892
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| | | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | | | | | | | | | |
|-------------------------------------|-------------------|------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| SPAN NUMBER | Criticality Index | 131 | 132 | 133 | 134 | 135 | 136 | 137 | 138 | 139 | 140 | 141 | 142 | 143 | 144 | 145 | 146 | 147 | 148 |
| Criticality INDEX (defined below) | 4 | 4 | 0.1 | 0.1 | 0.0 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 |
| Vulnerability INDEX (defined below) | 0.1 | 0.1 | 0.1 | 0.1 | 0.0 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | |
|--|------|
| 100-yr Water Surface Elevation (ft - MSL) | 9.6 |
| Bed Elevation (ft - MSL) | -2.2 |
| Low Chord Elevation (ft - MSL) | 15.3 |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 14.9 |
| 100-yr Wave Height (ft) | 7.6 |
| 100-yr Wave Period (seconds) | 6.1 |

| SPAN PROPERTIES | |
|-----------------------------------|-------|
| Span Length (ft) | 65.0 |
| Span Width (ft) | 45.8 |
| Deck Thickness (ft) | 0.6 |
| Overhang (ft) | 3.0 |
| Number of Beams | 6 |
| Beam Dead Weight (lb/ft) - Each | 583 |
| Beam Dead Weight (lb/ft) - Total | 3.5 |
| Slab Dead Weight (kip/ft) | 4.0 |
| Total Dead Weight (kip/ft) | 7.5 |
| Resisting Moment (kip-ft) | 148.9 |
| Resisting Vertical Force (kip/ft) | 7.5 |

| 100-YEAR FORCE-MOMENT VALUES | |
|--------------------------------------|------|
| Maximum Vertical Force (kips/span) | 14.3 |
| Maximum Vertical Force (kip/ft) | 0.2 |
| Maximum Horizontal Force (kips/span) | 5.1 |
| Maximum Horizontal Force (kip/ft) | 0.1 |
| Maximum Moment (kip-ft) | 0.0 |
| Maximum Moment (kip-ft) | 0.0 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable |
|----------------------------|----------------|------------------------|
|----------------------------|----------------|------------------------|

| Criticality Index | Multiplier | Description |
|-------------------|------------|--|
| 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

Notes:

1 - Bridge spans are not potentially subject to wave energy.

2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

LADOTD BRIDGE RECALL NO. 002892
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| | | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | | | | | | | | | |
|-------------------------------------|-------------------|------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| SPAN NUMBER | Criticality Index | 149 | 150 | 151 | 152 | 153 | 154 | 155 | 156 | 157 | 158 | 159 | 160 | 161 | 162 | 163 | 164 | 165 | 166 |
| Criticality INDEX (defined below) | | 4 | 4 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 |
| Vulnerability INDEX (defined below) | | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| | | HYDRAULIC VALUES | | | | | | | | | | | | | | | | | | | |
|--|-------|------------------------------|------------------------|------------------------------|-------|-------|-------|-------|-------|-------|-------|-------------|-------|--|-------|-------|-------|-------|--|--|--|
| | | SPAN PROPERTIES | | | | | | | | | | | | | | | | | | | |
| | | 100-YEAR FORCE-MOMENT VALUES | | | | | | | | | | | | | | | | | | | |
| 100-yr Water Surface Elevation (ft - MSL) | 9.6 | 9.6 | 9.6 | 9.6 | 9.7 | 9.6 | 9.7 | 9.7 | 9.7 | 9.7 | 9.7 | 9.7 | 9.7 | 9.7 | 9.7 | 9.7 | 9.7 | 9.7 | | | |
| Bed Elevation (ft - MSL) | -2.4 | -2.4 | -2.5 | -2.4 | -2.4 | -2.4 | -2.4 | -2.4 | -2.4 | -2.4 | -2.4 | -2.4 | -2.3 | -2.3 | -2.3 | -2.4 | -2.4 | -2.3 | | | |
| Low Chord Elevation (ft - MSL) | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | | | |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 15.1 | 15.1 | 15.1 | 15.2 | 15.2 | 15.1 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | | | |
| 100-yr Wave Height (ft) | 7.8 | 7.9 | 7.9 | 7.9 | 7.9 | 7.9 | 7.9 | 7.9 | 7.9 | 7.9 | 7.8 | 7.8 | 7.8 | 7.8 | 7.8 | 7.8 | 7.8 | 7.8 | | | |
| 100-yr Wave Period (seconds) | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | | | |
| Span Length (ft) | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | | | |
| Span Width (ft) | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | | | |
| Deck Thickness (ft) | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | | | |
| Overhang (ft) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | | | |
| Number of Beams | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | | | |
| Beam Dead Weight (lb/ft) - Each | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | | | |
| Beam Dead Weight (lb/ft) - Total | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | | | |
| Slab Dead Weight (kip/ft) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | | | |
| Total Dead Weight (kip/ft) | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | | | |
| Resisting Moment (kip-ft) | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | | | |
| Resisting Vertical Force (kip/ft) | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | | | |
| | | CRITICALITY INDEX LEGEND | | | | | | | | | | DESCRIPTION | | | | | | | | | |
| | | Not Vulnerable | Potentially Vulnerable | CRITICALITY INDEX MULTIPLIER | | | | | | | | | | DESCRIPTION | | | | | | | |
| | | 1 | 1 | 1 | | | | | | | | | | Minor impact to economy or emergency needs if closed (alternative routes exist) | | | | | | | |
| | | 2 | 1 | 2 | | | | | | | | | | Medium impact if closed - may lead to a barrier island but an alternative route exists | | | | | | | |
| | | 3 | 1.75 | 3 | | | | | | | | | | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives | | | | | | | |
| | | 4 | 1.75 | 4 | | | | | | | | | | Extreme impact if closed - Interstate or major economic connector (detour very long) | | | | | | | |

Notes:

1 - Bridge spans are not potentially subject to wave energy.

2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

LADOTD BRIDGE RECALL NO. 002892
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| | | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | | | | | | | | | |
|--|-----------------------------------|------------------------------|------|------|------|------|------|------|------|------|------|------|------|------|------|------|------|------|-----|
| SPAN NUMBER | Criticality Index (defined below) | 167 | 168 | 169 | 170 | 171 | 172 | 173 | 174 | 175 | 176 | 177 | 178 | 179 | 180 | 181 | 182 | 183 | 184 |
| Vulnerability Index (defined below) | | 4 | 4 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 |
| 100-yr Water Surface Elevation (ft - MSL) | 9.7 | 9.7 | 9.7 | 9.7 | 9.7 | 9.7 | 9.7 | 9.7 | 9.7 | 9.7 | 9.7 | 9.7 | 9.7 | 9.7 | 9.7 | 9.7 | 9.7 | 9.7 | |
| Bed Elevation (ft - MSL) | -2.3 | -2.2 | -2.2 | -2.2 | -2.2 | -2.2 | -2.2 | -2.2 | -2.2 | -2.2 | -2.2 | -2.2 | -2.2 | -2.2 | -2.2 | -2.3 | -2.3 | -2.3 | |
| Bed Elevation (ft - MSL) | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 15.2 | 15.1 | 15.1 | 15.1 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | |
| 100-yr Wave Height (ft) | 7.8 | 7.8 | 7.8 | 7.8 | 7.8 | 7.8 | 7.8 | 7.8 | 7.8 | 7.8 | 7.8 | 7.8 | 7.8 | 7.8 | 7.8 | 7.8 | 7.8 | 7.7 | |
| 100-yr Wave Period (seconds) | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| | | HYDRAULIC VALUES | | | | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | | | | | | | |
|--------------------------------------|-----------------------------------|------------------|-------|------------------------|-------|------------------------------|-------|-------|-------|-------------------|-------|-------|-------|--|-------|-------|-------|-------|-------|--|--|
| SPAN NUMBER | Criticality Index (defined below) | 167 | 168 | 169 | 170 | 171 | 172 | 173 | 174 | 175 | 176 | 177 | 178 | 179 | 180 | 181 | 182 | 183 | 184 | | |
| Vulnerability Index (defined below) | | 4 | 4 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | | |
| SPAN PROPERTIES | | | | | | | | | | | | | | | | | | | | | |
| Span Length (ft) | | | | | | | | | | | | | | | | | | | | | |
| Span Width (ft) | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | | |
| Deck Thickness (ft) | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | | |
| Overhang (ft) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | | |
| Number of Beams | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | | |
| Beam Dead Weight (lb/ft) - Each | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | | |
| Beam Dead Weight (lb/ft) - Total | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | | |
| Slab Dead Weight (kip/ft) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | | |
| Total Dead Weight (kip/ft) | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | | |
| Resisting Moment (kip-ft) | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | | |
| Resisting Vertical Force (kip/ft) | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | | |
| 100-YEAR FORCE-MOMENT VALUES | | | | | | | | | | | | | | | | | | | | | |
| Maximum Vertical Force (kips/span) | 17.4 | 17.4 | 17.3 | 17.3 | 17.4 | 17.2 | 17.3 | 17.6 | 17.3 | 17.4 | 17.7 | 18.1 | 18.4 | 18.6 | 19.0 | 18.8 | 18.8 | 17.3 | | | |
| Maximum Vertical Force (kip/sft) | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | | |
| Maximum Horizontal Force (kips/span) | 12.2 | 12.4 | 12.5 | 12.5 | 14.7 | 14.6 | 12.4 | 12.6 | 12.6 | 15.4 | 15.0 | 13.1 | 16.9 | 19.4 | 18.1 | 17.0 | 17.0 | 17.0 | 17.0 | | |
| Maximum Horizontal Force (kip/sft) | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | | |
| Maximum Moment (kip-ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | | |
| Maximum Moment (kip-ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | | |
| Vulnerability Index Legend | | Not Vulnerable | | Potentially Vulnerable | | Criticality Index | | | | Multiplier | | | | Description | | | | | | | |
| | | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) | | | | | | | |
| | | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | Medium impact if closed - may lead to a barrier island but an alternative route exists | | | | | | | |
| | | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives | | | | | | | |
| | | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | Extreme impact if closed - Interstate or major economic connector (detour very long) | | | | | | | |

Notes:

1 - Bridge spans are not potentially subject to wave energy.

2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

LADOTD BRIDGE RECALL NO. 002892
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| BRIDGE VULNERABILITY SUMMARY | | | | | | | | | |
|--|------|------|------|------|------|------|------|------|------|
| SPAN NUMBER | 185 | 186 | 187 | 188 | 189 | 190 | 191 | 192 | 193 |
| Criticality Index (defined below) | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 |
| Vulnerability Index (defined below) | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 |
| 100-yr Water Surface Elevation (ft - MSL) | 9.8 | 9.8 | 9.8 | 9.8 | 9.8 | 9.8 | 9.8 | 9.8 | 9.8 |
| Bed Elevation (ft - MSL) | -2.3 | -2.2 | -2.2 | -2.2 | -2.1 | -2.0 | -2.0 | -2.0 | -1.9 |
| Low Chord Elevation (ft - MSL) | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 15.3 | 15.3 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 |
| 100-yr Wave Height (ft) | 7.8 | 7.8 | 7.8 | 7.8 | 7.7 | 7.7 | 7.7 | 7.7 | 7.6 |
| 100-yr Wave Period (seconds) | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 5.9 | 5.9 | 6.0 | 6.0 |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | | | |
|--|------|------|------|
| 100-yr Water Surface Elevation (ft - MSL) | 9.8 | 9.8 | 9.8 |
| Bed Elevation (ft - MSL) | -2.3 | -2.2 | -2.2 |
| Low Chord Elevation (ft - MSL) | 15.3 | 15.3 | 15.3 |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 15.3 | 15.3 | 15.2 |
| 100-yr Wave Height (ft) | 7.8 | 7.8 | 7.8 |
| 100-yr Wave Period (seconds) | 6.0 | 6.0 | 6.0 |

| SPAN PROPERTIES | | | |
|-----------------------------------|-------|-------|-------|
| Span Length (ft) | 65.0 | 65.0 | 65.0 |
| Span Width (ft) | 45.8 | 45.8 | 45.8 |
| Deck Thickness (ft) | 0.6 | 0.6 | 0.6 |
| Overhang (ft) | 3.0 | 3.0 | 3.0 |
| Number of Beams | 6 | 6 | 6 |
| Beam Dead Weight (lb/ft) - Each | 583 | 583 | 583 |
| Beam Dead Weight (lb/ft) - Total | 3.5 | 3.5 | 3.5 |
| Slab Dead Weight (kip/ft) | 4.0 | 4.0 | 4.0 |
| Total Dead Weight (kip/ft) | 7.5 | 7.5 | 7.5 |
| Resisting Moment (kip-ft) | 148.9 | 148.9 | 148.9 |
| Resisting Vertical Force (kip/ft) | 7.5 | 7.5 | 7.5 |

| 100-YEAR FORCE-MOMENT VALUES | | | |
|--------------------------------------|------|------|------|
| Maximum Vertical Force (kips/span) | 18.9 | 18.9 | 18.6 |
| Maximum Vertical Force (kip/ft) | 0.3 | 0.3 | 0.3 |
| Maximum Horizontal Force (kips/span) | 17.2 | 17.0 | 17.2 |
| Maximum Horizontal Force (kip/ft) | 0.3 | 0.3 | 0.3 |
| Maximum Moment (kip-ft) | 0.0 | 0.0 | 0.0 |
| Maximum Moment (kip-ft) | 0.0 | 0.0 | 0.0 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable | Description |
|----------------------------|----------------|------------------------|--|
| 1 | 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| 2 | 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| 3 | 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| 4 | 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

- Notes:
- 1 - Bridge spans are not potentially subject to wave energy.
 - 2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

Table 11 of 14

LADOTD BRIDGE RECALL NO. 002892
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| SPAN NUMBER | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | |
|-------------------------------------|------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| | 203 | 204 | 205 | 206 | 207 | 208 | 209 | 210 | 211 | 212 |
| Criticality Index (defined below) | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 |
| Vulnerability Index (defined below) | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | | | | | | | | | | | | | | | | | | |
|--|------------|------------------------------|-------|-------------------|-------------|--|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|--|--|--|--|--|--|--|--|--|
| Span Length (ft) | Width (ft) | 203 | 204 | 205 | 206 | 207 | 208 | 209 | 210 | 211 | 212 | 213 | 214 | 215 | 216 | 217 | 218 | 219 | 220 | | | | | | | | | |
| 100-yr Water Surface Elevation (ft - MSL) | 9.9 | 9.9 | 9.9 | 9.9 | 9.9 | 9.9 | 9.9 | 9.9 | 9.9 | 9.9 | 9.9 | 9.9 | 9.9 | 9.9 | 9.9 | 9.9 | 9.9 | 9.9 | 9.9 | | | | | | | | | |
| Bed Elevation (ft - MSL) | -1.7 | -1.7 | -1.6 | -1.6 | -1.6 | -1.6 | -1.6 | -1.6 | -1.6 | -1.7 | -1.7 | -1.6 | -1.6 | -1.6 | -1.6 | -1.6 | -1.6 | -1.6 | -1.5 | | | | | | | | | |
| Low Chord Elevation (ft - MSL) | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | | | | | | | | | |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 15.1 | 15.1 | 15.1 | 15.1 | 15.1 | 15.1 | 15.1 | 15.1 | 15.1 | 15.1 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | | | | | | | | | |
| 100-yr Wave Height (ft) | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | | | | | | | | | |
| 100-yr Wave Period (seconds) | 6.0 | 6.0 | 6.1 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | | | | | | | | | |
| SPAN PROPERTIES | | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Span Length (ft) | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | | | | | | | | | |
| Span Width (ft) | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | | | | | | | | | |
| Deck Thickness (ft) | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | | | | | | | | | |
| Overhang (ft) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | | | | | | | | | |
| Number of Beams | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | | | | | | | | | |
| Beam Dead Weight (lb/ft) - Each | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | | | | | | | | | |
| Beam Dead Weight (lb/ft) - Total | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | | | | | | | | | |
| Slab Dead Weight (kip/ft) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | | | | | | | | | |
| Total Dead Weight (kip/ft) | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | | | | | | | | | |
| Resisting Moment (kip-ft) | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | | | | | | | | | |
| Resisting Vertical Force (kip/ft) | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | | | | | | | | | |
| 100-YEAR FORCE-MOMENT VALUES | | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Maximum Vertical Force (kips/span) | 16.1 | 16.2 | 16.1 | 16.1 | 18.6 | 16.4 | 16.3 | 16.4 | 16.4 | 16.6 | 16.6 | 16.8 | 16.7 | 16.6 | 18.3 | 16.5 | 16.5 | 16.2 | 16.2 | | | | | | | | | |
| Maximum Vertical Force (kip/sft) | 0.2 | 0.2 | 0.2 | 0.2 | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | 0.2 | | | | | | | | | |
| Maximum Horizontal Force (kips/span) | 13.5 | 11.9 | 11.5 | 11.8 | 12.5 | 12.0 | 14.0 | 14.0 | 14.1 | 14.0 | 14.0 | 13.4 | 12.8 | 11.3 | 14.0 | 12.0 | 12.0 | 12.0 | 12.0 | | | | | | | | | |
| Maximum Horizontal Force (kip/sft) | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | | | | | | | | | |
| Maximum Moment (kip-ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | | | | | | | | | |
| Maximum Moment (kip-ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | | | | | | | | | |
| Vulnerability Index Legend | | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | | | | | | | | | | | | | | | | | | |
| Not Vulnerable | | Potentially Vulnerable | | Criticality Index | Multipplier | Description | | | | | | | | | | | | | | | | | | | | | | |
| | | | | 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) | | | | | | | | | | | | | | | | | | | | | | |
| | | | | 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists | | | | | | | | | | | | | | | | | | | | | | |
| | | | | 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives | | | | | | | | | | | | | | | | | | | | | | |
| | | | | 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) | | | | | | | | | | | | | | | | | | | | | | |

Notes:

- 1 - Bridge spans are not potentially subject to wave energy.
- 2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

LADOTD BRIDGE RECALL NO. 002892
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| SPAN NUMBER | 221 | 222 | 223 | 224 | 225 | 226 | 227 | 228 | 229 | 230 | 231 | 232 | 233 | 234 | 235 | 236 | 237 | 238 |
|-------------------------------------|-----------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| | Criticality Index (defined below) | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 |
| Vulnerability Index (defined below) | 0.1 | 0.1 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.3 | 0.5 | |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| BRIDGE VULNERABILITY SUMMARY | | | | | | | | | | | | | | | | | | |
|--|---|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| HYDRAULIC VALUES | | | | | | | | | | | | | | | | | | |
| 100-yr Water Surface Elevation (ft - MSL) | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.1 | 10.1 | 10.1 |
| Bed Elevation (ft - MSL) | -1.5 | -1.3 | -1.3 | -1.2 | -1.2 | -1.1 | -1.0 | -1.0 | -0.9 | -0.9 | -0.8 | -0.8 | -0.7 | -0.7 | -0.6 | -0.6 | -0.5 | -0.5 |
| Low Chord Elevation (ft - MSL) | 15.3 | 15.4 | 15.6 | 15.9 | 16.3 | 16.7 | 16.5 | 16.6 | 16.5 | 16.5 | 16.6 | 16.6 | 16.0 | 15.3 | 14.5 | 13.8 | 13.2 | 13.2 |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 15.1 | 15.1 | 15.1 | 15.1 | 15.1 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 | 14.9 | 14.9 | 14.9 | 14.9 | 14.9 |
| 100-yr Wave Height (ft) | 7.4 | 7.4 | 7.3 | 7.3 | 7.2 | 7.2 | 7.2 | 7.2 | 7.1 | 7.1 | 7.1 | 7.0 | 7.0 | 6.9 | 6.9 | 6.9 | 6.9 | 6.9 |
| 100-yr Wave Period (seconds) | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 |
| SPAN PROPERTIES | | | | | | | | | | | | | | | | | | |
| Span Length (ft) | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 |
| Span Width (ft) | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 |
| Deck Thickness (ft) | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 |
| Overhang (ft) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Number of Beams | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 |
| Beam Dead Weight (lb/ft) - Each | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 |
| Beam Dead Weight (lb/ft) - Total | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 |
| Slab Dead Weight (kip/ft) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Total Dead Weight (kip/ft) | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Resisting Moment (kip-ft) | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 |
| Resisting Vertical Force (kip/ft) | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| 100-YEAR FORCE-MOMENT VALUES | | | | | | | | | | | | | | | | | | |
| Maximum Vertical Force (kips/span) | 15.9 | 15.2 | 13.8 | 11.8 | 9.2 | 6.3 | 4.0 | 4.6 | 4.5 | | | | | 3.7 | 6.5 | 12.1 | 41.6 | 93.2 |
| Maximum Vertical Force (kip/sft) | 0.2 | 0.2 | 0.2 | 0.2 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.2 | 0.2 | 0.6 | 1.4 | 2.1 |
| Maximum Horizontal Force (kips/span) | 11.3 | 8.8 | 9.0 | 9.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 34.2 | 64.4 | 80.9 |
| Maximum Horizontal Force (kip/sft) | 0.2 | 0.1 | 0.1 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.1 | 0.5 | 1.0 | 1.4 | 1.4 |
| Maximum Moment (kip-ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Moment (kip-ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Vulnerability Index Legend | | | | | | | | | | | | | | | | | | |
| Not Vulnerable | | | | | | | | | | | | | | | | | | |
| Potentially Vulnerable | | | | | | | | | | | | | | | | | | |
| Criticality Index Multiplier | | | | | | | | | | | | | | | | | | |
| Criticality Index | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| Description | Minor impact to economy or emergency needs if closed (alternative routes exist) | | | | | | | | | | | | | | | | | |
| 2 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| 3 | 1 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 |
| 4 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 |

Notes:

- 1 - Bridge spans are not potentially subject to wave energy.
- 2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

Criticality Index

Multiplier

Description

LADOTD BRIDGE RECALL NO. 002892
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| SPAN NUMBER | 239 | 240 | 241 | 242 | | | | | | | | |
|-------------------------------------|-----|-----|-----|-----|--|--|--|--|--|--|--|--|
| Criticality Index (defined below) | 4 | 4 | 4 | 4 | | | | | | | | |
| Vulnerability Index (defined below) | 0.6 | 0.8 | 0.9 | 0.8 | | | | | | | | |

| SURGE/WAVE LOAD COMPUTATION INPUT VALUES | |
|--|--|
|--|--|

| HYDRAULIC VALUES | |
|--|------|
| 100-yr Water Surface Elevation (ft - MSL) | 10.1 |
| Bed Elevation (ft - MSL) | -0.5 |
| Low Chord Elevation (ft - MSL) | 12.8 |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 14.9 |
| 100-yr Wave Height (ft) | 6.9 |
| 100-yr Wave Period (seconds) | 6.1 |

| SPAN PROPERTIES | |
|-----------------------------------|-------|
| Span Length (ft) | 65.0 |
| Span Width (ft) | 45.8 |
| Deck Thickness (ft) | 0.6 |
| Overhang (ft) | 3.0 |
| Number of Beams | 6 |
| Beam Dead Weight (lb/ft) - Each | 583 |
| Beam Dead Weight (lb/ft) - Total | 3.5 |
| Slab Dead Weight (kip/ft) | 4.0 |
| Total Dead Weight (kip/ft) | 7.5 |
| Resisting Moment (kip-ft) | 148.9 |
| Resisting Vertical Force (kip/ft) | 7.5 |

| 100-YEAR FORCE-MOMENT VALUES | |
|--------------------------------------|--------|
| Maximum Vertical Force (kips/span) | 177.7 |
| Maximum Vertical Force (kip/ft) | 2.7 |
| Maximum Horizontal Force (kips/span) | 99.8 |
| Maximum Horizontal Force (kip/ft) | 1.5 |
| Maximum Moment (kip-ft) | 2390.4 |
| Maximum Moment (kip-ft) | 35.9 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable |
|----------------------------|----------------|------------------------|
|----------------------------|----------------|------------------------|

| Criticality Index | Multiplier | Description |
|-------------------|------------|--|
| 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

Notes:

1 - Bridge spans are not potentially subject to wave energy.

2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

LADOTD BRIDGE RECALL NO. 002894
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| BRIDGE VULNERABILITY SUMMARY | | | | | | | | | |
|-------------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| SPAN NUMBER | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 |
| Criticality Index (defined below) | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 |
| Vulnerability Index (defined below) | 1.2 | 1.2 | 0.3 | 0.3 | 0.1 | 0.0 | 0.0 | 0.0 | 0.0 |
| | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |

| SURGE/WAVE LOAD COMPUTATION INPUT VALUES | | | | | | | | | |
|--|--|--|--|--|--|--|--|--|--|
|--|--|--|--|--|--|--|--|--|--|

| HYDRAULIC VALUES | | | | | | | | | |
|--|------|------|------|------|------|------|------|------|------|
| 100-yr Water Surface Elevation (ft - MSL) | 9.7 | 9.7 | 9.0 | 9.0 | 9.1 | 9.1 | 9.1 | 9.0 | 9.1 |
| Bad Elevation (ft - MSL) | -2.5 | -2.5 | -0.3 | -0.3 | -0.4 | -0.4 | -0.4 | -0.5 | -0.5 |
| Low Chord Elevation (ft - MSL) | 12.0 | 12.0 | 12.2 | 12.2 | 13.3 | 14.1 | 15.2 | 16.4 | 17.9 |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 15.3 | 15.3 | 13.3 | 13.3 | 13.4 | 13.4 | 13.4 | 13.4 | 13.4 |
| 100-yr Wave Height (ft) | 8.0 | 8.0 | 6.1 | 6.1 | 6.1 | 6.1 | 6.2 | 6.2 | 6.2 |
| 100-yr Wave Period (seconds) | 6.0 | 6.0 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 |

| SPAN PROPERTIES | | | | | | | | | |
|-----------------------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Span Length (ft) | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 |
| Span Width (ft) | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 |
| Deck Thickness (ft) | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 |
| Overhang (ft) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Number of Beams | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 |
| Beam Dead Weight (lb/ft) - Each | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 |
| Beam Dead Weight (kip/ft) - Total | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 |
| Slab Dead Weight (kip/ft) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Total Dead Weight (kip/ft) | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Resisting Moment (kip/ft) | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 |
| Resisting Vertical Force (kip/ft) | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |

| 100-YEAR FORCE-MOMENT VALUES | | | | | | | | | |
|--------------------------------------|--------|--------|--------|-------|------|-----|-----|-----|-----|
| Maximum Vertical Force (kips/span) | 331.1 | 324.3 | 81.2 | 81.3 | 29.3 | 6.5 | 2.0 | 0.4 | 0.0 |
| Maximum Vertical Force (kips/ft) | 5.1 | 5.0 | 1.2 | 1.3 | 0.5 | 0.1 | 0.0 | 0.0 | 0.0 |
| Maximum Horizontal Force (kips/span) | 171.6 | 169.9 | 52.8 | 51.3 | 17.1 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Horizontal Force (kips/ft) | 2.6 | 2.6 | 0.8 | 0.8 | 0.3 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Moment (kip-ft) | 5367.4 | 5198.5 | 1677.2 | 805.2 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Moment (k-ft/ft) | 82.6 | 80.0 | 25.8 | 12.4 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable |
|----------------------------|----------------|------------------------|
|----------------------------|----------------|------------------------|

| Criticality Index | Multiplier | Description |
|-------------------|------------|--|
| 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

Notes:

1 - Bridge spans 5-6 are potentially subject to wave energy.

2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

LADOTD BRIDGE RECALL NO. 002894
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| SPAN NUMBER | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | |
|-------------------------------------|------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| | 23 | 24 | 25 | 26 | 27 | 28 | 29 | 30 | 31 | 32 |
| Criticality Index (defined below) | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 |
| Vulnerability Index (defined below) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | |
|--|------|
| 100-yr Water Surface Elevation (ft - MSL) | 9.1 |
| Bed Elevation (ft - MSL) | -0.7 |
| Low Chord Elevation (ft - MSL) | 30.9 |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 13.6 |
| 100-yr Wave Height (ft) | 6.4 |
| 100-yr Wave Period (seconds) | 6.1 |

| SPAN PROPERTIES | |
|-----------------------------------|-------|
| Span Length (ft) | 65.0 |
| Span Width (ft) | 45.8 |
| Deck Thickness (ft) | 0.6 |
| Overhang (ft) | 3.0 |
| Number of Beams | 6 |
| Beam Dead Weight (lb/ft) - Each | 583 |
| Beam Dead Weight (kip/ft) - Total | 3.5 |
| Slab Dead Weight (kip/ft) | 4.0 |
| Total Dead Weight (kip/ft) | 7.5 |
| Resisting Moment (kip-ft) | 148.9 |
| Resisting Vertical Force (kip/ft) | 7.5 |

| 100-YEAR FORCE-MOMENT VALUES | |
|--------------------------------------|-----|
| Maximum Vertical Force (kips/span) | 0.0 |
| Maximum Vertical Force (kip/ft) | 0.0 |
| Maximum Horizontal Force (kips/span) | 0.0 |
| Maximum Horizontal Force (kip/ft) | 0.0 |
| Maximum Moment (kip-ft) | 0.0 |
| Maximum Moment (kip-ft) | 0.0 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable |
|----------------------------|----------------|------------------------|
|----------------------------|----------------|------------------------|

| Criticality Index | Multiplier | Description |
|-------------------|------------|--|
| 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

Notes:

1 - Bridge spans are not potentially subject to wave energy.

2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

LADOTD BRIDGE RECALL NO. 002894
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| BRIDGE VULNERABILITY SUMMARY | | | | | | | | | |
|--|------|------|------|------|------|------|------|------|------|
| SPAN NUMBER | 41 | 42 | 43 | 44 | 45 | 46 | 47 | 48 | 49 |
| Criticality Index (defined below) | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 |
| Vulnerability Index (defined below) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 100-yr Water Surface Elevation (ft - MSL) | 9.2 | 9.2 | 9.2 | 9.2 | 9.2 | 9.2 | 9.2 | 9.2 | 9.2 |
| Bed Elevation (ft - MSL) | -1.2 | -1.2 | -1.3 | -1.3 | -1.3 | -1.4 | -1.4 | -1.5 | -1.5 |
| Low Chord Elevation (ft - MSL) | 24.0 | 23.3 | 22.6 | 22.1 | 21.4 | 20.8 | 20.3 | 19.4 | 18.6 |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 13.9 | 14.0 | 14.0 | 14.0 | 14.1 | 13.9 | 14.1 | 14.1 | 14.2 |
| 100-yr Wave Height (ft) | 6.8 | 6.8 | 6.8 | 6.8 | 6.9 | 6.7 | 6.9 | 7.0 | 7.0 |
| 100-yr Wave Period (seconds) | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | | | |
|--|------|------|------|
| 100-yr Water Surface Elevation (ft - MSL) | 9.2 | 9.2 | 9.2 |
| Bed Elevation (ft - MSL) | -1.2 | -1.2 | -1.3 |
| Low Chord Elevation (ft - MSL) | 24.0 | 23.3 | 22.6 |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 13.9 | 14.0 | 14.0 |
| 100-yr Wave Height (ft) | 6.8 | 6.8 | 6.8 |
| 100-yr Wave Period (seconds) | 6.1 | 6.1 | 6.1 |

| SPAN PROPERTIES | | | |
|-----------------------------------|-------|-------|-------|
| Span Length (ft) | 65.0 | 65.0 | 65.0 |
| Span Width (ft) | 45.8 | 45.8 | 45.8 |
| Deck Thickness (ft) | 0.6 | 0.6 | 0.6 |
| Overhang (ft) | 3.0 | 3.0 | 3.0 |
| Number of Beams | 6 | 6 | 6 |
| Beam Dead Weight (lb/ft) - Each | 583 | 583 | 583 |
| Beam Dead Weight (lb/ft) - Total | 3.5 | 3.5 | 3.5 |
| Slab Dead Weight (kip/ft) | 4.0 | 4.0 | 4.0 |
| Total Dead Weight (kip/ft) | 7.5 | 7.5 | 7.5 |
| Resisting Moment (kip-ft) | 148.9 | 148.9 | 148.9 |
| Resisting Vertical Force (kip/ft) | 7.5 | 7.5 | 7.5 |

| 100-YEAR FORCE-MOMENT VALUES | | | |
|--------------------------------------|-----|-----|-----|
| Maximum Vertical Force (kips/span) | 0.0 | 0.0 | 0.0 |
| Maximum Vertical Force (kip/ft) | 0.0 | 0.0 | 0.0 |
| Maximum Horizontal Force (kips/span) | 0.0 | 0.0 | 0.0 |
| Maximum Horizontal Force (kip/ft) | 0.0 | 0.0 | 0.0 |
| Maximum Moment (kip-ft) | 0.0 | 0.0 | 0.0 |
| Maximum Moment (kip-ft) | 0.0 | 0.0 | 0.0 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable | Description |
|----------------------------|----------------|------------------------|--|
| 1 | 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| 2 | 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| 3 | 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| 4 | 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

- Notes:
- 1 - Bridge spans are not potentially subject to wave energy.
 - 2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

Table 3 of 14

LADOTD BRIDGE RECALL NO. 002894
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | | 68 | 69 | 70 | 71 | 72 | 73 | 74 | 75 | 76 |
|--|------------------------------|------|------|------|------|------|------|------|------|------|------|------|------|------|------|------|------|------|----|
| SPAN NUMBER | 59 | 60 | 61 | 62 | 63 | 64 | 65 | 66 | 67 | 68 | 69 | 70 | 71 | 72 | 73 | 74 | 75 | 76 | |
| Criticality Index (defined below) | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | |
| Vulnerability Index (defined below) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | |
| 100-yr Water Surface Elevation (ft - MSL) | 9.3 | 9.3 | 9.3 | 9.3 | 9.3 | 9.3 | 9.3 | 9.3 | 9.3 | 9.3 | 9.3 | 9.3 | 9.3 | 9.3 | 9.3 | 9.3 | 9.3 | 9.3 | |
| Bed Elevation (ft - MSL) | -1.7 | -1.7 | -1.8 | -1.8 | -1.8 | -1.8 | -1.8 | -1.8 | -1.8 | -1.9 | -1.9 | -1.9 | -2.0 | -2.0 | -2.0 | -2.0 | -2.0 | -2.0 | |
| Low Chord Elevation (ft - MSL) | 15.4 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 14.3 | 14.3 | 14.3 | 14.3 | 14.3 | 14.4 | 14.4 | 14.4 | 14.4 | 14.4 | 14.4 | 14.4 | 14.5 | 14.5 | 14.5 | 14.5 | 14.5 | 14.5 | |
| 100-yr Wave Height (ft) | 7.1 | 7.2 | 7.2 | 7.2 | 7.2 | 7.2 | 7.2 | 7.2 | 7.2 | 7.1 | 7.3 | 7.3 | 7.4 | 7.4 | 7.4 | 7.4 | 7.4 | 7.4 | |
| 100-yr Wave Period (seconds) | 6.1 | 6.1 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.1 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | | |
|--|------|------|
| 100-yr Water Surface Elevation (ft - MSL) | 9.3 | 9.3 |
| Bed Elevation (ft - MSL) | -1.7 | -1.7 |
| Low Chord Elevation (ft - MSL) | 15.4 | 15.3 |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 14.3 | 14.3 |
| 100-yr Wave Height (ft) | 7.1 | 7.2 |
| 100-yr Wave Period (seconds) | 6.1 | 6.1 |

| SPAN PROPERTIES | | |
|-----------------------------------|-------|-------|
| Span Length (ft) | 65.0 | 65.0 |
| Span Width (ft) | 45.8 | 45.8 |
| Deck Thickness (ft) | 0.6 | 0.6 |
| Overhang (ft) | 3.0 | 3.0 |
| Number of Beams | 6 | 6 |
| Beam Dead Weight (lb/ft) - Each | 583 | 583 |
| Beam Dead Weight (lb/ft) - Total | 3.5 | 3.5 |
| Slab Dead Weight (kip/ft) | 4.0 | 4.0 |
| Total Dead Weight (kip/ft) | 7.5 | 7.5 |
| Resisting Moment (kip-ft) | 148.9 | 148.9 |
| Resisting Vertical Force (kip/ft) | 7.5 | 7.5 |

| 100-YEAR FORCE-MOMENT VALUES | | |
|--------------------------------------|-----|-----|
| Maximum Vertical Force (kips/span) | 6.8 | 7.3 |
| Maximum Vertical Force (kip/ft) | 0.1 | 0.1 |
| Maximum Horizontal Force (kips/span) | 0.0 | 0.0 |
| Maximum Horizontal Force (kip/ft) | 0.0 | 0.0 |
| Maximum Moment (kip-ft) | 0.0 | 0.0 |
| Maximum Moment (kip-ft) | 0.0 | 0.0 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable |
|----------------------------|----------------|------------------------|
| | 1 | 1 |

| Criticality Index | Multiplier | Description |
|-------------------|------------|--|
| 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

Notes:

1 - Bridge spans are not potentially subject to wave energy.

2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

LADOTD BRIDGE RECALL NO. 002894
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| SPAN NUMBER | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | |
|-------------------------------------|------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| | 77 | 78 | 79 | 80 | 81 | 82 | 83 | 84 | 85 | 86 |
| Criticality Index (defined below) | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 |
| Vulnerability Index (defined below) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | | | | | | | | | | |
|--|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| 100-yr Water Surface Elevation (ft - MSL) | 9.3 | 9.4 | 9.4 | 9.4 | 9.4 | 9.4 | 9.4 | 9.4 | 9.4 | 9.4 |
| Bed Elevation (ft - MSL) | -1.9 | -2.0 | -2.0 | -2.0 | -2.0 | -2.1 | -2.1 | -2.1 | -2.0 | -2.0 |
| Low Chord Elevation (ft - MSL) | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 14.4 | 14.5 | 14.6 | 14.6 | 14.6 | 14.6 | 14.6 | 14.6 | 14.6 | 14.6 |
| 100-yr Wave Height (ft) | 7.3 | 7.4 | 7.4 | 7.4 | 7.4 | 7.4 | 7.4 | 7.4 | 7.4 | 7.4 |
| 100-yr Wave Period (seconds) | 6.0 | 6.0 | 6.1 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 |
| SPAN PROPERTIES | | | | | | | | | | |
| Span Length (ft) | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 |
| Span Width (ft) | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 |
| Deck Thickness (ft) | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 |
| Overhang (ft) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Number of Beams | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 |
| Beam Dead Weight (lb/ft) - Each | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 |
| Beam Dead Weight (kip/ft) - Total | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 |
| Slab Dead Weight (kip/ft) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Total Dead Weight (kip/ft) | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Resisting Moment (kip-ft) | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 |
| Resisting Vertical Force (kip/ft) | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |

| 100-YEAR FORCE-MOMENT VALUES | | | | | | | | | | |
|--------------------------------------|-----|-----|------|------|------|------|------|------|------|------|
| Maximum Vertical Force (kips/span) | 8.6 | 9.8 | 10.0 | 10.1 | 10.2 | 10.2 | 10.3 | 10.3 | 10.3 | 10.4 |
| Maximum Vertical Force (kip/ft) | 0.1 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 |
| Maximum Horizontal Force (kips/span) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Horizontal Force (kip/ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Moment (kip-ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Moment (kip/ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable |
|----------------------------|----------------|------------------------|
|----------------------------|----------------|------------------------|

| Criticality Index | Multiplier | Description |
|-------------------|------------|--|
| 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

Notes:

1 - Bridge spans are not potentially subject to wave energy.

2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

LADOTD BRIDGE RECALL NO. 002894
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| BRIDGE VULNERABILITY SUMMARY | | | | | | | | | |
|-------------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| SPAN NUMBER | 95 | 96 | 97 | 98 | 99 | 100 | 101 | 102 | 103 |
| Criticality Index (defined below) | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 |
| Vulnerability Index (defined below) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| | | | | | | | | | |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | | | | | | | | | |
|--|----------------|------------------------|-------------------|------------|--|-------|-------|-------|-------|
| 100-yr Water Surface Elevation (ft - MSL) | 9.4 | 9.4 | 9.4 | 9.4 | 9.4 | 9.4 | 9.4 | 9.4 | 9.5 |
| Bed Elevation (ft - MSL) | -2.0 | -2.0 | -2.0 | -2.0 | -2.0 | -2.0 | -2.0 | -2.1 | -2.1 |
| Low Chord Elevation (ft - MSL) | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 14.6 | 14.6 | 14.6 | 14.6 | 14.6 | 14.7 | 14.7 | 14.7 | 14.7 |
| 100-yr Max Wave Height (ft) | 7.4 | 7.4 | 7.4 | 7.4 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| 100-yr Wave Period (seconds) | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 |
| SPAN PROPERTIES | | | | | | | | | |
| Span Length (ft) | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 |
| Span Width (ft) | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 |
| Deck Thickness (ft) | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 |
| Overhang (ft) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Number of Beams | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 |
| Beam Dead Weight (lb/ft) - Each | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 |
| Beam Dead Weight (lb/ft) - Total | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 |
| Slab Dead Weight (kip/ft) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Total Dead Weight (kip/ft) | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Resisting Moment (kip-ft) | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 |
| Resisting Vertical Force (kip/ft) | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| 100-YEAR FORCE-MOMENT VALUES | | | | | | | | | |
| Maximum Vertical Force (kips/span) | 10.6 | 10.7 | 10.5 | 10.7 | 10.9 | 11.1 | 11.2 | 11.1 | 11.3 |
| Maximum Vertical Force (kip/sft) | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 |
| Maximum Horizontal Force (kips/span) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Horizontal Force (kip/sft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Moment (k-ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Moment (kip-ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable | Criticality Index | Multiplier | Description | | | | |
| | | | 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) | | | | |
| | | | 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists | | | | |
| | | | 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives | | | | |
| | | | 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) | | | | |

Notes:

1 - Bridge spans are not potentially subject to wave energy.

2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

LADOTD BRIDGE RECALL NO. 002894
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| | | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | | | | | | | | | |
|-------------------------------------|-----------------------------------|------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| SPAN NUMBER | Criticality Index (defined below) | 113 | 114 | 115 | 116 | 117 | 118 | 119 | 120 | 121 | 122 | 123 | 124 | 125 | 126 | 127 | 128 | 129 | 130 |
| Vulnerability Index (defined below) | | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 |
| 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.1 | 0.0 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| | | HYDRAULIC VALUES | | | | | | | | | | | | | | | | |
|--|-------|----------------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| | | SPAN PROPERTIES | | | | | | | | | | | | | | | | |
| | | 100-YR FORCE-MOMENT VALUES | | | | | | | | | | | | | | | | |
| 100-yr Water Surface Elevation (ft - MSL) | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 |
| Bed Elevation (ft - MSL) | -2.3 | -2.3 | -2.3 | -2.2 | -2.3 | -2.3 | -2.3 | -2.4 | -2.4 | -2.4 | -2.4 | -2.4 | -2.4 | -2.4 | -2.4 | -2.4 | -2.4 | -2.4 |
| Low Chord Elevation (ft - MSL) | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 14.8 | 14.8 | 14.9 | 14.9 | 14.9 | 14.9 | 14.9 | 14.9 | 14.9 | 14.9 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 |
| 100-yr Wave Height (ft) | 7.6 | 7.6 | 7.7 | 7.6 | 7.7 | 7.7 | 7.7 | 7.7 | 7.7 | 7.7 | 7.8 | 7.8 | 7.8 | 7.8 | 7.8 | 7.8 | 7.8 | 7.8 |
| 100-yr Wave Period (seconds) | 6.1 | 6.1 | 6.1 | 6.0 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 |
| Span Length (ft) | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 |
| Span Width (ft) | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 |
| Deck Thickness (ft) | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 |
| Overhang (ft) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Number of Beams | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 |
| Beam Dead Weight (lb/ft) - Each | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 |
| Beam Dead Weight (lb/ft) - Total | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 |
| Slab Dead Weight (kip/ft) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Total Dead Weight (kip/ft) | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Resisting Moment (kip-ft) | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 |
| Resisting Vertical Force (kip/ft) | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Maximum Vertical Force (kips/span) | 13.3 | 13.6 | 13.7 | 13.9 | 12.1 | 14.1 | 14.6 | 14.9 | 15.2 | 15.3 | 15.4 | 15.5 | 14.5 | 15.6 | 15.6 | 15.6 | 16.0 | 16.1 |
| Maximum Horizontal Force (kip/ft) | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 |
| Maximum Horizontal Force (kips/span) | 1.8 | 3.9 | 3.3 | 3.7 | 3.0 | 3.7 | 4.3 | 5.8 | 6.4 | 8.1 | 8.1 | 7.9 | 6.0 | 7.1 | 8.0 | 8.1 | 8.0 | 8.1 |
| Maximum Moment (kip-ft) | 0.0 | 0.1 | 0.1 | 0.1 | 0.0 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 |
| Maximum Moment (kip-ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable |
|----------------------------|----------------|------------------------|
|----------------------------|----------------|------------------------|

| Criticality Index | Multiplier | Description |
|-------------------|------------|--|
| 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

Notes:

1 - Bridge spans are not potentially subject to wave energy.

2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

LADOTD BRIDGE RECALL NO. 002894
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| SPAN NUMBER | 131 | | 132 | | 133 | | 134 | | 135 | | 136 | | 137 | | 138 | | 139 | | 140 | | 141 | | 142 | | 143 | | 144 | | 145 | | 146 | | 147 | | 148 | |
|-------------------------------------|-----------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|---|-----|--|-----|--|
| | Criticality Index (defined below) | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | | | | |
| Vulnerability Index (defined below) | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | | | | | |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| BRIDGE VULNERABILITY SUMMARY | | | |
|--|-------|----------------------------|-----|
| HYDRAULIC VALUES | | | |
| 100-yr Water Surface Elevation (ft - MSL) | 9.5 | 9.6 | |
| Bed Elevation (ft - MSL) | -2.5 | -2.5 | |
| Low Chord Elevation (ft - MSL) | 15.3 | 15.3 | |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 15.0 | 15.0 | |
| 100-yr Wave Height (ft) | 7.8 | 7.8 | |
| 100-yr Wave Period (seconds) | 6.1 | 6.1 | |
| SPAN PROPERTIES | | | |
| Span Length (ft) | 65.0 | 65.0 | |
| Span Width (ft) | 45.8 | 45.8 | |
| Deck Thickness (ft) | 0.6 | 0.6 | |
| Overhang (ft) | 3.0 | 3.0 | |
| Number of Beams | 6 | 6 | |
| Beam Dead Weight (lb/ft) - Each | 583 | 583 | |
| Beam Dead Weight (lb/ft) - Total | 3.5 | 3.5 | |
| Slab Dead Weight (kip/ft) | 4.0 | 4.0 | |
| Total Dead Weight (kip/ft) | 7.5 | 7.5 | |
| Resisting Moment (kip-ft) | 148.9 | 148.9 | |
| Resisting Vertical Force (kip/ft) | 7.5 | 7.5 | |
| 100-YEAR FORCE-MOMENT VALUES | | | |
| Maximum Vertical Force (kips/span) | 16.2 | 16.3 | |
| Maximum Vertical Force (kip/ft) | 0.2 | 0.3 | |
| Maximum Horizontal Force (kips/span) | 7.2 | 9.1 | |
| Maximum Horizontal Force (kip/ft) | 0.1 | 0.2 | |
| Maximum Moment (kip-ft) | 0.0 | 0.0 | |
| Maximum Moment (kip-ft) | 0.0 | 0.0 | |
| Vulnerability Index Legend | | | |
| Not Vulnerable | Green | Not Potentially Vulnerable | Red |

- Notes:
- 1 - Bridge spans are not potentially subject to wave energy.
 - 2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

| Vulnerability Index | Not Vulnerable | Potentially Vulnerable |
|---------------------|----------------|------------------------|
|---------------------|----------------|------------------------|

| Criticality Index | Multiplier | Description |
|-------------------|------------|--|
| 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

LADOTD BRIDGE RECALL NO. 002894
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| | | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | | | | | | | | | |
|-------------------------------------|-----------------------------------|------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| SPAN NUMBER | Criticality Index (defined below) | 149 | 150 | 151 | 152 | 153 | 154 | 155 | 156 | 157 | 158 | 159 | 160 | 161 | 162 | 163 | 164 | 165 | 166 |
| VULNERABILITY INDEX (defined below) | | 4 | 4 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 |
| | | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | |
|--|------|
| 100-yr Water Surface Elevation (ft - MSL) | 9.6 |
| Bed Elevation (ft - MSL) | -2.7 |
| Low Chord Elevation (ft - MSL) | 15.3 |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 15.2 |
| 100-yr Wave Height (ft) | 8.0 |
| 100-yr Wave Period (seconds) | 6.1 |

| SPAN PROPERTIES | |
|-----------------------------------|-------|
| Span Length (ft) | 65.0 |
| Span Width (ft) | 45.8 |
| Deck Thickness (ft) | 0.6 |
| Overhang (ft) | 3.0 |
| Number of Beams | 6 |
| Beam Dead Weight (lb/ft) - Each | 583 |
| Beam Dead Weight (lb/ft) - Total | 3.5 |
| Slab Dead Weight (kip/ft) | 4.0 |
| Total Dead Weight (kip/ft) | 7.5 |
| Resisting Moment (kip-ft) | 148.9 |
| Resisting Vertical Force (kip/ft) | 7.5 |

| 100-YEAR FORCE-MOMENT VALUES | |
|--------------------------------------|------|
| Maximum Vertical Force (kips/span) | 19.3 |
| Maximum Vertical Force (kip/ft) | 0.3 |
| Maximum Horizontal Force (kips/span) | 17.5 |
| Maximum Horizontal Force (kip/ft) | 0.3 |
| Maximum Moment (kip-ft) | 0.0 |
| Maximum Moment (kip-ft) | 0.0 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable |
|----------------------------|----------------|------------------------|
|----------------------------|----------------|------------------------|

| Criticality Index | Multiplier | Description |
|-------------------|------------|--|
| 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

- Notes:
- 1 - Bridge spans are not potentially subject to wave energy.
 - 2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

Table 9 of 14

LADOTD BRIDGE RECALL NO. 002894
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| | | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | | | | | | | | | |
|-------------------------------------|-----------------------------------|------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| SPAN NUMBER | Criticality Index (defined below) | 167 | 168 | 169 | 170 | 171 | 172 | 173 | 174 | 175 | 176 | 177 | 178 | 179 | 180 | 181 | 182 | 183 | 184 |
| Vulnerability Index (defined below) | | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 |
| 1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| | | HYDRAULIC VALUES | | | | | | | | | | SPAN PROPERTIES | | | | | | | | | |
|---|---|---|----------------------------------|--------------------------------------|------------------------------------|-------------------------|---------------------------------|-----------------------------------|---------------------------|----------------------------|---------------------------|---|--|--|--|-------------------|--|-----------------------------------|---------------------------|----------------------------|---------------------------|
| | | 100-yr Water Surface Elevation (ft - MSL) | | | | | Bed Elevation (ft - MSL) | | | | | Low Chord Elevation (ft - MSL) | | | | | 100-yr Max Wave Crest Elevation (ft - MSL) | | | | |
| | | Span Length (ft) | Span Width (ft) | Deck Thickness (ft) | Overhang (ft) | Number of Beams | Beam Dead Weight (lb/ft) - Each | Beam Dead Weight (kip/ft) - Total | Slab Dead Weight (kip/ft) | Total Dead Weight (kip/ft) | Resisting Moment (kip-ft) | Span Length (ft) | Span Width (ft) | Deck Thickness (ft) | Overhang (ft) | Number of Beams | Beam Dead Weight (lb/ft) - Each | Beam Dead Weight (kip/ft) - Total | Slab Dead Weight (kip/ft) | Total Dead Weight (kip/ft) | Resisting Moment (kip-ft) |
| | | 97.0 | 97.0 | 97.0 | 97.0 | 6 | 583 | 583 | 4.0 | 148.9 | 148.9 | 65.0 | 45.8 | 0.6 | 6 | 583 | 583 | 4.0 | 148.9 | 148.9 | |
| 1 | 1 | 97.0 | 97.0 | 97.0 | 97.0 | 6 | 583 | 583 | 4.0 | 148.9 | 148.9 | 65.0 | 45.8 | 0.6 | 6 | 583 | 583 | 4.0 | 148.9 | 148.9 | |
| 2 | 2 | 97.0 | 97.0 | 97.0 | 97.0 | 6 | 583 | 583 | 4.0 | 148.9 | 148.9 | 65.0 | 45.8 | 0.6 | 6 | 583 | 583 | 4.0 | 148.9 | 148.9 | |
| 3 | 3 | 97.0 | 97.0 | 97.0 | 97.0 | 6 | 583 | 583 | 4.0 | 148.9 | 148.9 | 65.0 | 45.8 | 0.6 | 6 | 583 | 583 | 4.0 | 148.9 | 148.9 | |
| 4 | 4 | 97.0 | 97.0 | 97.0 | 97.0 | 6 | 583 | 583 | 4.0 | 148.9 | 148.9 | 65.0 | 45.8 | 0.6 | 6 | 583 | 583 | 4.0 | 148.9 | 148.9 | |
| | | 100-YEAR FORCE-MOMENT VALUES | | | | | | | | | | Description | | | | | | | | | |
| | | Maximum Vertical Force (kips/span) | Maximum Vertical Force (kip/sft) | Maximum Horizontal Force (kips/span) | Maximum Horizontal Force (kip/sft) | Maximum Moment (kip-ft) | Maximum Moment (kip-ft) | Maximum Moment (kip-ft) | Maximum Moment (kip-ft) | Maximum Moment (kip-ft) | Maximum Moment (kip-ft) | 1 | 2 | 3 | 4 | Criticality Index | Multiplier | Not Vulnerable | | Potentially Vulnerable | |
| 1 | 1 | 20.0 | 0.3 | 19.8 | 0.3 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | Minor impact to economy or emergency needs if closed (alternative routes exist) | Medium impact if closed - may lead to a barrier island but an alternative route exists | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives | Extreme impact if closed - Interstate or major economic connector (detour very long) | 1 | 1 | 1 | 1 | 1 | 1 |
| 2 | 2 | 20.0 | 0.3 | 20.0 | 0.3 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | Minor impact to economy or emergency needs if closed (alternative routes exist) | Medium impact if closed - may lead to a barrier island but an alternative route exists | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives | Extreme impact if closed - Interstate or major economic connector (detour very long) | 2 | 1 | 1 | 1 | 1 | 1 |
| 3 | 3 | 20.0 | 0.3 | 20.0 | 0.3 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | Minor impact to economy or emergency needs if closed (alternative routes exist) | Medium impact if closed - may lead to a barrier island but an alternative route exists | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives | Extreme impact if closed - Interstate or major economic connector (detour very long) | 3 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 |
| 4 | 4 | 20.0 | 0.3 | 20.0 | 0.3 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | Minor impact to economy or emergency needs if closed (alternative routes exist) | Medium impact if closed - may lead to a barrier island but an alternative route exists | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives | Extreme impact if closed - Interstate or major economic connector (detour very long) | 4 | 1.75 | 1.75 | 1.75 | 1.75 | 1.75 |

Notes:

1 - Bridge spans are not potentially subject to wave energy.

2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

LADOTD BRIDGE RECALL NO. 002894
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| BRIDGE VULNERABILITY SUMMARY | | | | | | | | | |
|-------------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| SPAN NUMBER | 185 | 186 | 187 | 188 | 189 | 190 | 191 | 192 | 193 |
| Criticality Index (defined below) | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 |
| Vulnerability Index (defined below) | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 |
| | | | | | | | | | |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | |
|--|------|
| 100-yr Water Surface Elevation (ft - MSL) | 9.8 |
| Bed Elevation (ft - MSL) | -2.6 |
| Low Chord Elevation (ft - MSL) | 15.3 |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 15.4 |
| 100-yr Wave Height (ft) | 8.0 |
| 100-yr Wave Period (seconds) | 6.0 |

| SPAN PROPERTIES | |
|-----------------------------------|-------|
| Span Length (ft) | 65.0 |
| Span Width (ft) | 45.8 |
| Deck Thickness (ft) | 0.6 |
| Overhang (ft) | 3.0 |
| Number of Beams | 6 |
| Beam Dead Weight (lb/ft) - Each | 583 |
| Beam Dead Weight (lb/ft) - Total | 3.5 |
| Slab Dead Weight (kip/ft) | 4.0 |
| Total Dead Weight (kip/ft) | 7.5 |
| Resisting Moment (kip-ft) | 148.9 |
| Resisting Vertical Force (kip/ft) | 7.5 |

| 100-YEAR FORCE-MOMENT VALUES | | | | | | | | | |
|--------------------------------------|------|------|------|------|------|------|------|------|------|
| Maximum Vertical Force (kips/span) | 21.9 | 28.1 | 21.6 | 29.9 | 35.1 | 20.8 | 20.6 | 35.5 | 22.0 |
| Maximum Vertical Force (kip/ft) | 0.3 | 0.4 | 0.3 | 0.3 | 0.5 | 0.3 | 0.3 | 0.5 | 0.3 |
| Maximum Horizontal Force (kips/span) | 22.0 | 25.5 | 22.6 | 22.8 | 24.9 | 21.9 | 21.7 | 22.8 | 21.7 |
| Maximum Horizontal Force (kip/ft) | 0.3 | 0.4 | 0.3 | 0.4 | 0.4 | 0.3 | 0.3 | 0.4 | 0.3 |
| Maximum Moment (kip-ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Moment (kip-ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable |
|----------------------------|----------------|------------------------|
|----------------------------|----------------|------------------------|

| Criticality Index | Multiplier | Description |
|-------------------|------------|--|
| 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

Notes:
 1 - Bridge spans are not potentially subject to wave energy.
 2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

LADOTD BRIDGE RECALL NO. 002894
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| SPAN NUMBER | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | |
|-------------------------------------|------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| | 203 | 204 | 205 | 206 | 207 | 208 | 209 | 210 | 211 | 212 |
| Criticality Index (defined below) | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 |
| Vulnerability Index (defined below) | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | | | |
|--|--|------------------------------|------------------------|--|-------|-------|-------|-------|-------|-------|-------|--|--|
| (100-yr Water Surface Elevation (ft - MSL) | | 9.8 | 9.8 | 9.9 | 9.9 | 9.9 | 9.9 | 9.9 | 9.9 | 9.9 | 9.9 | | |
| Bed Elevation (ft - MSL) | | -1.9 | -2.1 | -1.9 | -1.9 | -1.9 | -1.9 | -1.9 | -1.9 | -1.9 | -1.8 | | |
| Low Chord Elevation (ft - MSL) | | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | | |
| (100-yr) Max Wave Crest Elevation (ft - MSL) | | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | | |
| (100-yr) Max Wave Height (ft) | | 7.7 | 7.8 | 7.7 | 7.6 | 7.6 | 7.7 | 7.7 | 7.7 | 7.7 | 7.6 | | |
| (100-yr) Wave Period (seconds) | | 6.0 | 6.0 | 6.0 | 6.1 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.1 | | |
| SPAN PROPERTIES | | | | | | | | | | | | | |
| Span Length (ft) | | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | | |
| Span Width (ft) | | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | | |
| Deck Thickness (ft) | | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | | |
| Overhang (ft) | | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | | |
| Number of Beams | | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | | |
| Beam Dead Weight (lb/ft) - Each | | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | | |
| Beam Dead Weight (lb/ft) - Total | | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | | |
| Slab Dead Weight (kip/ft) | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | | |
| Total Dead Weight (kip/ft) | | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | | |
| Resisting Moment (kip-ft) | | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | | |
| Resisting Vertical Force (kip/ft) | | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | | |
| (100-YEAR FORCE-MOMENT VALUES | | | | | | | | | | | | | |
| Maximum Vertical Force (kips/span) | | 17.8 | 18.8 | 17.8 | 26.1 | 17.8 | 17.9 | 18.1 | 18.2 | 18.4 | 18.5 | | |
| Maximum Vertical Force (kip/sft) | | 0.3 | 0.3 | 0.3 | 0.4 | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | | |
| Maximum Horizontal Force (kips/span) | | 14.3 | 14.3 | 14.5 | 16.5 | 14.5 | 14.4 | 13.0 | 11.7 | 16.7 | 16.5 | | |
| Maximum Horizontal Force (kip/sft) | | 0.2 | 0.3 | 0.2 | 0.3 | 0.2 | 0.2 | 0.3 | 0.2 | 0.3 | 0.2 | | |
| Maximum Moment (k-ft) | | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | | |
| Maximum Moment (kip-ft) | | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | | |
| Vulnerability Index Legend | | Not Vulnerable | Potentially Vulnerable | Description | | | | | | | | | |
| | | 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) | | | | | | | | | |
| | | 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists | | | | | | | | | |
| | | 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives | | | | | | | | | |
| | | 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) | | | | | | | | | |

Notes:

1 - Bridge spans are not potentially subject to wave energy.

2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

Table 12 of 14

LADOTD BRIDGE RECALL NO. 002894
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| SPAN NUMBER | 221 | 222 | 223 | 224 | 225 | 226 | 227 | 228 | 229 | 230 | 231 | 232 | 233 | 234 | 235 | 236 | 237 | 238 |
|-------------------------------------|-----------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| | Criticality Index (defined below) | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 |
| Vulnerability Index (defined below) | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| BRIDGE VULNERABILITY SUMMARY | | | | | | | | | | | | | | | | | | | |
|--|--|----------------|-------|------------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|--|
| HYDRAULIC VALUES | | | | | | | | | | | | | | | | | | | |
| 100-yr Water Surface Elevation (ft - MSL) | 9.9 | 9.9 | 9.9 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.1 | 10.1 | 10.1 | |
| Bed Elevation (ft - MSL) | -1.7 | -1.7 | -1.6 | -1.6 | -1.5 | -1.4 | -1.3 | -1.2 | -1.1 | -1.1 | -1.1 | -1.0 | -0.9 | -0.9 | -0.8 | -0.8 | -0.7 | -0.7 | |
| Low Chord Elevation (ft - MSL) | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.3 | 15.4 | 15.5 | 15.6 | 15.9 | 16.3 | 16.6 | 16.5 | 16.6 | 16.5 | 16.5 | 16.5 | 16.6 | |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 15.2 | 15.2 | 15.2 | 15.2 | 15.2 | 15.1 | 15.1 | 15.1 | 15.1 | 15.1 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 | 15.0 | |
| 100-yr Wave Height (ft) | 7.6 | 7.5 | 7.5 | 7.4 | 7.4 | 7.4 | 7.3 | 7.3 | 7.3 | 7.2 | 7.2 | 7.2 | 7.1 | 7.1 | 7.0 | 7.0 | 7.0 | 7.0 | |
| 100-yr Wave Period (seconds) | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | 6.1 | |
| SPAN PROPERTIES | | | | | | | | | | | | | | | | | | | |
| Span Length (ft) | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | 65.0 | |
| Span Width (ft) | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | 45.8 | |
| Deck Thickness (ft) | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | |
| Overhang (ft) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | |
| Number of Beams | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | |
| Beam Dead Weight (lb/ft) - Each | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | |
| Beam Dead Weight (kip/ft) - Total | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | |
| Slab Dead Weight (kip/ft) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | |
| Total Dead Weight (kip/ft) | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | |
| Resisting Moment (kip-ft) | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | 148.9 | |
| Resisting Vertical Force (kip/ft) | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | |
| 100-YEAR FORCE-MOMENT VALUES | | | | | | | | | | | | | | | | | | | |
| Maximum Vertical Force (kips/span) | 17.9 | 17.6 | 17.3 | 16.9 | 16.4 | 15.7 | 14.8 | 13.6 | 11.5 | 8.8 | 6.2 | 4.3 | 4.5 | 4.4 | 4.4 | 4.4 | 4.4 | 4.4 | |
| Maximum Vertical Force (kip/sft) | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | 0.3 | 0.2 | 0.2 | 0.2 | 0.2 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | |
| Maximum Horizontal Force (kips/span) | 15.5 | 12.8 | 13.8 | 13.9 | 12.9 | 11.4 | 9.6 | 7.5 | 1.7 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | |
| Maximum Horizontal Force (kip/sft) | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.1 | 0.1 | 0.1 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | |
| Maximum Moment (kip-ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | |
| Maximum Moment (kip-ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | |
| Vulnerability Index Legend | | Not Vulnerable | | Potentially Vulnerable | | | | | | | | | | | | | | | |
| Criticality Index | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | |
| Description | Minor impact to economy or emergency needs if closed (alternative routes exist) | | | | | | | | | | | | | | | | | | |
| Notes: | <p>1 - Bridge spans are not potentially subject to wave energy.</p> <p>2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)</p> | | | | | | | | | | | | | | | | | | |
| | Extreme impact if closed - Interstate or major economic connector (detour very long) | | | | | | | | | | | | | | | | | | |

Notes:
 1 - Bridge spans are not potentially subject to wave energy.
 2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

Table 13 of 14

LADOTD BRIDGE RECALL NO. 002894
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| | | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | |
|-------------------------------------|-----------------------------------|------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| SPAN NUMBER | Criticality Index (defined below) | 239 | 240 | 241 | 242 | 243 | 244 | 245 | 246 | 247 | |
| VULNERABILITY INDEX (defined below) | | 4 | 4 | 0.0 | 0.1 | 0.3 | 0.5 | 0.7 | 0.7 | 0.9 | 0.8 |
| | | | | | | | | | | | |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | |
|------------------------------------|--------------------------------------|------------------------------|--------------------------------|--|-------------------------|------------------------------|-------------------|------------|--|--------|---|
| Span Length (ft) | Span Width (ft) | Bed Elevation (ft - MSL) | Low Chord Elevation (ft - MSL) | 100-yr Max Wave Crest Elevation (ft - MSL) | 100-yr Wave Height (ft) | 100-yr Wave Period (seconds) | Criticality Index | Multiplier | Description | | |
| 65.0 | 45.8 | 65.0 | 45.8 | 14.9 | 7.0 | 6.1 | 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) | | |
| 65.0 | 45.8 | 65.0 | 45.8 | 14.9 | 6.9 | 6.1 | 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists | | |
| 65.0 | 45.8 | 65.0 | 45.8 | 14.9 | 6.9 | 6.1 | 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives | | |
| 65.0 | 45.8 | 65.0 | 45.8 | 14.9 | 6.9 | 6.1 | 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) | | |
| | | | | | | | | | | | |
| SPAN PROPERTIES | | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | |
| Number of Beams | Beam Dead Weight (lb/ft) - Each | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | | |
| Beam Dead Weight (lb/ft) - Total | Slab Dead Weight (kip/ft) | 583 | 583 | 583 | 583 | 583 | 583 | 583 | 583 | | |
| Total Dead Weight (kip/ft) | Resisting Moment (kip-ft) | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | | |
| Resisting Moment (kip-ft) | Resisting Vertical Force (kip/ft) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | | |
| Resisting Vertical Force (kip/ft) | | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | | |
| | | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | | |
| 100-YEAR FORCE-MOMENT VALUES | | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | |
| Maximum Vertical Force (kips/span) | Maximum Horizontal Force (kips/span) | 6.3 | 11.8 | 41.2 | 84.0 | 131.1 | 184.0 | 208.7 | 244.1 | 223.1 | |
| Maximum Vertical Force (kips/ft) | Maximum Horizontal Force (kips/ft) | 0.1 | 0.2 | 0.6 | 1.3 | 2.0 | 2.8 | 3.2 | 3.8 | 3.4 | |
| Maximum Moment (k-ft) | Maximum Moment (k-ft) | 0.0 | 0.3 | 30.5 | 60.4 | 83.0 | 104.7 | 106.7 | 130.0 | 110.9 | |
| Maximum Moment (k-ft) | Maximum Moment (k-ft) | 0.0 | 0.1 | 0.5 | 0.9 | 1.3 | 1.6 | 1.6 | 2.0 | 1.7 | |
| Maximum Moment (k-ft) | Maximum Moment (k-ft) | 0.0 | 0.0 | 71.0 | 885.8 | 1302.0 | 2626.0 | 3245.5 | 3741.0 | 3715.1 | |
| Maximum Moment (k-ft) | Maximum Moment (k-ft) | 0.0 | 0.0 | 1.1 | 13.6 | 20.0 | 40.4 | 49.9 | 57.6 | 57.2 | |
| | | | | | | | | | | | |
| Vulnerability Index Legend | | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | |
| Not Vulnerable | Potentially Vulnerable | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |

- Notes:
- 1 - Bridge spans are not potentially subject to wave energy.
 - 2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

Table 14 of 14

LADOTD BRIDGE RECALL NO. 003432
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| SPAN NUMBER | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 | 17 |
|-------------------------------------|------|----|-----|-----|-----|------|------|-----|------|-----|-----|-----|-----|-----|-----|------|----|
| Criticality Index (defined below) | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 |
| Vulnerability Index (defined below) | 22.4 | 22 | 3.4 | 3.0 | 2.9 | 18.5 | 17.4 | 7.0 | 21.9 | 3.3 | 2.3 | 2.5 | 2.7 | 3.2 | 2.4 | 31.4 | |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| BRIDGE VULNERABILITY SUMMARY | | | | | | | | | | | | | | | | | |
|--|------|------|------|------|------|------|------|------|------|------|------|------|------|------|------|------|------|
| HYDRAULIC VALUES | | | | | | | | | | | | | | | | | |
| [100-yr] Water Surface Elevation (ft - MSL) | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 |
| Bad Elevation (ft - MSL) | 5 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 |
| Low Chord Elevation (ft - MSL) | 8.1 | 8.8 | 9.5 | 10.2 | 10.9 | 11.6 | 12.3 | 13.0 | 10.4 | 10.4 | 10.5 | 12.1 | 11.4 | 10.7 | 10.0 | 9.3 | 8.6 |
| [100-yr] Max Wave Crest Elevation (ft - MSL) | 16.9 | 16.9 | 16.9 | 16.9 | 16.9 | 16.9 | 16.9 | 16.9 | 16.9 | 16.9 | 16.9 | 17.5 | 17.0 | 16.8 | 16.8 | 16.6 | 16.4 |
| [100-yr] Wave Height (ft) | 10.5 | 10.5 | 10.5 | 10.5 | 10.5 | 10.5 | 10.5 | 10.5 | 10.5 | 10.5 | 10.5 | 11.4 | 10.7 | 10.5 | 10.4 | 10.2 | 9.9 |
| [100-yr] Wave Period (seconds) | 4.8 | 4.8 | 4.8 | 4.8 | 4.8 | 4.8 | 4.8 | 4.8 | 4.8 | 4.8 | 4.8 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 |

| SPAN PROPERTIES | | | | | | | | | | | | | | | | | | |
|---|---------|--------|--------|--------|--------|---------|---------|--------|---------|------|--------|--------|--------|--------|--------|--------|---------|--|
| Span Length (ft) | | | | | | | | | | | | | | | | | | |
| Span Width (ft) | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | 30.0 | |
| Deck Thickness (ft) | 1.1 | 1.1 | 1.1 | 1.1 | 1.1 | 1.1 | 1.1 | 1.1 | 1.1 | 1.1 | 1.1 | 1.1 | 1.1 | 1.1 | 1.1 | 1.1 | 1.1 | |
| Overhang (ft) | | | | | | | | | | | | | | | | | | |
| Number of Beams | | | | | | | | | | | | | | | | | | |
| Beam Dead Weight (lb/ft) - Each | | | | | | | | | | | | | | | | | | |
| Beam Dead Weight (kip/ft) - Total | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | |
| Slab Dead Weight (kip/ft) | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | |
| Total Dead Weight (kip/ft) | 73.1 | 73.1 | 73.1 | 73.1 | 73.1 | 73.1 | 73.1 | 73.1 | 73.1 | 73.1 | 73.1 | 73.1 | 73.1 | 73.1 | 73.1 | 73.1 | 73.1 | |
| Resisting Moment (kip/ft) | | | | | | | | | | | | | | | | | | |
| Resisting Vertical Force (kip/ft) | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | 4.9 | |
| 100-YEAR FORCE-MOMENT VALUES | | | | | | | | | | | | | | | | | | |
| Maximum Vertical Force (kip/ft/span) | 527.7 | 121.9 | 192.1 | 168.2 | 163.0 | 741.1 | 970.8 | 170.5 | 585.9 | | 181.1 | 129.5 | 138.9 | 151.4 | 176.8 | 132.6 | 741.2 | |
| Maximum Vertical Force (kips/ft) | 26.4 | 6.1 | 9.6 | 8.4 | 8.1 | 37.1 | 48.5 | 8.5 | 29.3 | | 9.1 | 6.5 | 6.9 | 7.6 | 8.8 | 6.6 | 37.1 | |
| Maximum Horizontal Force (kips/ft/span) | 12.7 | 8.5 | 16.7 | 17.6 | 40.7 | 76.2 | 47.9 | 28.3 | | 31.5 | 28.0 | 6.4 | 7.0 | 7.6 | 8.1 | 32.2 | | |
| Maximum Horizontal Force (kips/ft) | 0.6 | 0.4 | 0.8 | 0.8 | 2.0 | 3.8 | 2.4 | 1.4 | | 1.6 | 1.4 | 0.3 | 0.4 | 0.4 | 1.6 | | | |
| Maximum Moment (kip-ft) | 18748.5 | 1629.0 | 1688.3 | 1564.9 | 2167.2 | 15431.9 | 12644.5 | 5846.1 | 18283.3 | | 1901.3 | 1846.2 | 1508.8 | 1562.1 | 1664.3 | 1643.5 | 26236.5 | |
| Maximum Moment (k-ft/ft) | 937.4 | 81.5 | 84.4 | 78.2 | 108.4 | 771.6 | 632.2 | 292.3 | 914.2 | | 95.1 | 92.3 | 75.4 | 78.1 | 83.2 | 82.2 | 1311.8 | |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable |
|----------------------------|----------------|------------------------|
|----------------------------|----------------|------------------------|

Notes:

1 - Bridge spans 1-9, 11-17 are potentially subject to wave energy.

2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

| Criticality Index | Multiplier | Description |
|-------------------|------------|--|
| 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

Table 1 of 1

LADOTD BRIDGE RECALL NO. 003440
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| SPAN NUMBER | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 |
|-------------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| Criticality Index (defined below) | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 |
| Vulnerability Index (defined below) | 0.8 | 1.3 | 1.1 | 1.2 | 1.3 | 1.2 | 1.1 | 1.3 | 1.1 |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| BRIDGE VULNERABILITY SUMMARY | | | | | | | | | |
|--|------|------|------|------|------|------|------|------|------|
| HYDRAULIC VALUES | | | | | | | | | |
| [100-yr] Water Surface Elevation (ft - MSL) | 8.1 | 8.1 | 8.1 | 8.1 | 8.1 | 8.1 | 8.1 | 8.1 | 8.1 |
| Bad Elevation (ft - MSL) | -4 | -17 | -27 | -31 | -33 | -32 | -28 | -18 | -6 |
| Low Chord Elevation (ft - MSL) | 10.2 | 10.4 | 10.6 | 10.5 | 10.5 | 10.5 | 10.6 | 10.4 | 10.2 |
| [100-yr] Max Wave Crest Elevation (ft - MSL) | 13.6 | 14.8 | 14.8 | 14.8 | 14.8 | 14.8 | 14.8 | 14.8 | 14.6 |
| [100-yr] Wave Height (ft) | 7.8 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.5 | 9.4 | 9.2 |
| [100-yr] Wave Period (seconds) | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.4 | 4.4 |

| SPAN PROPERTIES | | | | | | | | | |
|-----------------------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Span Length (ft) | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 |
| Span Width (ft) | 44.0 | 44.0 | 44.0 | 44.0 | 44.0 | 44.0 | 44.0 | 44.0 | 44.0 |
| Deck Thickness (ft) | 1.1 | 1.1 | 1.1 | 1.1 | 1.1 | 1.1 | 1.1 | 1.1 | 1.1 |
| Overhang (ft) | | | | | | | | | |
| Number of Beams | | | | | | | | | |
| Beam Dead Weight (lb/ft) - Each | | | | | | | | | |
| Beam Dead Weight (kip/ft) - Total | 7.1 | 7.1 | 7.1 | 7.1 | 7.1 | 7.1 | 7.1 | 7.1 | 7.1 |
| Slab Dead Weight (kip/ft) | 7.1 | 7.1 | 7.1 | 7.1 | 7.1 | 7.1 | 7.1 | 7.1 | 7.1 |
| Total Dead Weight (kip/ft) | 157.3 | 157.3 | 157.3 | 157.3 | 157.3 | 157.3 | 157.3 | 157.3 | 157.3 |
| Resisting Moment (kip/ft) | 7.1 | 7.1 | 7.1 | 7.1 | 7.1 | 7.1 | 7.1 | 7.1 | 7.1 |
| Resisting Vertical Force (kip/ft) | | | | | | | | | |

| 100-YEAR FORCE-MOMENT VALUES | | | | | | | | | |
|--------------------------------------|---------|---------|---------|---------|---------|---------|---------|---------|---------|
| Maximum Vertical Force (kips/span) | 69.4 | 109.3 | 91.8 | 98.3 | 181.4 | 96.7 | 93.9 | 103.4 | 89.3 |
| Maximum Vertical Force (kips/ft) | 3.5 | 5.5 | 4.6 | 4.9 | 5.2 | 4.8 | 4.7 | 5.2 | 4.5 |
| Maximum Horizontal Force (kips/span) | 21.1 | 15.9 | 9.9 | 10.5 | 12.4 | 9.9 | 8.3 | 14.2 | 24.7 |
| Maximum Horizontal Force (kips/ft) | 1.1 | 0.8 | 0.5 | 0.5 | 0.4 | 0.5 | 0.4 | 0.7 | 1.2 |
| Maximum Moment (kip-ft) | 1,219.3 | 2,042.5 | 1,428.6 | 1,426.4 | 2,677.4 | 1,376.4 | 1,441.1 | 1,680.3 | 1,732.7 |
| Maximum Moment (kip-ft/ft) | 61.0 | 102.1 | 71.4 | 71.3 | 76.5 | 68.8 | 72.1 | 84.0 | 86.6 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable |
|----------------------------|----------------|------------------------|
|----------------------------|----------------|------------------------|

Notes:

1 - Bridge spans 2-9 are potentially subject to wave energy.

2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

Description

Minor impact to economy or emergency needs if closed (alternative routes exist)

Medium impact if closed - may lead to a barrier island but an alternative route exists

Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives

Extreme impact if closed - Interstate or major economic connector (detour very long)

LADOTD BRIDGE RECALL NO. 003450
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| SPAN NUMBER | 1 | 2 | 3 | 4 | 5 | 6 |
|-------------------------------------|-----|----|-----|---|-----|-----|
| Criticality Index (defined below) | 3 | 3 | 3 | 3 | 3 | |
| Vulnerability Index (defined below) | 1.5 | 12 | 2.7 | | 2.7 | 1.2 |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | |
|--|------|
| [100-yr] Water Surface Elevation (ft - MSL) | 9.3 |
| Bed Elevation (ft - MSL) | 8 |
| Low Chord Elevation (ft - MSL) | 10.7 |
| [100-yr] Max Wave Crest Elevation (ft - MSL) | 14.0 |
| [100-yr] Wave Height (ft) | 8.6 |
| [100-yr] Wave Period (seconds) | 4.8 |

| SPAN PROPERTIES | |
|-----------------------------------|------|
| Span Length (ft) | 20.0 |
| Span Width (ft) | 27.0 |
| Deck Thickness (ft) | 1.0 |
| Overhang (ft) | |
| Number of Beams | |
| Beam Dead Weight (lb/ft) - Each | |
| Beam Dead Weight (kip/ft) - Total | |
| Slab Dead Weight (kip/ft) | 4.1 |
| Total Dead Weight (kip/ft) | 4.1 |
| Resisting Moment (kip/ft) | 54.7 |
| Resisting Vertical Force (kip/ft) | 4.1 |

| 100-YEAR FORCE-MOMENT VALUES | |
|--------------------------------------|-------|
| Maximum Vertical Force (kips/span) | 68.1 |
| Maximum Vertical Force (kips/ft) | 3.4 |
| Maximum Horizontal Force (kips/span) | 9.4 |
| Maximum Horizontal Force (kips/ft) | 0.5 |
| Maximum Moment (kip-ft) | 849.6 |
| Maximum Moment (kip-ft) | 42.5 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable |
|----------------------------|----------------|------------------------|
|----------------------------|----------------|------------------------|

Notes:

1 - Bridge spans 1-3 and 5-6 are potentially subject to wave energy.

2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

| Criticality Index | Multipplier | Description |
|-------------------|-------------|--|
| 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

Table 1 of 1

LADOTD BRIDGE RECALL NO. 003480
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| SPAN NUMBER | 1 | 2 | 3 | 4 | 5 |
|-------------------------------------|-----|-----|-----|-----|-----|
| Criticality Index (defined below) | 3 | 3 | 3 | 3 | 3 |
| Vulnerability Index (defined below) | 0.7 | 1.2 | 1.3 | 1.1 | 0.9 |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | |
|--|-----|
| [100-yr Water Surface Elevation (ft - MSL)] | 8.1 |
| Bad Elevation (ft - MSL) | 4 |
| Low Chord Elevation (ft - MSL) | 7.6 |
| [100-yr Max Wave Crest Elevation (ft - MSL)] | 9.7 |
| [100-yr Wave Height (ft)] | 3.3 |
| [100-yr Wave Period (seconds)] | 2.2 |

| SPAN PROPERTIES | |
|-----------------------------------|------|
| Span Length (ft) | 20.0 |
| Span Width (ft) | 27.0 |
| Deck Thickness (ft) | 1.1 |
| Overhang (ft) | 2.4 |
| Number of Beams | 4 |
| Beam Dead Weight (lb/ft) - Each | 129 |
| Beam Dead Weight (kip/ft) - Total | 0.5 |
| Slab Dead Weight (kip/ft) | 4.4 |
| Total Dead Weight (kip/ft) | 4.4 |
| Resisting Moment (kip/ft) | 59.2 |
| Resisting Vertical Force (kip/ft) | 4.4 |

| 100-YEAR FORCE-MOMENT VALUES | |
|--------------------------------------|-------|
| Maximum Vertical Force (kip/ft/span) | 30.2 |
| Maximum Vertical Force (kip/ft) | 1.5 |
| Maximum Horizontal Force (kips/ft) | 1.2 |
| Maximum Horizontal Force (kips/ft) | 0.1 |
| Maximum Moment (kip-ft) | 463.5 |
| Maximum Moment (kip-ft) | 23.2 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable | Criticality Index | Multiplier | Description |
|----------------------------|----------------|------------------------|-------------------|------------|--|
| | | | 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| | | | 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| | | | 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| | | | 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

- Notes:
- 1 - Bridge spans 2-4 are potentially subject to wave energy.
 - 2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

LADOTD BRIDGE RECALL NO. 003510
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| SPAN NUMBER | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 |
|-------------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|
| Criticality Index (defined below) | 3 | 3 | 3 | 3 | 3 | 3 | 3 | |
| Vulnerability Index (defined below) | 0.0 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.0 | 0.0 |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| BRIDGE VULNERABILITY SUMMARY | | | | | | | | |
|--|------|------|------|------|------|------|------|------|
| HYDRAULIC VALUES | | | | | | | | |
| [100-yr Water Surface Elevation (ft - MSL)] | 8.9 | 8.9 | 8.9 | 8.9 | 8.9 | 8.9 | 8.9 | 8.9 |
| Bed Elevation (ft - MSL) | 6 | -2 | -7 | -7 | -6 | -2 | 3 | 6 |
| Low Chord Elevation (ft - MSL) | 14.0 | 14.1 | 14.3 | 14.4 | 14.4 | 14.4 | 14.3 | 14.0 |
| [100-yr Max Wave Crest Elevation (ft - MSL)] | 13.5 | 13.9 | 14.7 | 14.7 | 14.7 | 14.1 | 13.5 | 13.4 |
| [100-yr Wave Height (ft)] | 6.5 | 7.1 | 8.2 | 8.3 | 8.3 | 7.4 | 6.5 | 6.4 |
| [100-yr Wave Period (seconds)] | 4.4 | 4.4 | 4.4 | 4.4 | 4.4 | 4.4 | 4.4 | 4.4 |

| SPAN PROPERTIES | | | | | | | | |
|-----------------------------------|------|------|------|------|------|------|------|------|
| Span Length (ft) | | | | | | | | |
| Span Width (ft) | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 |
| Deck Thickness (ft) | 27.7 | 27.7 | 27.7 | 27.7 | 27.7 | 27.7 | 27.7 | 27.7 |
| Overhang (ft) | 1.1 | 1.1 | 1.1 | 1.1 | 1.1 | 1.1 | 1.1 | 1.1 |
| Number of Beams | | | | | | | | |
| Beam Dead Weight (lb/ft) - Each | | | | | | | | |
| Beam Dead Weight (kip/ft) - Total | | | | | | | | |
| Slab Dead Weight (kip/ft) | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 |
| Total Dead Weight (kip/ft) | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 |
| Resisting Moment (kip/ft) | 62.3 | 62.3 | 62.3 | 62.3 | 62.3 | 62.3 | 62.3 | 62.3 |
| Resisting Vertical Force (kip/ft) | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 |

| 100-YEAR FORCE-MOMENT VALUES | | | | | | | | |
|------------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|
| Maximum Vertical Force (kips/span) | | | | | | | | |
| Maximum Vertical Force (kips/ft) | 2.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 |
| Maximum Horizontal Force (kips/ft) | 0.1 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 |
| Maximum Horizontal Force (kips/ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Moment (kip-ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Moment (kip-ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable | Criticality Index | Multiplier | Description |
|----------------------------|----------------|------------------------|-------------------|------------|--|
| | | | 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| | | | 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| | | | 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| | | | 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

Notes:

1 - Bridge spans are not potentially subject to wave energy.

2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

LADOTD BRIDGE RECALL NO. 003520
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| SPAN NUMBER | 1 | 2 | 3 | | | | | | | | | |
|-------------------------------------|-----|-----|-----|--|--|--|--|--|--|--|--|--|
| Criticality Index (defined below) | 3 | 3 | 3 | | | | | | | | | |
| Vulnerability Index (defined below) | 2.0 | 2.0 | 2.0 | | | | | | | | | |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | |
|--|------|
| [100-yr Water Surface Elevation (ft - MSL)] | 9.2 |
| Bed Elevation (ft - MSL) | 2 |
| Low Chord Elevation (ft - MSL) | 5.7 |
| [100-yr Max Wave Crest Elevation (ft - MSL)] | 14.7 |
| [100-yr Wave Height (ft)] | 7.9 |
| [100-yr Wave Period (seconds)] | 4.3 |

| SPAN PROPERTIES | |
|-----------------------------------|------|
| Span Length (ft) | 20.0 |
| Span Width (ft) | 27.0 |
| Deck Thickness (ft) | 1.1 |
| Overhang (ft) | 1.1 |
| Number of Beams | |
| Beam Dead Weight (lb/ft) - Each | |
| Beam Dead Weight (kip/ft) - Total | |
| Slab Dead Weight (kip/ft) | 4.4 |
| Total Dead Weight (kip/ft) | 4.4 |
| Resisting Moment (kip/ft) | 59.2 |
| Resisting Vertical Force (kip/ft) | 4.4 |

| 100-YEAR FORCE-MOMENT VALUES | |
|--------------------------------------|--------|
| Maximum Vertical Force (kips/span) | 69.6 |
| Maximum Vertical Force (kips/ft) | 3.5 |
| Maximum Horizontal Force (kips/span) | 6.6 |
| Maximum Horizontal Force (kips/ft) | 0.3 |
| Maximum Moment (kip-ft) | 1352.6 |
| Maximum Moment (k-ft) | 67.6 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable | Criticality Index | Multiplier | Description |
|----------------------------|----------------|------------------------|-------------------|------------|--|
| | | | 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| | | | 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| | | | 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| | | | 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

Notes:
 1 - Bridge spans 1-3 are potentially subject to wave energy.

2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

LADOTD BRIDGE RECALL NO. 003690
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| SPAN NUMBER | 1 | 2 | 3 | 4 | 5 |
|-------------------------------------|------|------|------|------|------|
| Criticality Index (defined below) | 3 | 3 | 3 | 3 | 3 |
| Vulnerability Index (defined below) | 14.1 | 13.4 | 13.4 | 14.9 | 13.6 |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | |
|--|------|
| [100-yr] Water Surface Elevation (ft - MSL) | 9.5 |
| Bed Elevation (ft - MSL) | 1 |
| Low Chord Elevation (ft - MSL) | 1.8 |
| [100-yr] Max Wave Crest Elevation (ft - MSL) | 16.8 |
| [100-yr] Wave Height (ft) | 10.4 |
| [100-yr] Wave Period (seconds) | 4.9 |

| SPAN PROPERTIES | |
|-----------------------------------|------|
| Span Length (ft) | 18.5 |
| Span Width (ft) | 26.8 |
| Deck Thickness (ft) | 1.1 |
| Overhang (ft) | 0.8 |
| Number of Beams | 18 |
| Beam Dead Weight (lb/ft) - Each | 13 |
| Beam Dead Weight (kip/ft) - Total | 0.2 |
| Slab Dead Weight (kip/ft) | 0.7 |
| Total Dead Weight (kip/ft) | 0.9 |
| Resisting Moment (kip/ft) | 11.4 |
| Resisting Vertical Force (kip/ft) | 0.9 |

| 100-YEAR FORCE-MOMENT VALUES | |
|--------------------------------------|--------|
| Maximum Vertical Force (kip/ft/span) | 105.0 |
| Maximum Vertical Force (kip/ft) | 5.7 |
| Maximum Horizontal Force (kips/ft) | 16.7 |
| Maximum Horizontal Force (kips/ft) | 0.9 |
| Maximum Moment (kip-ft) | 1702.2 |
| Maximum Moment (kip-ft) | 92.0 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable |
|----------------------------|----------------|------------------------|
|----------------------------|----------------|------------------------|

Notes:

1 - Bridge spans 1-5 are potentially subject to wave energy.

2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

| Criticality Index | Multipplier | Description |
|-------------------|-------------|--|
| 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

LADOTD BRIDGE RECALL NO. 009030
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| SPAN NUMBER | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 |
|-------------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|
| Criticality Index (defined below) | 3 | 3 | 3 | 3 | 3 | 3 | 3 | |
| Vulnerability Index (defined below) | 7.9 | 8.0 | 7.9 | 7.9 | 8.0 | 8.0 | 8.0 | 8.0 |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | |
|--|-----|
| [100-yr] Water Surface Elevation (ft - MSL) | 6.6 |
| Bed Elevation (ft - MSL) | 7 |
| Low Chord Elevation (ft - MSL) | 4.9 |
| [100-yr] Max Wave Crest Elevation (ft - MSL) | 8.6 |
| [100-yr] Wave Height (ft) | 3.8 |
| [100-yr] Wave Period (seconds) | 4.3 |

| SPAN PROPERTIES | |
|-----------------------------------|------|
| Span Length (ft) | 19.0 |
| Span Width (ft) | 25.2 |
| Deck Thickness (ft) | 1.1 |
| Overhang (ft) | |
| Number of Beams | 18 |
| Beam Dead Weight (lb/ft) - Each | 19 |
| Beam Dead Weight (kip/ft) - Total | 0.4 |
| Slab Dead Weight (kip/ft) | 0.6 |
| Total Dead Weight (kip/ft) | 1.0 |
| Resisting Moment (kip/ft) | 12.4 |
| Resisting Vertical Force (kip/ft) | 1.0 |

| 100-YEAR FORCE-MOMENT VALUES | |
|--------------------------------------|--------|
| Maximum Vertical Force (kips/span) | 69.4 |
| Maximum Vertical Force (kips/ft) | 3.7 |
| Maximum Horizontal Force (kips/span) | 8.0 |
| Maximum Horizontal Force (kips/ft) | 0.4 |
| Maximum Moment (kip-ft) | 1066.8 |
| Maximum Moment (k-ft) | 56.1 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable |
|----------------------------|----------------|------------------------|
|----------------------------|----------------|------------------------|

Notes:

- 1 - Bridge spans 1-8 are potentially subject to wave energy.
- 2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

| Criticality Index | Multipplier | Description |
|-------------------|-------------|--|
| 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

LADOTD BRIDGE RECALL NO. 031755
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| SPAN NUMBER | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 |
|-------------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| Criticality Index (defined below) | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 |
| Vulnerability Index (defined below) | 1.9 | 0.5 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| BRIDGE VULNERABILITY SUMMARY | | | | | | | | | | | | | | | | | | |
|--|-----|-----|-----|------|------|------|------|------|------|------|------|------|------|------|------|------|-------|-------|
| HYDRAULIC VALUES | | | | | | | | | | | | | | | | | | |
| 100-yr Water Surface Elevation (ft - MSL) | 5.8 | 5.8 | 5.8 | 5.8 | 5.8 | 5.8 | 5.8 | 5.8 | 5.8 | 5.8 | 5.8 | 5.8 | 5.8 | 5.8 | 5.8 | 5.8 | 5.8 | 5.8 |
| Bad Elevation (ft - MSL) | 1.3 | 0.8 | 0.3 | -0.3 | -0.8 | -1.4 | -1.9 | -2.3 | -3.0 | -3.7 | -4.4 | -5.1 | -5.9 | -6.6 | -6.6 | -7.4 | -12.0 | -13.4 |
| Low Chord Elevation (ft - MSL) | 3.9 | 6.4 | 7.6 | 9.1 | 10.8 | 12.5 | 14.1 | 15.4 | 16.7 | 17.7 | 18.6 | 19.4 | 20.0 | 20.5 | 20.8 | 21.0 | 20.8 | 20.5 |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 7.5 | 7.6 | 7.6 | 7.7 | 7.7 | 7.7 | 7.8 | 7.8 | 7.9 | 8.0 | 8.0 | 8.0 | 8.1 | 8.1 | 8.1 | 8.2 | 8.2 | 8.2 |
| 100-yr Wave Height (ft) | 2.5 | 2.5 | 2.7 | 2.8 | 3.0 | 3.1 | 3.2 | 3.2 | 3.4 | 3.6 | 3.6 | 3.8 | 3.8 | 3.8 | 3.8 | 3.8 | 3.8 | 3.8 |
| 100-yr Wave Period (seconds) | 3.7 | 3.3 | 3.6 | 4.2 | 4.5 | 4.7 | 4.6 | 4.6 | 5.2 | 2.4 | 2.3 | 2.3 | 2.2 | 2.2 | 2.3 | 2.3 | 2.4 | 3.2 |

| SPAN PROPERTIES | | | | | | | | | | | | | | | | | | |
|-----------------------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Span Length (ft) | | | | | | | | | | | | | | | | | | |
| Span Length (ft) | 50.0 | 50.0 | 50.0 | 50.0 | 50.0 | 50.0 | 50.0 | 50.0 | 50.0 | 50.0 | 50.0 | 50.0 | 50.0 | 50.0 | 50.0 | 50.0 | 50.0 | 50.0 |
| Span Width (ft) | 43.3 | 43.3 | 43.3 | 43.3 | 43.3 | 43.3 | 43.3 | 43.3 | 43.3 | 43.3 | 43.3 | 43.3 | 43.3 | 43.3 | 43.3 | 43.3 | 43.3 | 43.3 |
| Deck Thickness (ft) | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 |
| Overhang (ft) | 3.9 | 3.9 | 3.9 | 3.9 | 3.9 | 3.9 | 3.9 | 3.9 | 3.9 | 3.9 | 3.9 | 3.9 | 3.9 | 3.9 | 3.9 | 3.9 | 3.9 | 3.9 |
| Number of Beams | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 |
| Beam Dead Weight (lb/ft) - Each | 384 | 384 | 384 | 384 | 384 | 384 | 384 | 384 | 384 | 384 | 384 | 384 | 384 | 384 | 384 | 384 | 384 | 384 |
| Beam Dead Weight (kip/ft) - Total | 1.9 | 1.9 | 1.9 | 1.9 | 1.9 | 1.9 | 1.9 | 1.9 | 1.9 | 1.9 | 1.9 | 1.9 | 1.9 | 1.9 | 1.9 | 1.9 | 1.9 | 1.9 |
| Slab Dead Weight (kip/ft) | 4.1 | 4.1 | 4.1 | 4.1 | 4.1 | 4.1 | 4.1 | 4.1 | 4.1 | 4.1 | 4.1 | 4.1 | 4.1 | 4.1 | 4.1 | 4.1 | 4.1 | 4.1 |
| Total Dead Weight (kip/ft) | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 |
| Resisting Moment (kip-ft) | 106.2 | 106.2 | 106.2 | 106.2 | 106.2 | 106.2 | 106.2 | 106.2 | 106.2 | 106.2 | 106.2 | 106.2 | 106.2 | 106.2 | 106.2 | 106.2 | 106.2 | 106.2 |
| Resisting Vertical Force (kip/ft) | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 |

| 100-YEAR FORCE-MOMENT VALUES | | | | | | | | | | | | | | | | | | |
|--------------------------------------|--------|--------|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| Maximum Vertical Force (kips/span) | | | | | | | | | | | | | | | | | | |
| Maximum Vertical Force (kips/ft) | 4.9 | 1.2 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Horizontal Force (kips/span) | 21.9 | 8.8 | 0.2 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Horizontal Force (kips/ft) | 0.4 | 0.2 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Moment (kip-ft) | 5755.1 | 1418.3 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Moment (k-ft) | 115.1 | 28.4 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable |
|----------------------------|----------------|------------------------|
|----------------------------|----------------|------------------------|

Notes:

1 - Bridge span 1 is potentially subject to wave energy.

2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

| Criticality Index | Multiplier | Description |
|-------------------|------------|--|
| 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

Table 1 of 2

LADOTD BRIDGE RECALL NO. 031755
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| SPAN NUMBER | BRIDGE VULNERABILITY SUMMARY | | | | | | | | | |
|-------------------------------------|------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|--|
| | 19 | 20 | 21 | 22 | 23 | 24 | 25 | 26 | 27 | |
| Criticality Index (defined below) | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | |
| Vulnerability Index (defined below) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | |
|--|------|
| 100-yr Water Surface Elevation (ft - MSL) | 5.8 |
| Bed Elevation (ft - MSL) | -9.3 |
| Low Chord Elevation (ft - MSL) | 20.0 |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 8.2 |
| 100-yr Wave Height (ft) | 3.4 |
| 100-yr Wave Period (seconds) | 3.3 |

SPAN PROPERTIES

| | | | | | | | | | | |
|-----------------------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Span Length (ft) | 50.0 | 50.0 | 50.0 | 50.0 | 50.0 | 50.0 | 50.0 | 50.0 | 50.0 | 50.0 |
| Span Width (ft) | 43.3 | 43.3 | 43.3 | 43.3 | 43.3 | 43.3 | 43.3 | 43.3 | 43.3 | 43.3 |
| Deck Thickness (ft) | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 |
| Overhang (ft) | 3.9 | 3.9 | 3.9 | 3.9 | 3.9 | 3.9 | 3.9 | 3.9 | 3.9 | 3.9 |
| Number of Beams | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 |
| Beam Dead Weight (lb/ft) - Each | 384 | 384 | 384 | 384 | 384 | 384 | 384 | 384 | 384 | 384 |
| Beam Dead Weight (kip/ft) - Total | 1.9 | 1.9 | 1.9 | 1.9 | 1.9 | 1.9 | 1.9 | 1.9 | 1.9 | 1.9 |
| Slab Dead Weight (kip/ft) | 4.1 | 4.1 | 4.1 | 4.1 | 4.1 | 4.1 | 4.1 | 4.1 | 4.1 | 4.1 |
| Total Dead Weight (kip/ft) | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 |
| Resisting Moment (kft/ft) | 106.2 | 106.2 | 106.2 | 106.2 | 106.2 | 106.2 | 106.2 | 106.2 | 106.2 | 106.2 |
| Resisting Vertical Force (kip/ft) | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 |

100-YEAR FORCE-MOMENT VALUES

| | | | | | | | | | | |
|--------------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| Maximum Vertical Force (kips/span) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Vertical Force (kip/ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Horizontal Force (kips/span) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Horizontal Force (kip/ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Moment (k-ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Maximum Moment (kip-ft) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable |
|----------------------------|----------------|------------------------|
|----------------------------|----------------|------------------------|

| Criticality Index | Multiplier | Description |
|-------------------|------------|--|
| 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

Notes:

- 1 - Bridge spans are not potentially subject to wave energy.
- 2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

LADOTD BRIDGE RECALL NO. 033698
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| BRIDGE VULNERABILITY SUMMARY | | |
|-------------------------------------|-----|-----|
| SPAN NUMBER | 1 | 2 |
| Criticality Index (defined below) | 3 | 3 |
| Vulnerability Index (defined below) | 2.3 | 2.3 |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | |
|--|------|
| [100-yr] Water Surface Elevation (ft - MSL) | 8.3 |
| Bed Elevation (ft - MSL) | -2 |
| Low Chord Elevation (ft - MSL) | 2.7 |
| [100-yr] Max Wave Crest Elevation (ft - MSL) | 11.0 |
| [100-yr] Wave Height (ft) | 4.7 |
| [100-yr] Wave Period (seconds) | 2.6 |
| | 2.7 |

| SPAN PROPERTIES | |
|-----------------------------------|------|
| Span Length (ft) | 19.0 |
| Span Width (ft) | 28.0 |
| Deck Thickness (ft) | 0.8 |
| Overhang (ft) | |
| Number of Beams | |
| Beam Dead Weight (lb/ft) - Each | |
| Beam Dead Weight (kip/ft) - Total | |
| Slab Dead Weight (kip/ft) | 3.5 |
| Total Dead Weight (kip/ft) | 3.5 |
| Resisting Moment (kip/ft) | 49.0 |
| Resisting Vertical Force (kip/ft) | 3.5 |
| | 3.5 |

| 100-YEAR FORCE-MOMENT VALUES | |
|--------------------------------------|--------|
| Maximum Vertical Force (kips/span) | 54.8 |
| Maximum Vertical Force (kips/ft) | 2.9 |
| Maximum Horizontal Force (kips/span) | 6.5 |
| Maximum Horizontal Force (kips/ft) | 0.3 |
| Maximum Moment (k-ft) | 1236.7 |
| Maximum Moment (k-ft/ft) | 65.1 |
| | 64.0 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable | Criticality Index | Multiplier | Description |
|----------------------------|----------------|------------------------|-------------------|------------|--|
| | | | 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| | | | 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| | | | 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| | | | 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

Notes:
 1 - Bridge spans 1-2 are potentially subject to wave energy.
 2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

LADOTD BRIDGE RECALL NO. 033700
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| SPAN NUMBER | | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 |
|-------------------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|----|
| Criticality Index (defined below) | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | |
| Vulnerability Index (defined below) | 0.2 | 0.1 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | |

| SURGE/WAVE LOAD COMPUTATION INPUT VALUES | | |
|--|--|--|
|--|--|--|

| HYDRAULIC VALUES | |
|--|------|
| 100-yr Water Surface Elevation (ft - MSL) | 9.7 |
| Bad Elevation (ft - MSL) | -8.9 |
| Low Chord Elevation (ft - MSL) | 13.6 |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 14.8 |
| 100-yr Wave Height (ft) | 7.4 |
| 100-yr Wave Period (seconds) | 3.8 |

| SPAN PROPERTIES | |
|-----------------------------------|------|
| Span Length (ft) | 20.0 |
| Span Width (ft) | 27.0 |
| Deck Thickness (ft) | 1.0 |
| Overhang (ft) | |
| Number of Beams | |
| Beam Dead Weight (lb/ft) - Each | |
| Beam Dead Weight (kip/ft) - Total | |
| Slab Dead Weight (kip/ft) | 4.1 |
| Total Dead Weight (kip/ft) | 4.1 |
| Resisting Moment (kip/ft) | 54.7 |
| Resisting Vertical Force (kip/ft) | 4.1 |

| 100-YEAR FORCE/MOMENT VALUES | |
|--------------------------------------|------|
| Maximum Vertical Force (kips/span) | 9.4 |
| Maximum Vertical Force (kips/ft) | 0.5 |
| Maximum Horizontal Force (kips/span) | 0.6 |
| Maximum Horizontal Force (kips/ft) | 0.0 |
| Maximum Moment (kip-ft) | 67.5 |
| Maximum Moment (k-ft/ft) | 3.4 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable | Criticality Index | Multiplier | Description |
|----------------------------|----------------|------------------------|-------------------|------------|--|
| | | | 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| | | | 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| | | | 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| | | | 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

- Notes:
- 1 - Bridge spans are not potentially subject to wave energy.
 - 2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

LADOTD BRIDGE RECALL NO. 033700
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| SPAN NUMBER | BRIDGE VULNERABILITY SUMMARY | | | | |
|-------------------------------------|------------------------------|-----|-----|-----|-----|
| | 19 | 20 | 21 | 22 | 23 |
| Criticality Index (defined below) | 4 | 4 | 4 | 4 | 4 |
| Vulnerability Index (defined below) | 0.0 | 0.0 | 0.0 | 0.0 | 0.1 |

| SURGE/WAVE LOAD COMPUTATION INPUT VALUES | | |
|--|--|--|
|--|--|--|

| HYDRAULIC VALUES | |
|--|------|
| 100-yr Water Surface Elevation (ft - MSL) | 9.6 |
| Bed Elevation (ft - MSL) | -0.7 |
| Low Chord Elevation (ft - MSL) | 15.6 |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 14.2 |
| 100-yr Wave Height (ft) | 6.5 |
| 100-yr Wave Period (seconds) | 3.6 |

| SPAN PROPERTIES | |
|-----------------------------------|------|
| Span Length (ft) | 20.0 |
| Span Width (ft) | 27.0 |
| Deck Thickness (ft) | 1.0 |
| Overhang (ft) | 1.0 |
| Number of Beams | |
| Beam Dead Weight (lb/ft) - Each | |
| Beam Dead Weight (kip/ft) - Total | |
| Slab Dead Weight (kip/ft) | 4.1 |
| Total Dead Weight (kip/ft) | 4.1 |
| Resisting Moment (kip/ft) | 54.7 |
| Resisting Vertical Force (kip/ft) | 4.1 |

| 100-YEAR FORCE-MOMENT VALUES | |
|--------------------------------------|-----|
| Maximum Vertical Force (kips/span) | 0.9 |
| Maximum Vertical Force (kip/ft) | 0.0 |
| Maximum Horizontal Force (kips/span) | 0.0 |
| Maximum Horizontal Force (kip/ft) | 0.0 |
| Maximum Moment (kip-ft) | 0.0 |
| Maximum Moment (kip-ft) | 0.0 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable |
|----------------------------|----------------|------------------------|
|----------------------------|----------------|------------------------|

| Criticality Index | Multiplier | Description |
|-------------------|------------|--|
| 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

Notes:

- 1 - Bridge spans are not potentially subject to wave energy.
- 2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

LADOTD BRIDGE RECALL NO. 059432
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| SPAN NUMBER | | 1 | 2 | 3 | 4 | 5 | 6 | BRIDGE VULNERABILITY SUMMARY | |
|--|-----|-----|-----|-----|-----|-----|---|------------------------------|--|
| Criticality Index (defined below) | 3 | 3 | 3 | 3 | 3 | 3 | | | |
| Vulnerability Index (defined below) | 1.7 | 0.2 | 0.5 | 1.3 | 0.6 | | | | |
| HYDRAULIC VALUES | | | | | | | | | |
| 100-yr Water Surface Elevation (ft - MSL) | 6.7 | 6.7 | 6.7 | 6.7 | 6.7 | 6.7 | | | |
| Bad Elevation (ft - MSL) | -3 | -19 | -28 | -23 | -9 | 0 | | | |
| Low Chord Elevation (ft - MSL) | 6.1 | 7.3 | 7.1 | 7.1 | 6.1 | 6.8 | | | |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 8.5 | 8.5 | 8.5 | 8.3 | 8.1 | 8.0 | | | |
| 100-yr Wave Height (ft) | 3.5 | 2.6 | 2.6 | 2.3 | 2.0 | 2.8 | | | |
| 100-yr Wave Period (seconds) | 2.3 | 2.3 | 2.3 | 2.4 | 2.3 | 2.2 | | | |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| SPAN PROPERTIES | | SPAN LENGTH (ft) | | SPAN WIDTH (ft) | | DECK THICKNESS (ft) | | OVERHANG (ft) | |
|-----------------------------------|------|------------------|------|-----------------|------|---------------------|------|---------------|--|
| Span Length (ft) | 51.0 | 51.0 | 51.0 | 240.0 | 51.0 | 51.0 | 51.0 | | |
| Span Width (ft) | 37.7 | 37.7 | 37.7 | 37.7 | 37.7 | 37.7 | 37.7 | | |
| Deck Thickness (ft) | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | 0.6 | | |
| Overhang (ft) | 2.8 | 2.8 | 2.8 | 2.8 | 2.8 | 2.8 | 2.8 | | |
| Number of Beams | 5 | 5 | 5 | SWING | 5 | 5 | 5 | | |
| Beam Dead Weight (lb/ft) - Each | 384 | 384 | 384 | SPAN | 384 | 384 | 384 | | |
| Beam Dead Weight (kip/ft) - Total | 1.9 | 1.9 | 1.9 | | 1.9 | 1.9 | 1.9 | | |
| Slab Dead Weight (kip/ft) | 3.5 | 3.5 | 3.5 | | 3.5 | 3.5 | 3.5 | | |
| Total Dead Weight (kip/ft) | 5.5 | 5.5 | 5.5 | | 5.5 | 5.5 | 5.5 | | |
| Resisting Moment (kip/ft) | 87.3 | 87.3 | 87.3 | | 87.3 | 87.3 | 87.3 | | |
| Resisting Vertical Force (kip/ft) | 5.5 | 5.5 | 5.5 | | 5.5 | 5.5 | 5.5 | | |

| 100-YEAR FORCE-MOMENT VALUES | |
|--------------------------------------|--------|
| Maximum Vertical Force (kips/span) | 129.8 |
| Maximum Vertical Force (kips/ft) | 2.5 |
| Maximum Horizontal Force (kips/span) | 7.5 |
| Maximum Horizontal Force (kips/ft) | 0.1 |
| Maximum Moment (kip-ft) | 4302.6 |
| Maximum Moment (k-ft/ft) | 84.4 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable |
|----------------------------|----------------|------------------------|
|----------------------------|----------------|------------------------|

Notes:

- 1 - Bridge spans 1 and 5 are potentially subject to wave energy.
- 2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

| Criticality Index | Multiplier | Description |
|-------------------|------------|--|
| 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

Table 1 of 1

LADOTD BRIDGE RECALL NO. 060360
SUPERSTRUCTURE WAVE ENERGY EXPOSURE

| SPAN NUMBER | 1 | 2 | 3 | 4 | 5 |
|-------------------------------------|-----|-----|-----|-----|-----|
| Criticality Index (defined below) | 1 | 1 | 1 | 1 | 1 |
| Vulnerability Index (defined below) | 6.1 | 6.1 | 6.0 | 6.1 | 6.0 |

SURGE/WAVE LOAD COMPUTATION INPUT VALUES

| HYDRAULIC VALUES | |
|--|------|
| 100-yr Water Surface Elevation (ft - MSL) | 6.5 |
| Bed Elevation (ft - MSL) | -3.4 |
| Low Chord Elevation (ft - MSL) | -2.3 |
| 100-yr Max Wave Crest Elevation (ft - MSL) | 9.4 |
| 100-yr Wave Height (ft) | 4.4 |
| 100-yr Wave Period (seconds) | 5.1 |

SPAN PROPERTIES

| Span Length (ft) | 20.0 | 20.0 | 20.0 | 20.0 |
|-----------------------------------|------|------|------|------|
| Span Width (ft) | 21.0 | 21.0 | 21.0 | 21.0 |
| Deck Thickness (ft) | 0.8 | 0.8 | 0.8 | 0.8 |
| Overhang (ft) | | | | |
| Number of Beams | 14 | 14 | 14 | 14 |
| Beam Dead Weight (lb/ft) - Each | 19 | 19 | 19 | 19 |
| Beam Dead Weight (kip/ft) - Total | 0.3 | 0.3 | 0.3 | 0.3 |
| Slab Dead Weight (kip/ft) | 0.4 | 0.4 | 0.4 | 0.4 |
| Total Dead Weight (kip/ft) | 0.7 | 0.7 | 0.7 | 0.7 |
| Resisting Moment (kip-ft) | 7.0 | 7.0 | 7.0 | 7.0 |
| Resisting Vertical Force (kip/ft) | 0.7 | 0.7 | 0.7 | 0.7 |

100-YEAR FORCE-MOMENT VALUES

| Maximum Vertical Force (kips/span) | 66.0 | 66.3 | 66.1 | 66.3 | 66.2 |
|--------------------------------------|-------|-------|-------|-------|-------|
| Maximum Vertical Force (kips/ft) | 3.3 | 3.3 | 3.3 | 3.3 | 3.3 |
| Maximum Horizontal Force (kips/span) | 7.2 | 7.1 | 7.1 | 7.1 | 7.1 |
| Maximum Horizontal Force (kips/ft) | 0.4 | 0.4 | 0.4 | 0.4 | 0.4 |
| Maximum Moment (kip-ft) | 848.4 | 850.9 | 843.9 | 846.0 | 845.3 |
| Maximum Moment (kip-ft/ft) | 42.4 | 42.5 | 42.2 | 42.3 | 42.3 |

| Vulnerability Index Legend | Not Vulnerable | Potentially Vulnerable |
|----------------------------|----------------|------------------------|
|----------------------------|----------------|------------------------|

Notes:

1 - Bridge spans 1-5 are potentially subject to wave energy.

2 - Bridge Vulnerability Rating is defined as the greater value between the Ratio (Max Vertical Force / Resisting Vertical Force) and Ratio (Max Moment / Resisting Moment)

| Criticality Index | Multiplier | Description |
|-------------------|------------|--|
| 1 | 1 | Minor impact to economy or emergency needs if closed (alternative routes exist) |
| 2 | 1 | Medium impact if closed - may lead to a barrier island but an alternative route exists |
| 3 | 1.75 | Major impact if closed - only road to a barrier island, evacuation route with no reasonable alternatives |
| 4 | 1.75 | Extreme impact if closed - Interstate or major economic connector (detour very long) |

Table 1 of 1

This public document is published at a total cost of \$250
42 copies of this public document were published in this first
printing at a cost of \$250. The total cost of all printings of
this document including reprints is \$250. This document was
published by Louisiana Transportation Research Center to
report and publish research findings as required in R.S. 48:105.
This material was duplicated in accordance with standards for
printing by state agencies established pursuant to R.S. 43:31.
Printing of this material was purchased in accordance with the
provisions of Title 43 of the Louisiana Revised Statutes.